PORT OF TACOMA

WASHINGTON UNITED TERMINAL FENDER SYSTEM REPLACEMENT PROJECT NO. - 201107.01 CONTRACT NO. - 071421

PORT COMMISSIONERS:

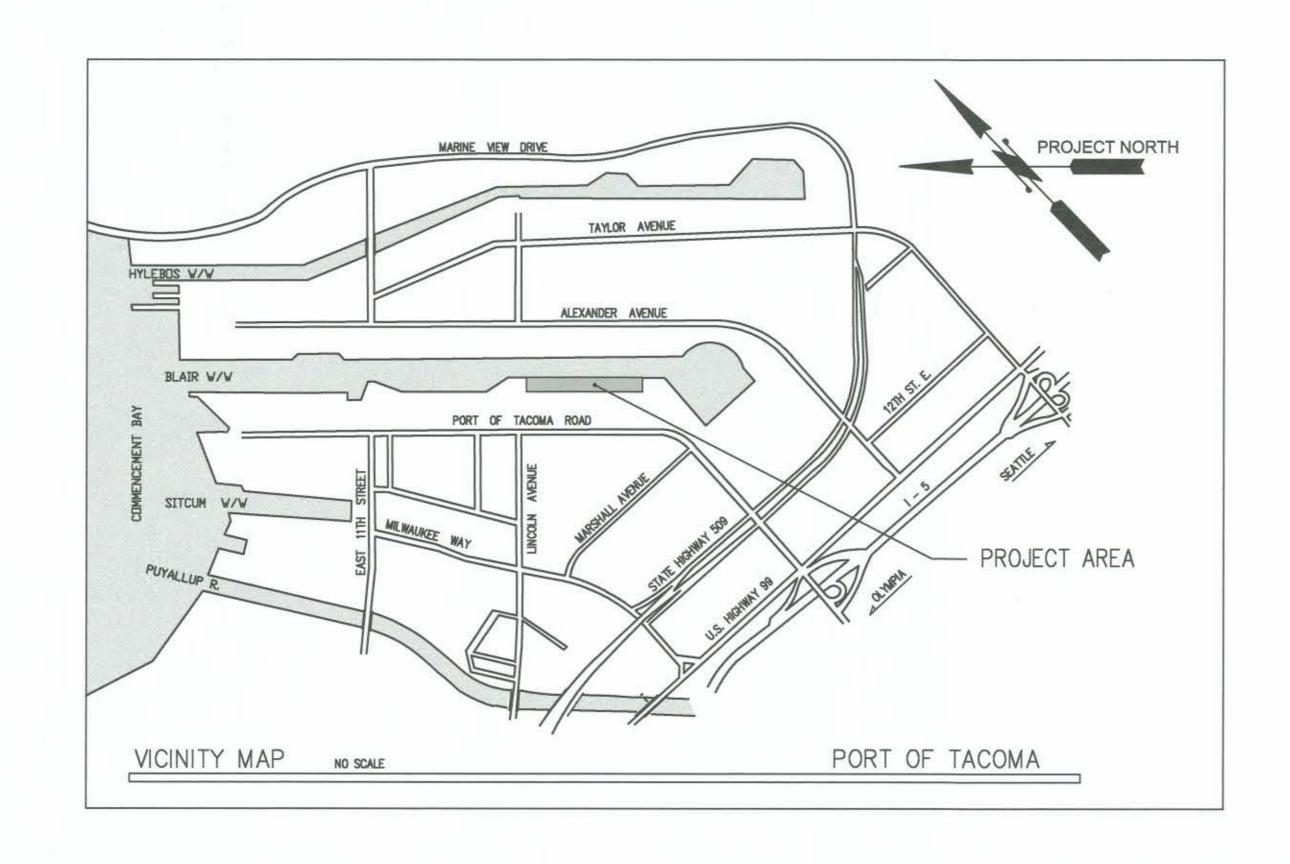
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SHEET NO.	DWG NO.	DRAWING TITLE
1	G1.1	COVER SHEET
2	G1.2	SYMBOLS, ABBREVIATIONS AND GENERAL NOTES
3	G2.1	CONSTRUCTION ACCESS, PHASING, AND SEQUENCING PLAN
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5	D1.2	DEMOLITION PLAN - SHEET 2
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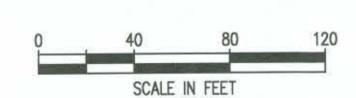
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PLAN SCALE: NTS



SURVEY CONTROL POINT

POT CONTROL POINT

N XXXX E XXXX COORDINATES

(67) INDICATES BENT NUMBERS

ABBREVIATIONS

#	NUMBER	LT.	LEFT
# ø	DIAMETER	M&R	MAINTENANCE AND REPAIR
0	AT	MAT	MATERIAL
ACP	ASPHALTIC CONCRETE PAVEMENT	MAX	MAXIMUM
ACI	AMERICAN CONCRETE INSTITUTE	MFR	MANUFACTURER
ADDL	ADDITIONAL	MHHW	MEAN HIGHER HIGH WATER
APPROX	APPROXIMATE	MHW	MEAN HIGH WATER
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	MLLW	MEAN LOWER LOW WATER
AVG	AVERAGE	MIN	MINIMUM
AWS	AMERICAN WELDING SOCIETY	MISC	MISCELLANEOUS
BLDG	BUILDING	NDT	NONDESTRUCTIVE TESTING
BOT	BOTTOM	NIC	NOT IN CONTRACT
	CENTERLINE	No.	NUMBER
CC	CENTER TO CENTER	NTS	NOT TO SCALE
	CONTROL DENSITY FILL	OC	ON CENTER
CDF	CAST IN PLACE	OD	OUTSIDE DIAMETER
CIP	CLEAR or CLEARANCE	OVH	OVERHEAD
CLR	CONCRETE	PL	PLATE
CONC	CONSTRUCT	POT	PORT OF TACOMA
CONST	CONE PENETROMETER TEST	POV	PRIVATELY OWNED VEHICLE
CPT		PSI	POUNDS PER SQUARE INCH
CTR	CENTER	PVMT	PAVEMENT
DET	DETAIL	R	RIDGE or RADIUS
DIA	DIAMETER	REF	REFERENCE
DIM	DIMENSION DRAWING	REINF	REINFORCE OR REINFORCING
DWG		REQ'D	REQUIRED
EA	EACH EXISTING CRADE	SCH or SCHED	SCHEDULE
EG	EXISTING GRADE	SHT	SHEET
EL	ELEVATION	SIM	SIMILAR
ELEC	ELECTRICAL	SPECS	SPECIFICATIONS
ENGR	ENGINEER	SQFT	SQUARE FEET
EQ	EQUAL	STD	STANDARD
ETC	ET CETERA	SYM	SYMMETRIC or SYMBOL
EXIST	EXISTING	TYP	TYPICAL
FT	FOOT, FEET	UNO	UNLESS NOTED OTHERWISE
HT	HEIGHT	V or VERT	VERTICAL
ID	INSIDE DIAMETER	W/	WITH
IN	INCH	W/O	WITHOUT
INCL	INCLUDE	WELD	WELDING
INFO	INFORMATION	WABO	WASHINGTON ASSOCIATION O
JT	JOINT		BUILDING ASSOCIATES
15	LENGTH	WHS	WELDED HEADED STUD
LF	LINEAL FOOT	70/3/2	ADMINISTRAÇÃO MODERNA AMERICA PARA PORTO
LIN	LINEAL or LINEAR		

GENERAL NOTES

- ALL CONSTRUCTION SHALL CONFORM TO THESE PLANS AND SPECIFICATIONS.
- 2. CONTRACTOR SHALL VERIFY ALL LEVELS, DIMENSIONS, AND EXISTING CONDITIONS IN THE FIELD BEFORE PROCEEDING. CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES OR FIELD CHANGES PRIOR TO INSTALLATION OR FABRICATION. IN CASE OF DISCREPANCIES BETWEEN THE EXISTING CONDITIONS AND THE PLANS, THE CONTRACTOR SHALL OBTAIN DIRECTION FROM THE ENGINEER BEFORE PROCEEDING. DIMENSIONS AND CALLOUTS NOTED AS PLUS OR MINUS (±) OR (REF) INDICATE UNVERIFIED DIMENSIONS AND ARE APPROXIMATE. NOTIFY THE ENGINEER IMMEDIATELY OF CONFLICTS OR EXCESSIVE VARIATIONS FROM AS INDICATED. NOTED DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS DO NOT SCALE THE PLANS.
- 3. A COPY OF THE PLANS SHALL BE ON-SITE WHENEVER CONSTRUCTION IS IN PROGRESS. THROUGHOUT THE PROGRESS OF THE WORK OF THIS CONTRACT, MAINTAIN AN ACCURATE RECORD OF ALL CHANGES IN THE CONTRACT DOCUMENTS. UPON THE COMPLETION OF THIS CONTRACT, PROVIDE ONE COMPLETE SET OF RECORD DOCUMENTS TO THE PORT OF TACOMA.
- 4. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS OF EXISTING STRUCTURES AND OTHER FEATURES THAT MAY IMPACT THE WORK. CONTRACTOR SHALL BRING ANY CONFLICTS TO THE ENGINEER'S ATTENTION PRIOR TO BEGINNING AFFECTED WORK.
- 5. ANY DAMAGE TO EXISTING UTILITIES, OTHER FACILITIES OR EQUIPMENT DUE TO THE CONTRACTOR'S NEGLIGENCE, EXCEPT FOR ITEMS DESIGNATED FOR DEMOLITION, SHALL BE PROMPTLY REPAIRED BY THE CONTRACTOR AT HIS EXPENSE. THIS INCLUDES ITEMS OUTSIDE THE WORK AREA AND WITHIN THE PORT OF TACOMA PROPERTY THAT ARE DAMAGED BY CONSTRUCTION ACTIVITIES DURING EXECUTION OF THIS CONTRACT.
- THE CONTRACTOR SHALL KEEP ALL STREETS AND VEHICULAR TRAFFIC AREAS USED FOR THIS WORK CLEAN AT ALL TIMES, SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- CONTRACTOR IS RESPONSIBLE FOR ANY TRAFFIC CONTROLS REQUIRED DURING THE DURATION OF THIS PROJECT, PER CONTRACTOR'S OPERATION. ALL TRAFFIC CONTROL SHALL BE IN ACCORDANCE WITH THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.
- 8. MAINTAIN UTILITY SERVICE TO EXISTING BUILDINGS, BULLRAIL UTILITIES AND FIRE HYDRANTS DURING CONSTRUCTION UNLESS NOTED OTHERWISE OR APPROVED BY THE ENGINEER.
- RECORD DRAWINGS OF EXISTING WHARF AND FENDER SYSTEMS ARE AVAILABLE FROM THE PORT.

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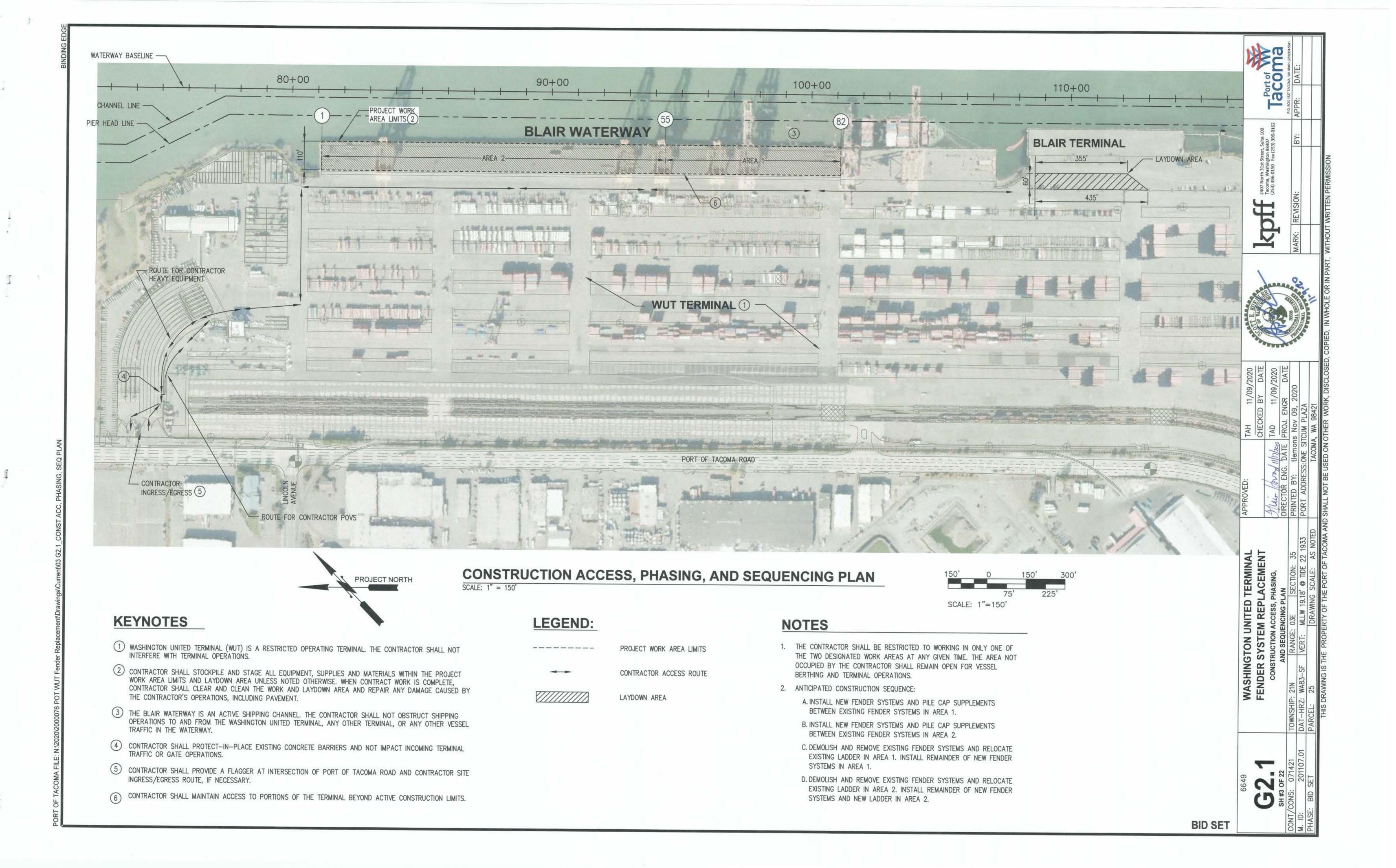
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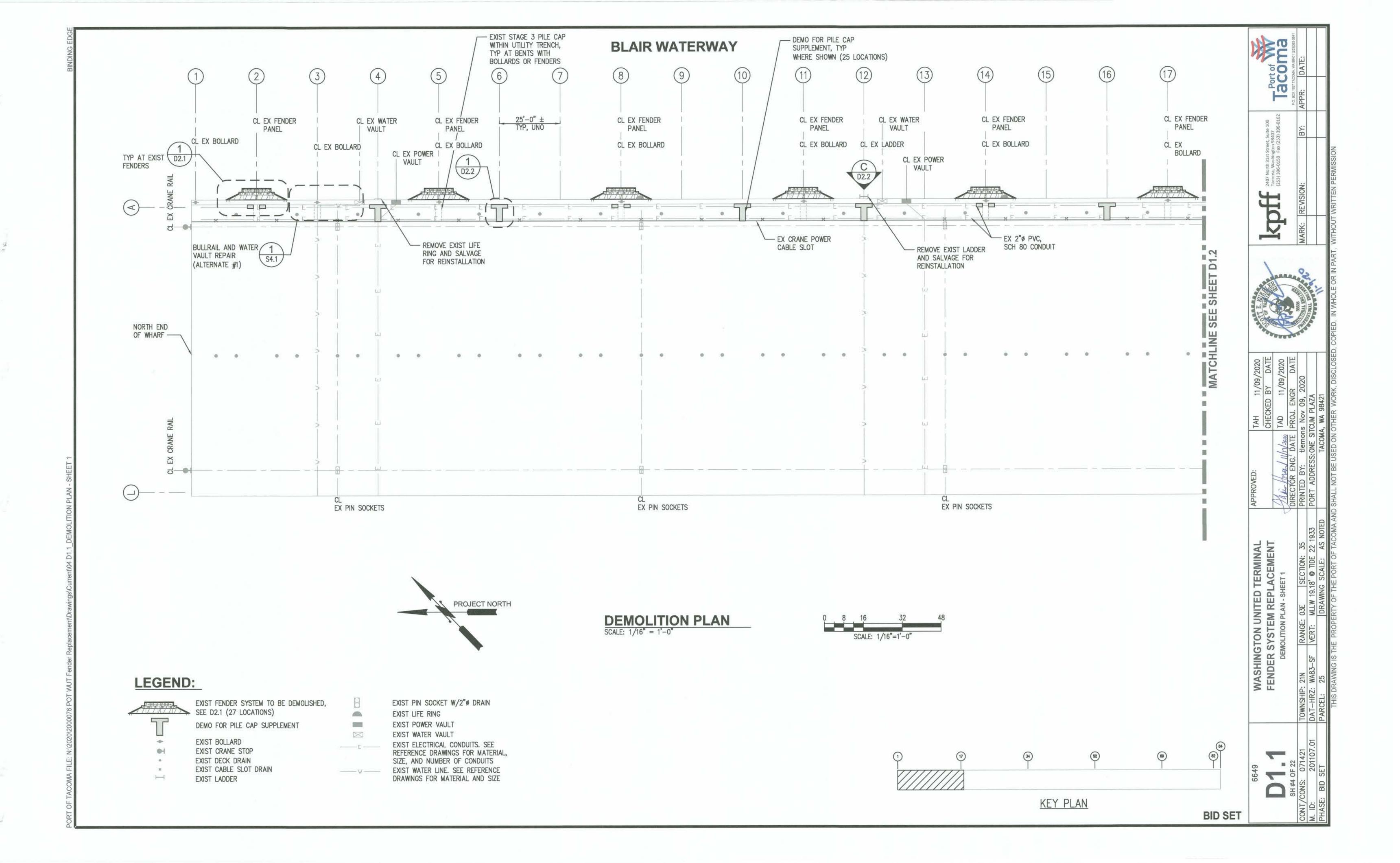
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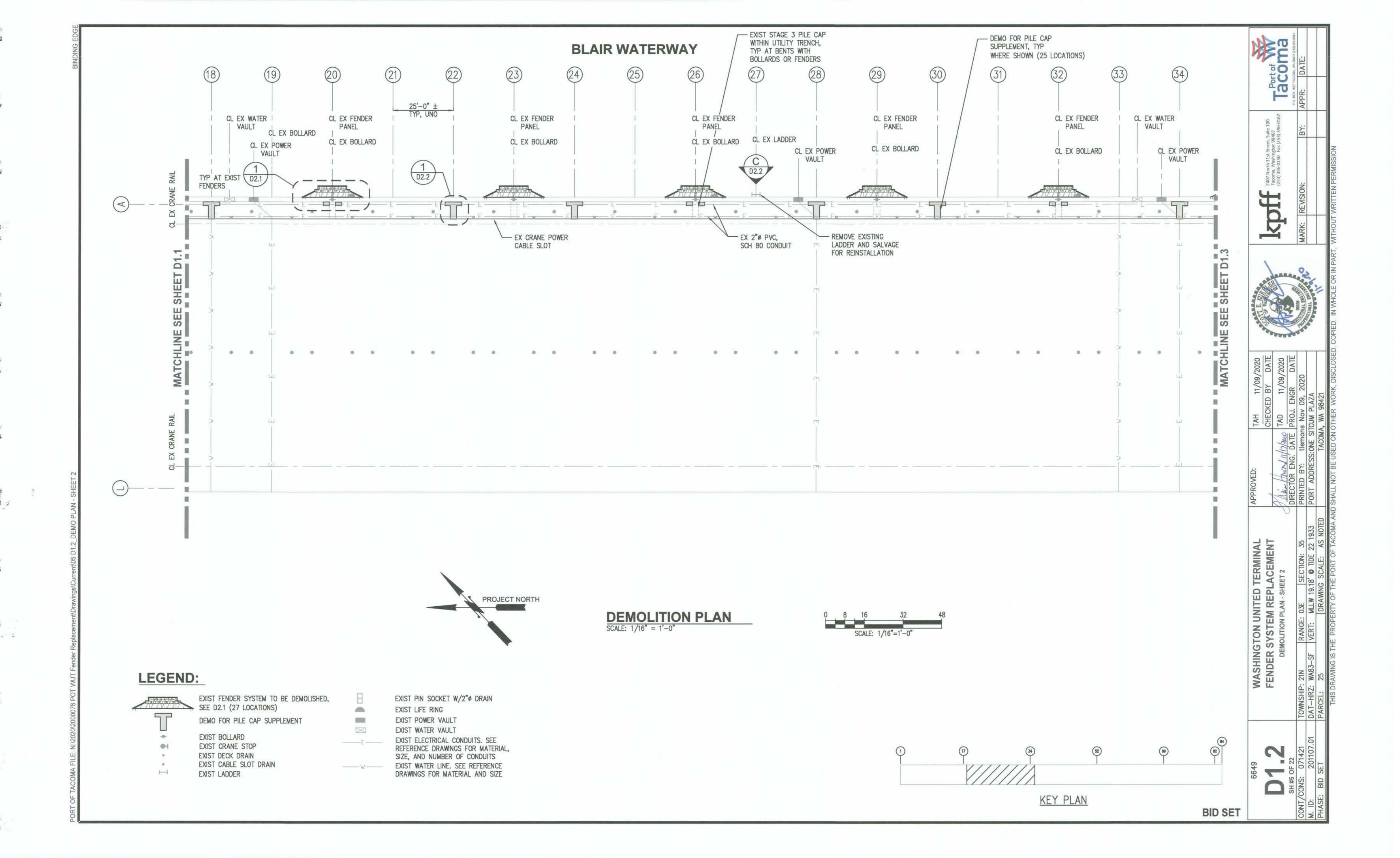
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SYMBOLS, ABBREVIATIONS AND GENERAL NOTESTOWNSHIP: 21N RANGE: 03E SECTION: 35

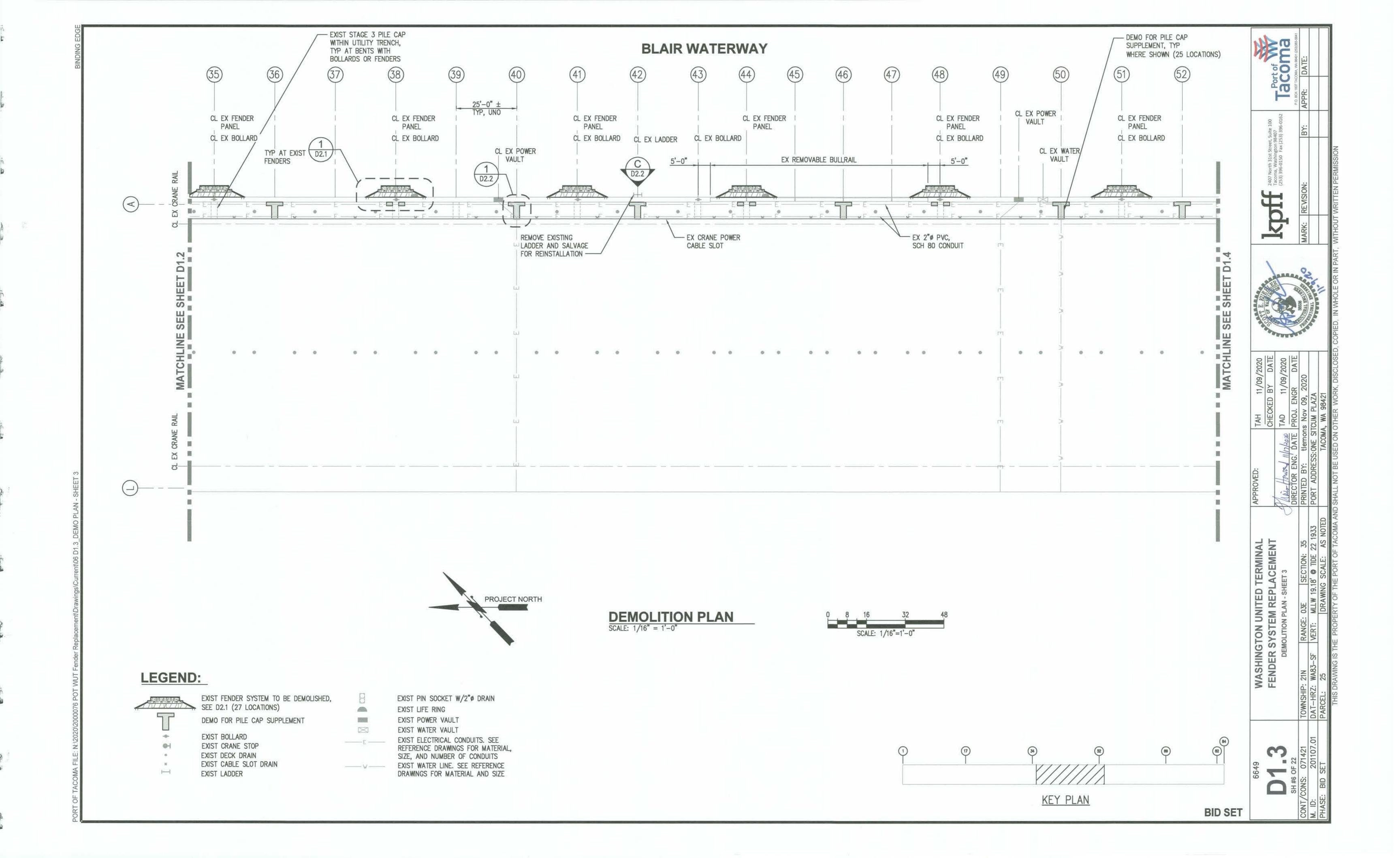
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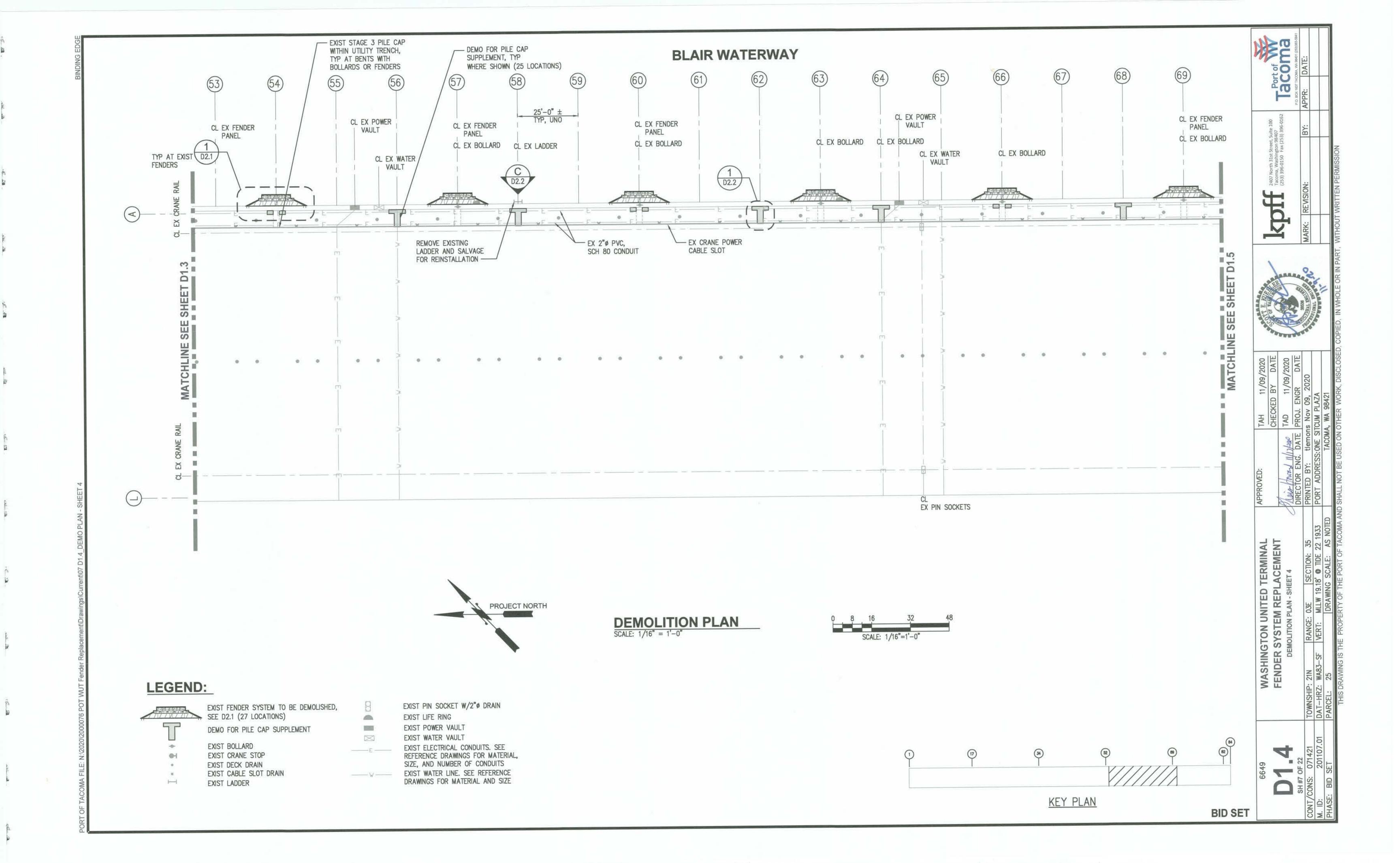
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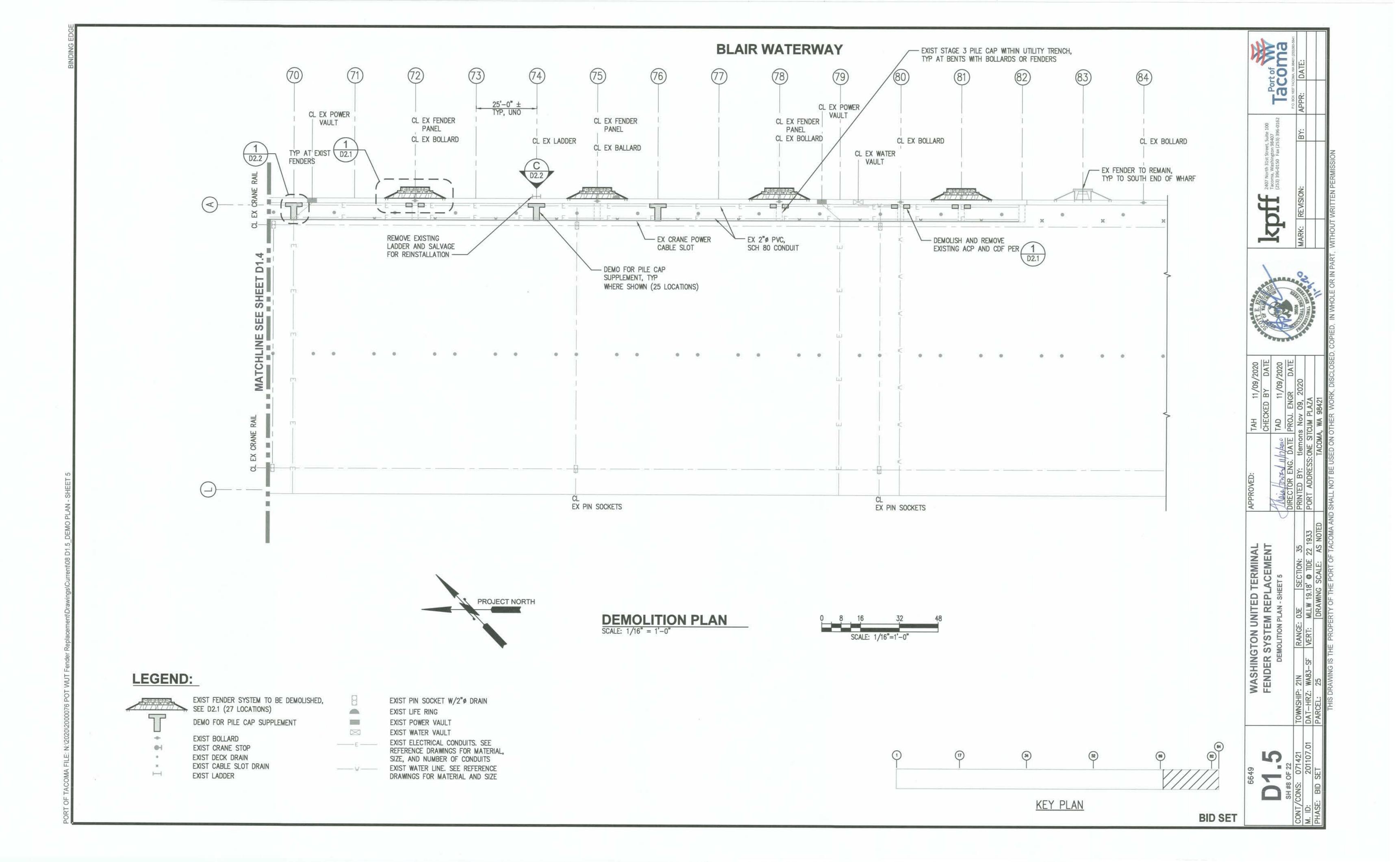


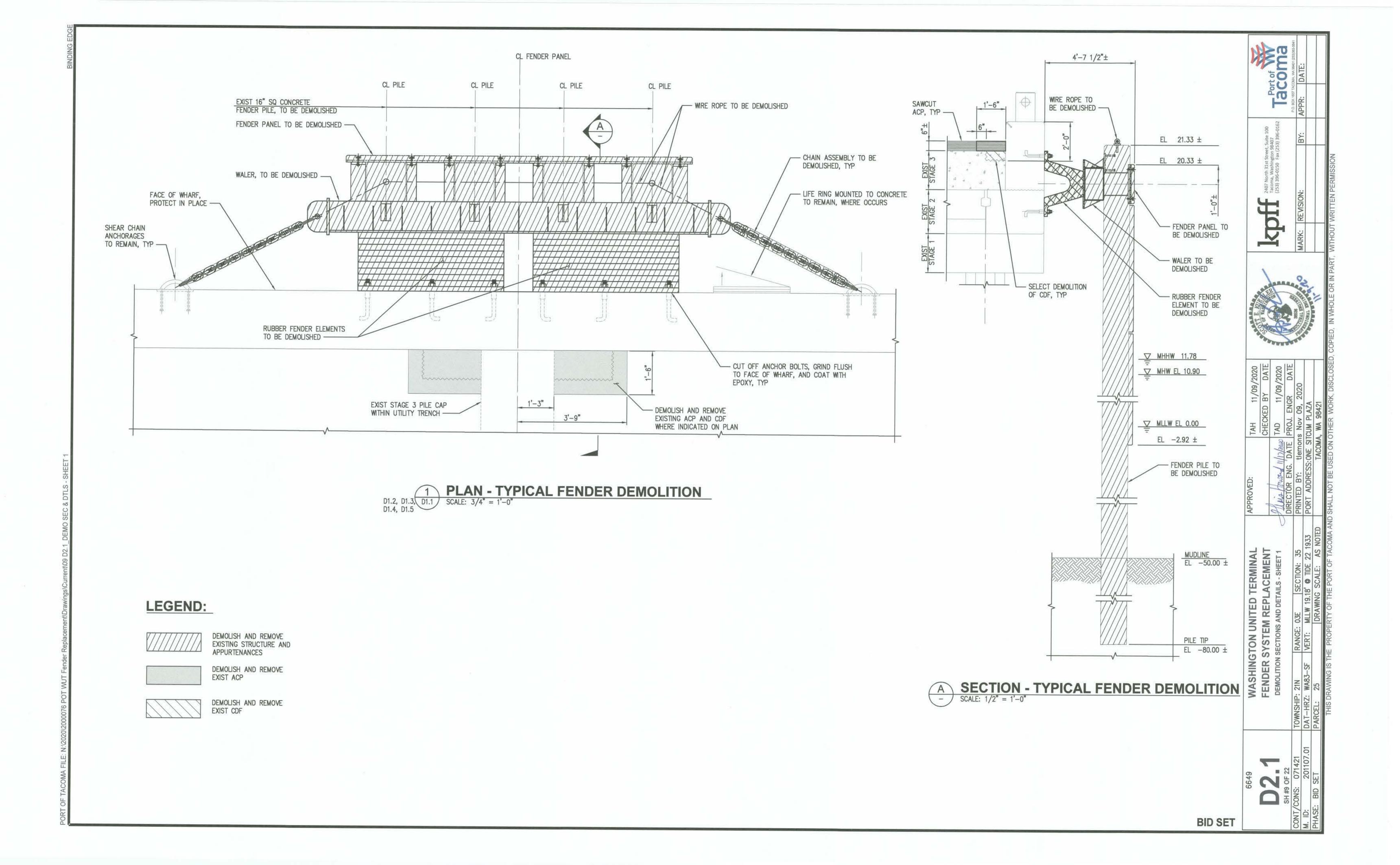


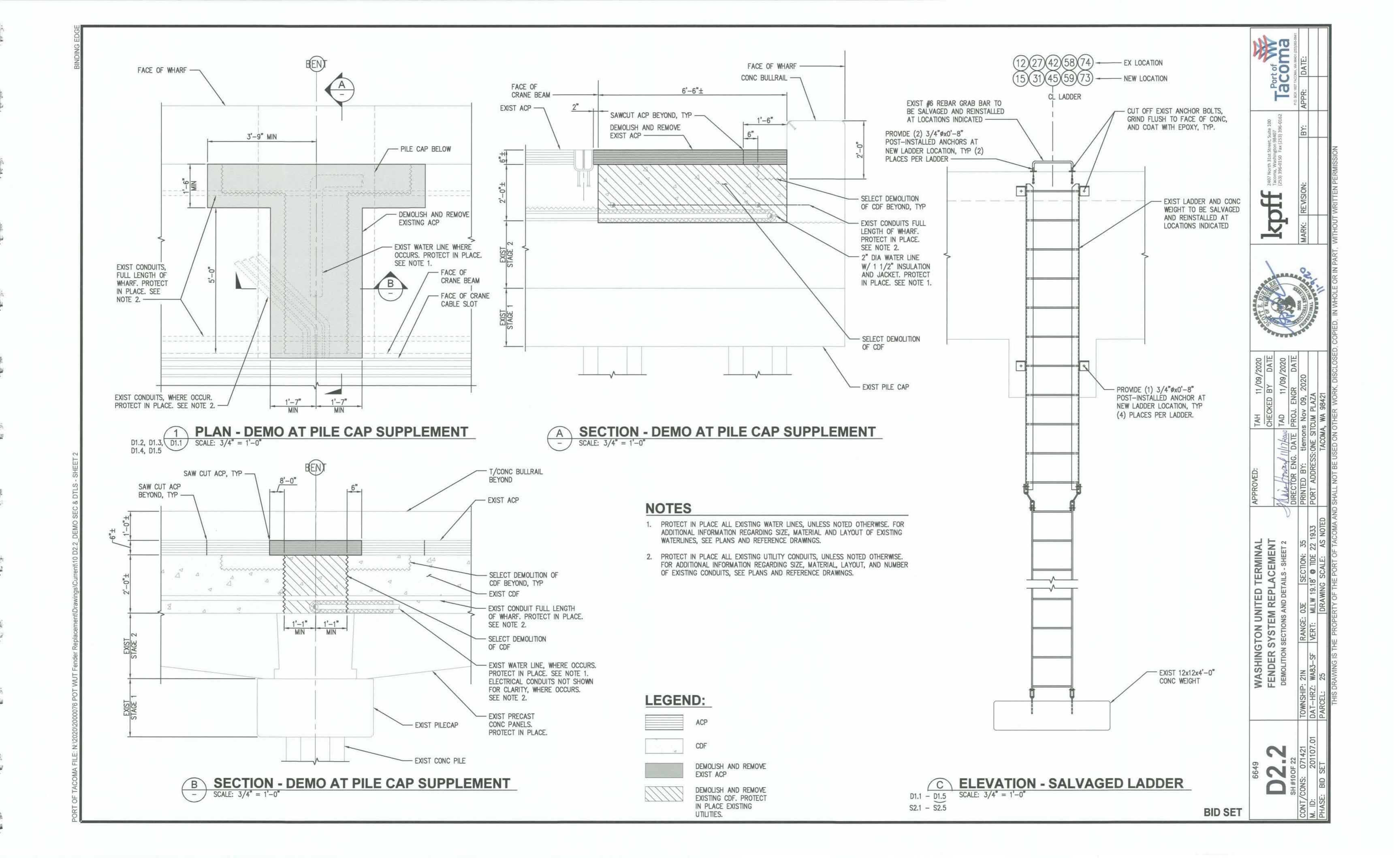












- 2. ALL DIMENSIONS AND DETAILS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND CONSTRUCTION.
- 3. ALL SHOP DRAWINGS FOR FENDER SYSTEM SHALL BE SUBMITTED TO AND REVIEWED BY THE ENGINEER PRIOR TO FABRICATION.
- 4. VERTICAL DATUM: MLLW = 0.00'

CODES AND STANDARDS

- 1. ALL METHODS AND MATERIALS SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE, 2015 EDITION.
- 2. REINFORCED CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".
- 3. STRUCTURAL AND MISCELLANEOUS STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" 2010.
- 4. WELDING OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE AWS D1.1, 2010.
- 5. WELDING OF REINFORCING STEEL SHALL CONFORM TO THE AWS D1.4.

DESIGN CRITERIA

VESSELS

CONTAINER SHIP 1 LENGTH OVERALL (LOA): BEAM: DRAFT: DISPLACEMENT:

1300 FEET 204 FEET 48 FEET 220,000 LT

CONTAINER SHIP 2 LENGTH OVERALL (LOA): DRAFT:

DISPLACEMENT:

902 FEET 131 FEET 40 FEET 90,000 LT

C. RO/RO VESSEL LENGTH OVERALL (LOA):

DISPLACEMENT:

790 FEET 110 FEET 38 FEET 60,000 LT

BERTHING LOADS

A. CONTAINER SHIP APPROACH VELOCITY: APPROACH ANGLE: PERPENDICULAR VELOCITY:

1.5 KNOTS (2.53 FT/S) 6 DEGREES 0.16 KNOT (0.26 FT/S)

RO/RO VESSEL APPROACH VELOCITY: APPROACH ANGLE: PERPENDICULAR VELOCITY: 0.24 KNOT (0.4 FT/S)

1.36 KNOT (2.3 FT/S) 10 DEGREES

FENDER REQUIREMENTS

A. MINIMUM BERTHING ENERGY = 490 KIP-FT

B. MAXIMUM BERTHING REACTION = 320 KIPS

REINFORCED CONCRETE

REINFORCING STEEL

- A. ALL REINFORCING STEEL SHALL BE DEFORMED STEEL BARS CONFORMING TO ASTM A 615 - GRADE 60, EXCEPT AS NOTED.
- B. PROVIDE CORNER BARS AT ALL CORNERS. CORNER BARS SHALL MATCH THE NUMBER, SPACING AND DIAMETER OF ALL HORIZONTAL REINFORCEMENT AT THE CORNER, UNLESS SHOWN OTHERWISE, TERMINATED STRAIGHT BARS SHALL EXTEND FULL AVAILABLE LENGTH INTO ADJOINING MEMBERS. SPACE BARS EQUALLY BETWEEN DIMENSIONED LIMITS. UNLESS NOTED OTHERWISE.
- C. MECHANICAL SPLICES SHALL DEVELOP AT LEAST 125 PERCENT OF THE YIELD STRENGTH OF THE BAR IN TENSION.
- D. REINFORCING SHALL BE SUPPORTED AS SPECIFIED BY THE PROJECT SPECIFICATIONS AND THE CRSI "MANUAL OF STANDARD PRACTICE," (MSP). REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH "ACI DETAILING MANUAL," ACI SP-66.
- E. ALL HORIZONTAL REINFORCEMENT AT DISCONTINUITIES AND CORNERS SHALL END WITH STANDARD 90° HOOKS IN ACCORDANCE WITH ACI SP-66, UNLESS SHOWN OTHERWISE.
- CAST-IN-PLACE CONCRETE

A. MINIMUM 7 DAY COMPRESSIVE STRENGTH

4000 PSI

- ALL EXPOSED CORNERS SHALL BE CHAMFERED 3/4 IN.
- 4. CONSTRUCTION JOINTS SHALL BE PROVIDED ONLY AS NOTED ON THE DRAWINGS AND AS SPECIFICALLY PERMITTED BY THE ENGINEER.
- 5. CONCRETE MIX SHALL HAVE A MAXIMUM WATER CEMENT RATIO OF 0.40 UNLESS NOTED OTHERWISE.
- 6. ALL CONCRETE SHALL BE AIR ENTRAINED WITH A TARGET OF $5\% \pm 1 \ 1/2\%$ UNLESS NOTED OTHERWISE IN THE SPECIFICATIONS.
- 7. REINFORCING BARS SHOWN AS EXISTING ARE PER PORT RECORD DRAWINGS. ACTUAL CONDITIONS MAY VARY. THE CONTRACTOR SHALL FIELD VERIFY EXISTING REINFORCING BAR SIZES AND QUANTITIES PRIOR TO ORDERING REINFORCING STEEL.

POST-INSTALLED ANCHORS AND DOWELS

- 1. USE OF DRILLED ADHESIVE CONCRETE ANCHORS OR DOWELS, WHERE NOT SPECIFIED IN THE DOCUMENTS, SHALL BE SUBJECT TO APPROVAL BY THE ENGINEER.
- ADHESIVE ANCHORS AND DOWELS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATION, SPECIAL INSPECTION IS REQUIRED FOR ALL ADHESIVE ANCHORS AND DOWELS.
- 3. ACCEPTABLE ADHESIVES ARE HILTI HIT-RE 500 V3 OR SIMPSON SET-XP. ICBO OR ICC REPORTS SHALL BE SUBMITTED FOR ALL ADHESIVE ANCHOR AND DOWEL PRODUCTS.
- 4. ALL POST-INSTALLED ANCHORS SHALL BE ASTM F1554 GR. 105 THREADED ROD UNLESS NOTED OTHERWISE, ALL POST-INSTALLED DOWELS SHALL BE ASTM A615 REINFORCING BAR.
- 5. HOLES FOR POST-INSTALLED ANCHORS AND DOWELS SHALL BE DRILLED BY HAMMER DRILL ONLY - CORE DRILLING NOT ALLOWED
- 6. CONTRACTOR SHALL FIELD LOCATE EXISTING SURFACE REINFORCING USING NDT METHODS, MARK LOCATIONS OF SURFACE REINFORCING, AND OBTAIN PORT APPROVAL PRIOR TO DRILLING. EXISTING REINFORCING SHALL NOT BE CUT WITHOUT PRIOR APPROVAL FROM THE PORT.

MISCELLANEOUS STEEL

- 1. ALL MISCELLANEOUS STEEL SHAPES AND PLATES, EXCEPT AS NOTED BELOW, SHALL CONFORM TO ASTM A 36.
- 2. STEEL PLATE NOTED AS GRADE 50 SHALL CONFORM TO ASTM A572, GRADE 50.
- 3. ECONOMY BOLTS SHALL CONFORM TO ASTM A 307, UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS.
- 4. ALL ANCHOR BOLTS SHALL CONFORM TO ASTM F 1554, GRADE 36.
- HIGH STRENGTH BOLTS FOR STEEL-TO-STEEL CONNECTIONS SHALL BE ASTM A325X.
- ALL NUTS SHALL BE ASTM A 563, UNLESS NOTED OTHERWISE.
- 7. ALL WASHERS SHALL BE ASTM F 436, UNLESS NOTED OTHERWISE.
- 8. ALL STEEL ITEMS SHALL BE HOT-DIP GALVANIZED AND/OR COATED IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS, UNLESS NOTED OTHERWISE.
- 9. SEE THE CONTRACT SPECIFICATIONS FOR ITEMS TO RECEIVE HIGH PERFORMANCE COATING IN ADDITION TO GALVANIZING.

WELDING

- 1. ALL WELDING SHALL BE PERFORMED BY WELDERS QUALIFIED FOR THE WELD AND POSITION SHOWN IN ACCORDANCE WITH AWS AND HAVING CURRENT CERTIFICATION FROM WABO.
- 2. ALL WELDS SHALL BE PERFORMED WITH PROCEDURES PREQUALIFIED OR QUALIFIED IN ACCORDANCE WITH AWS D1.1 AND D1.4.
- 3. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD SYMBOLS ARE SHOWN WHERE FIELD WELDS ARE REQUIRED BY THE STRUCTURAL DESIGN. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP- OR FIELD-WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL ERECTION.
- 4. WELDING ELECTRODES SHALL BE 70 KSI STRENGTH AND SHALL BE "LOW-HYDROGEN" ELECTRODES.

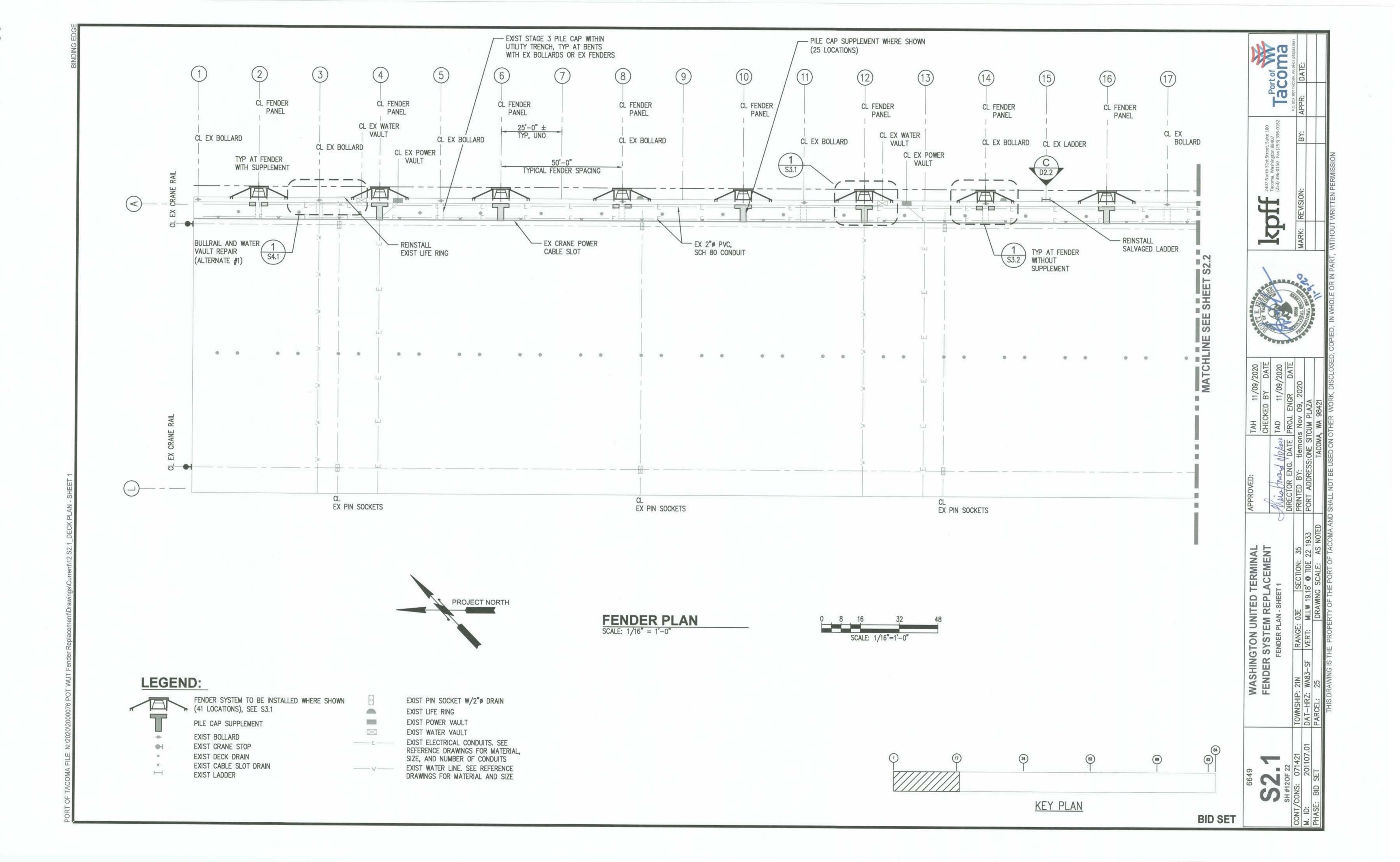
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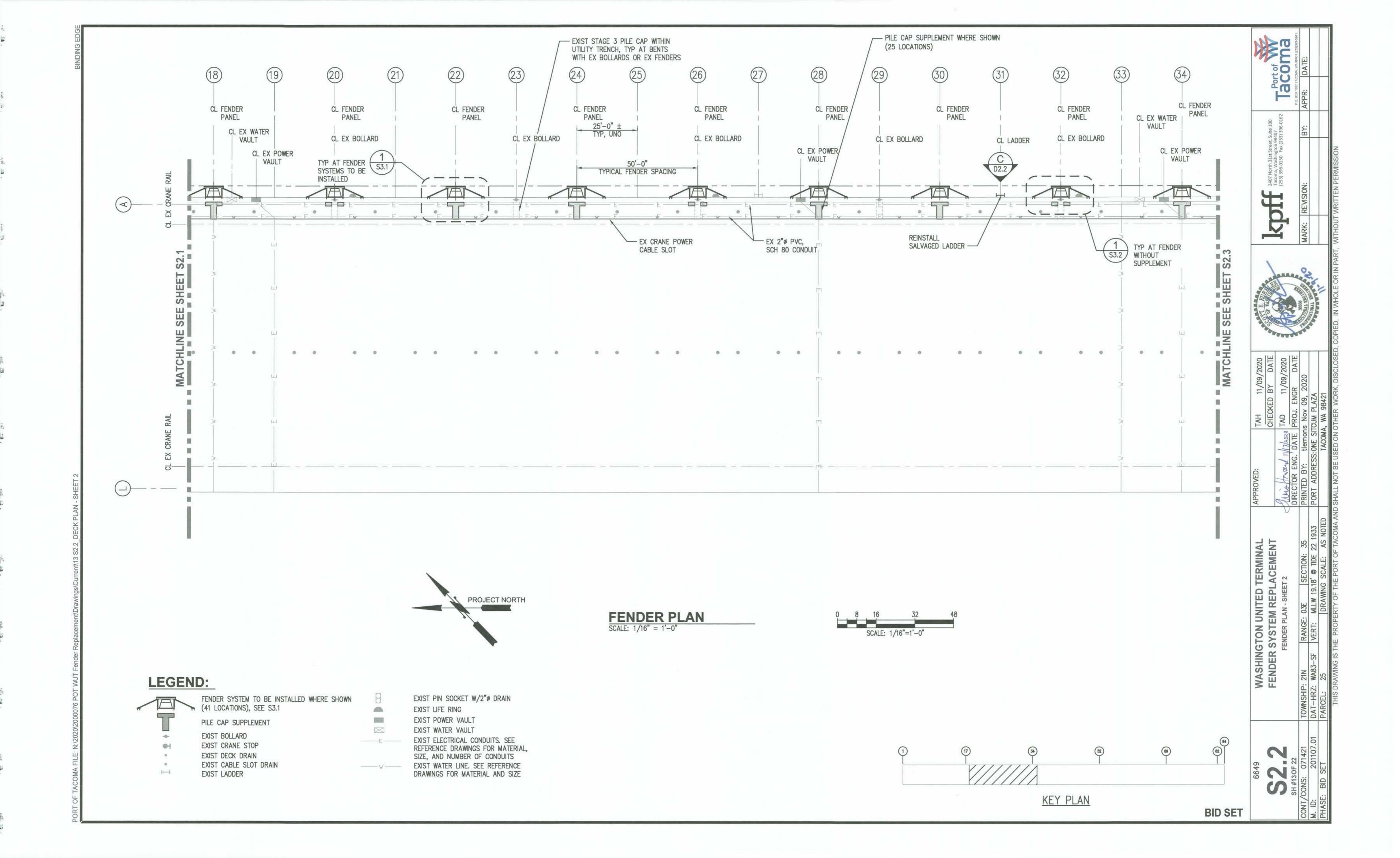
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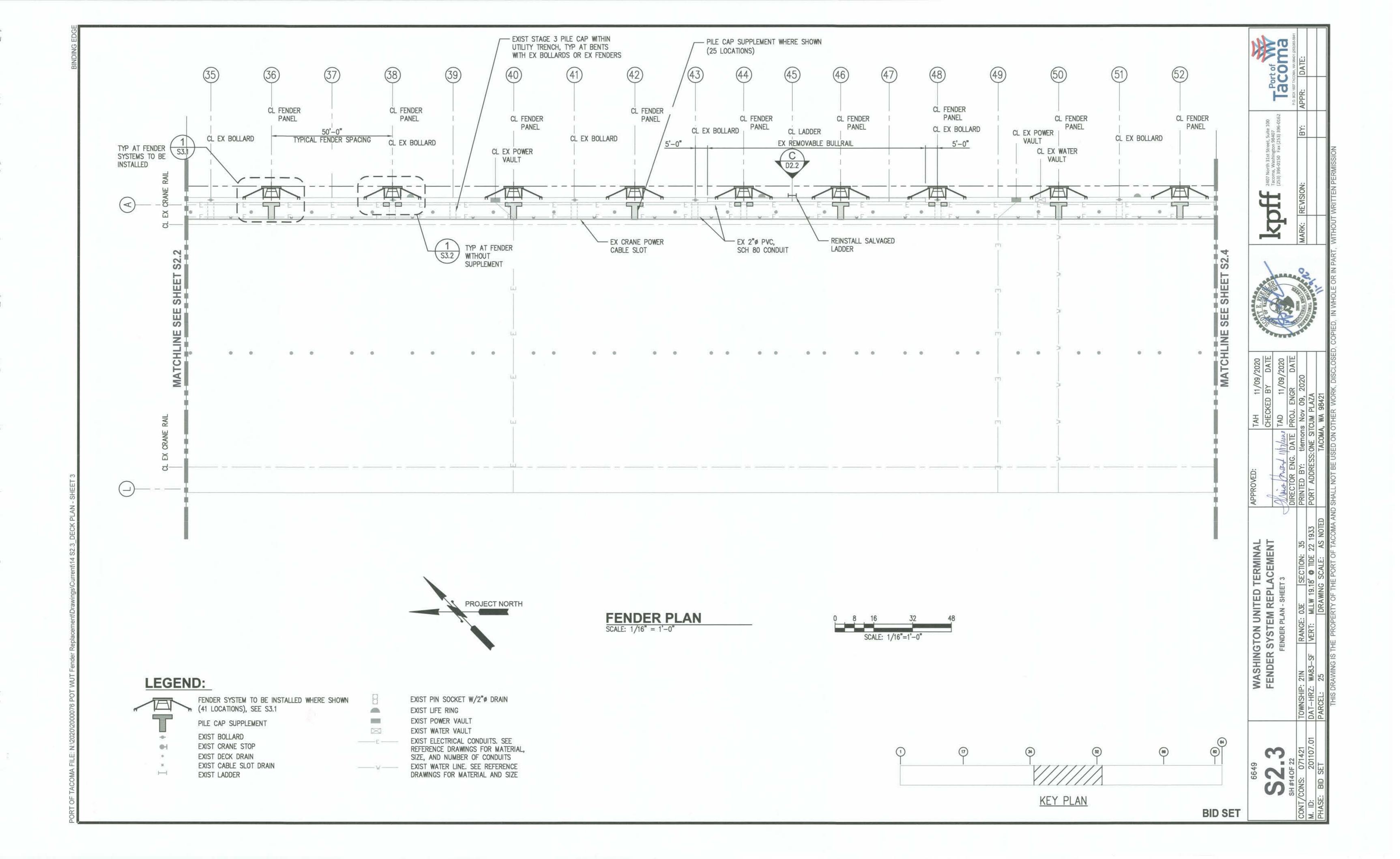
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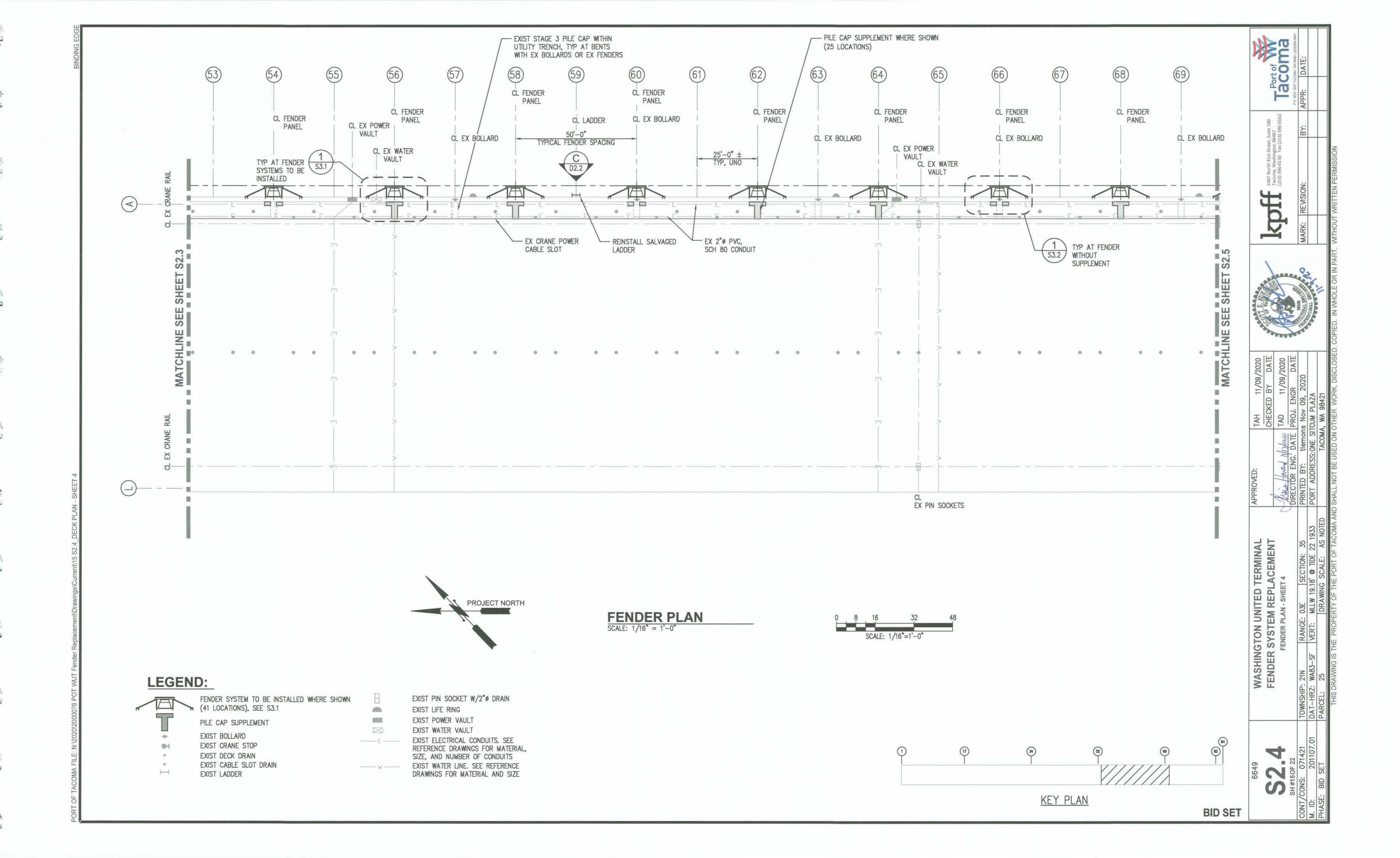
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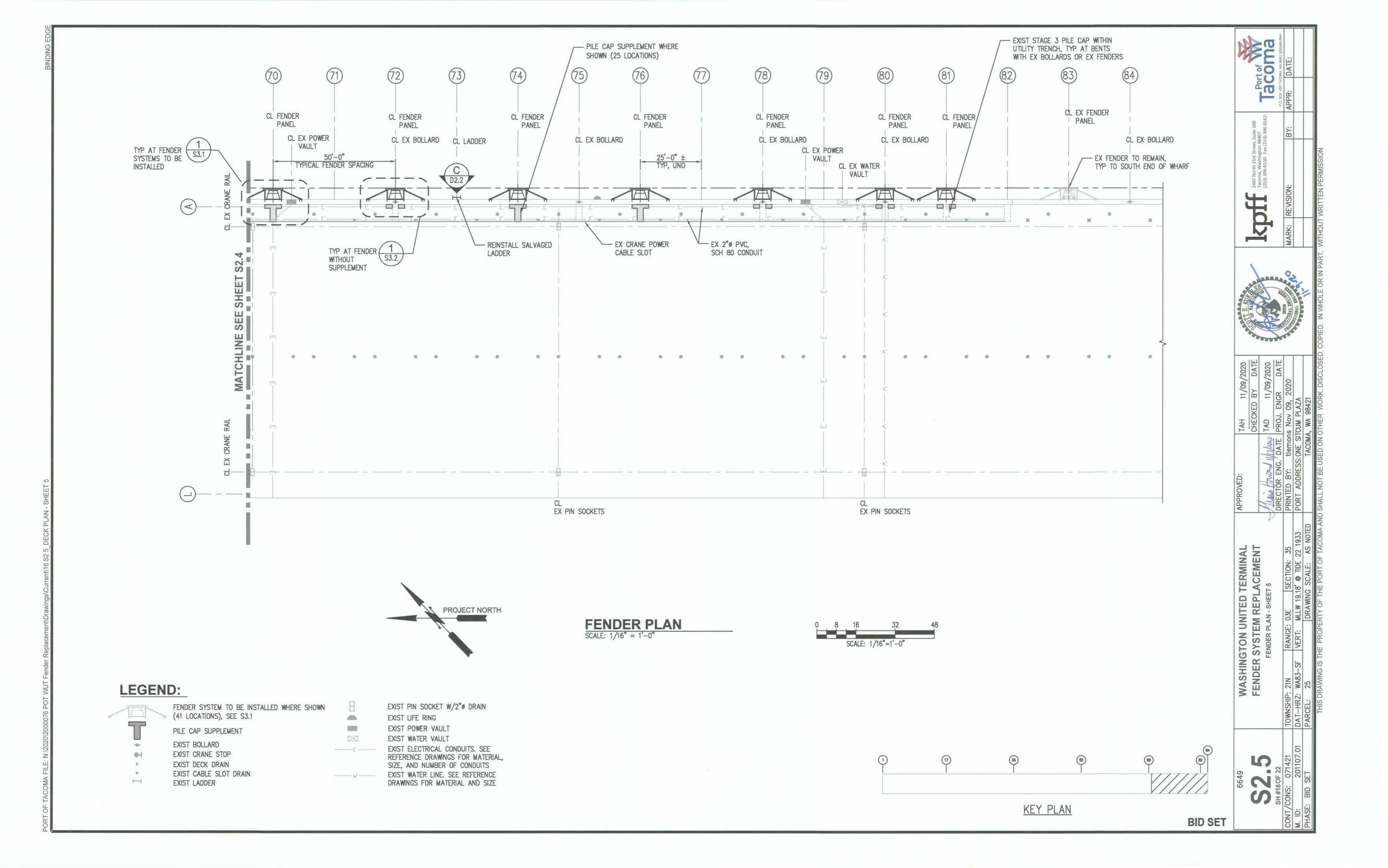
WASHINGTON UNITED TERMINAL FENDER SYSTEM REPLACEMENT STRUCTURAL NOTES

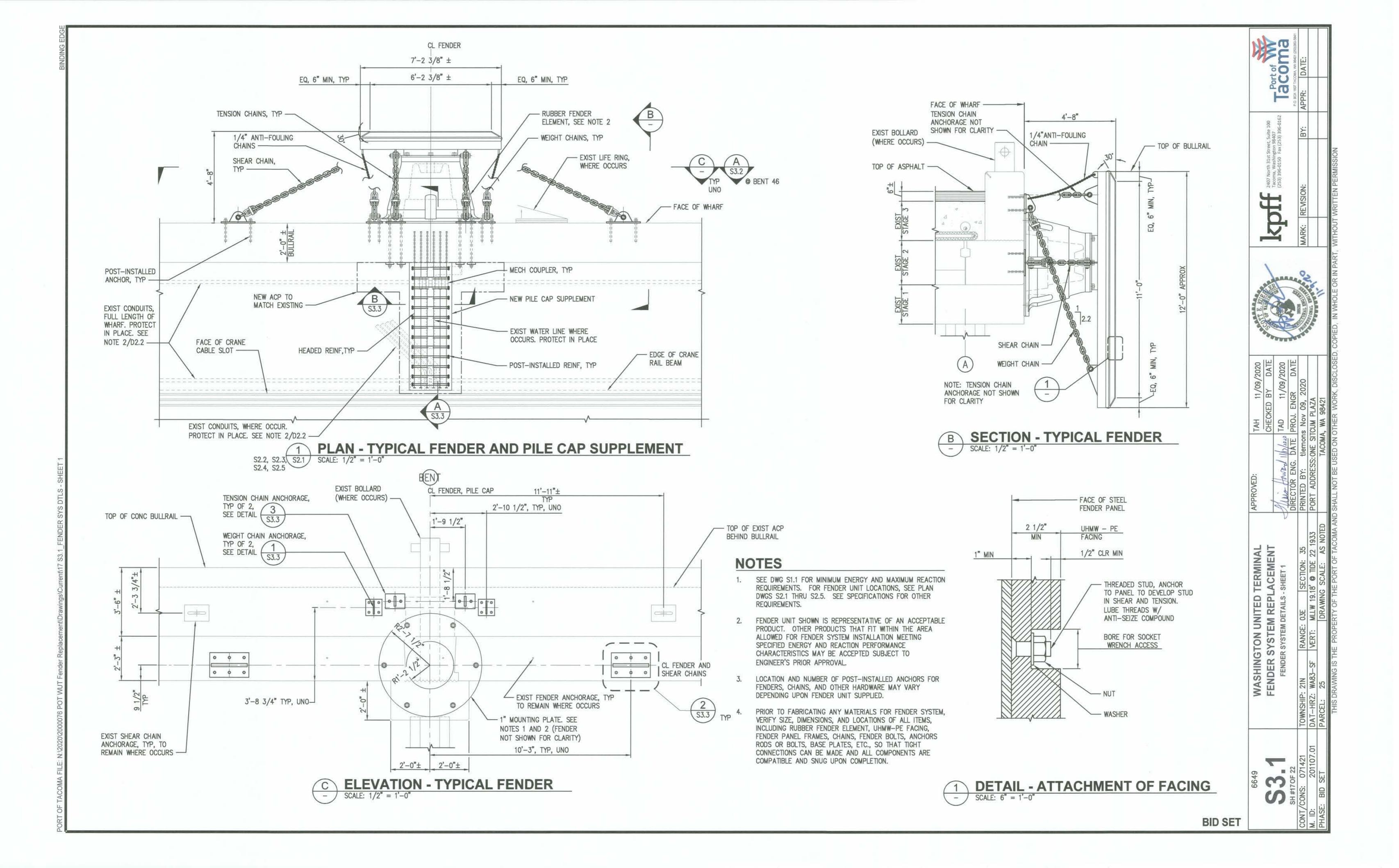


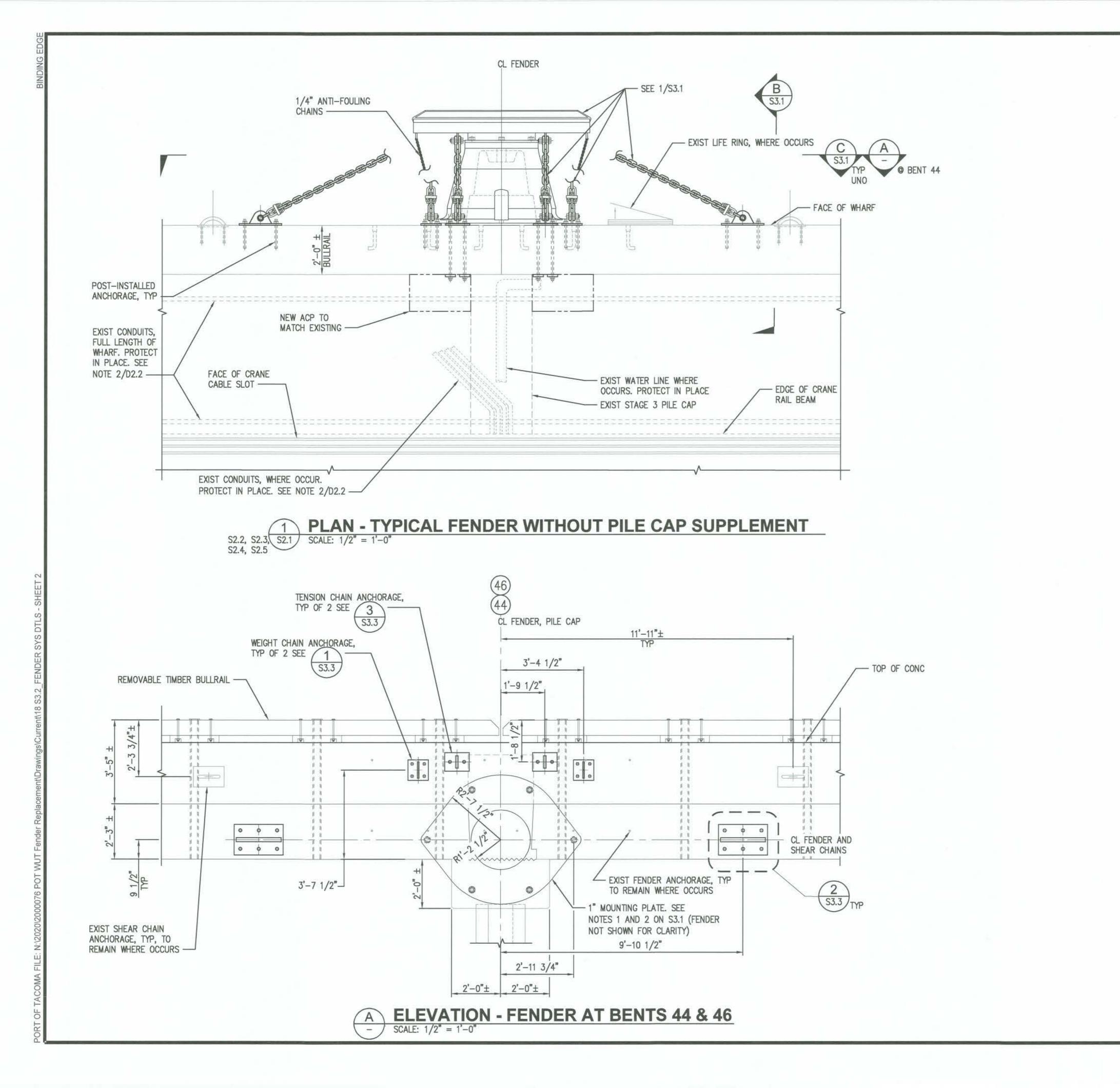






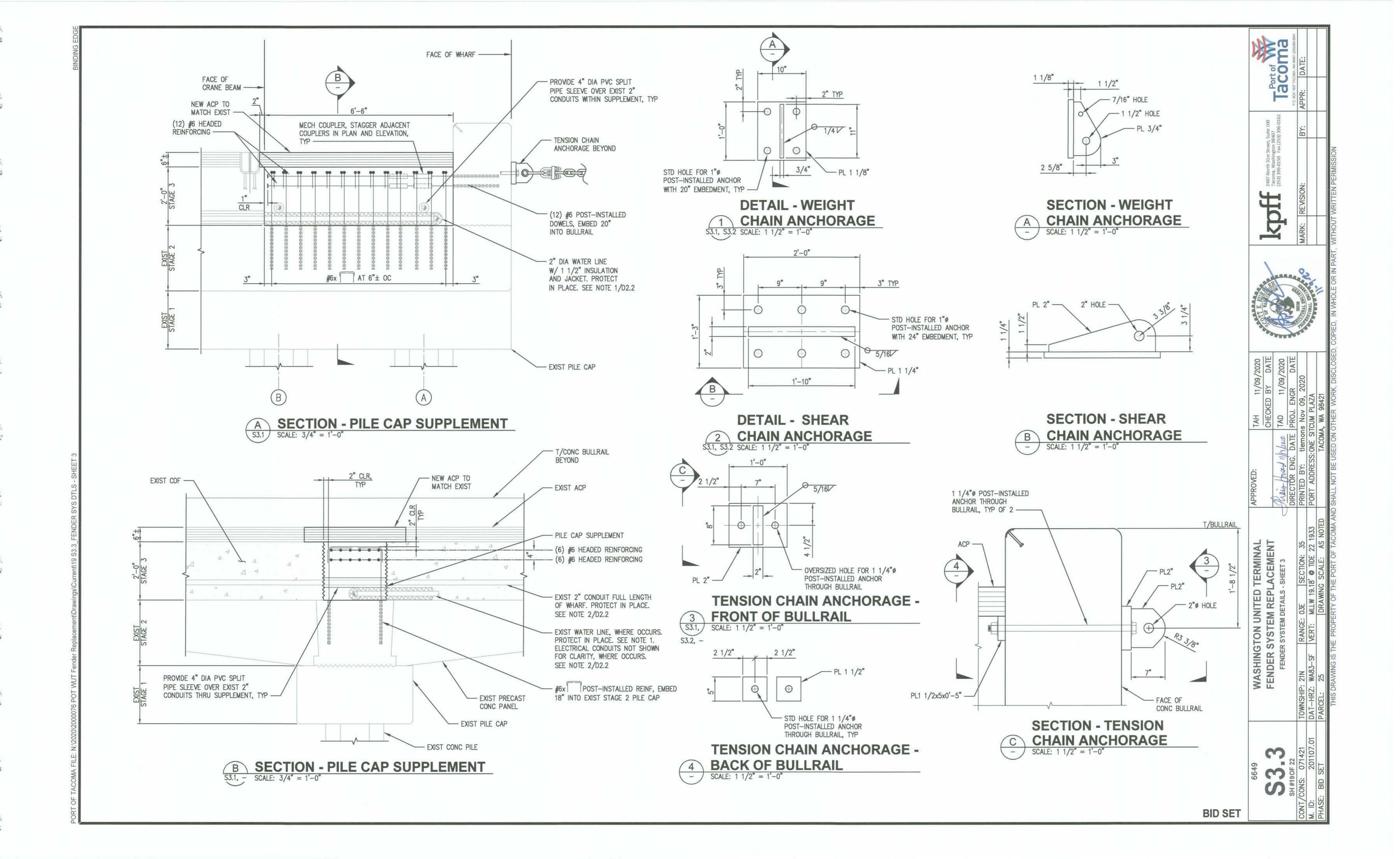


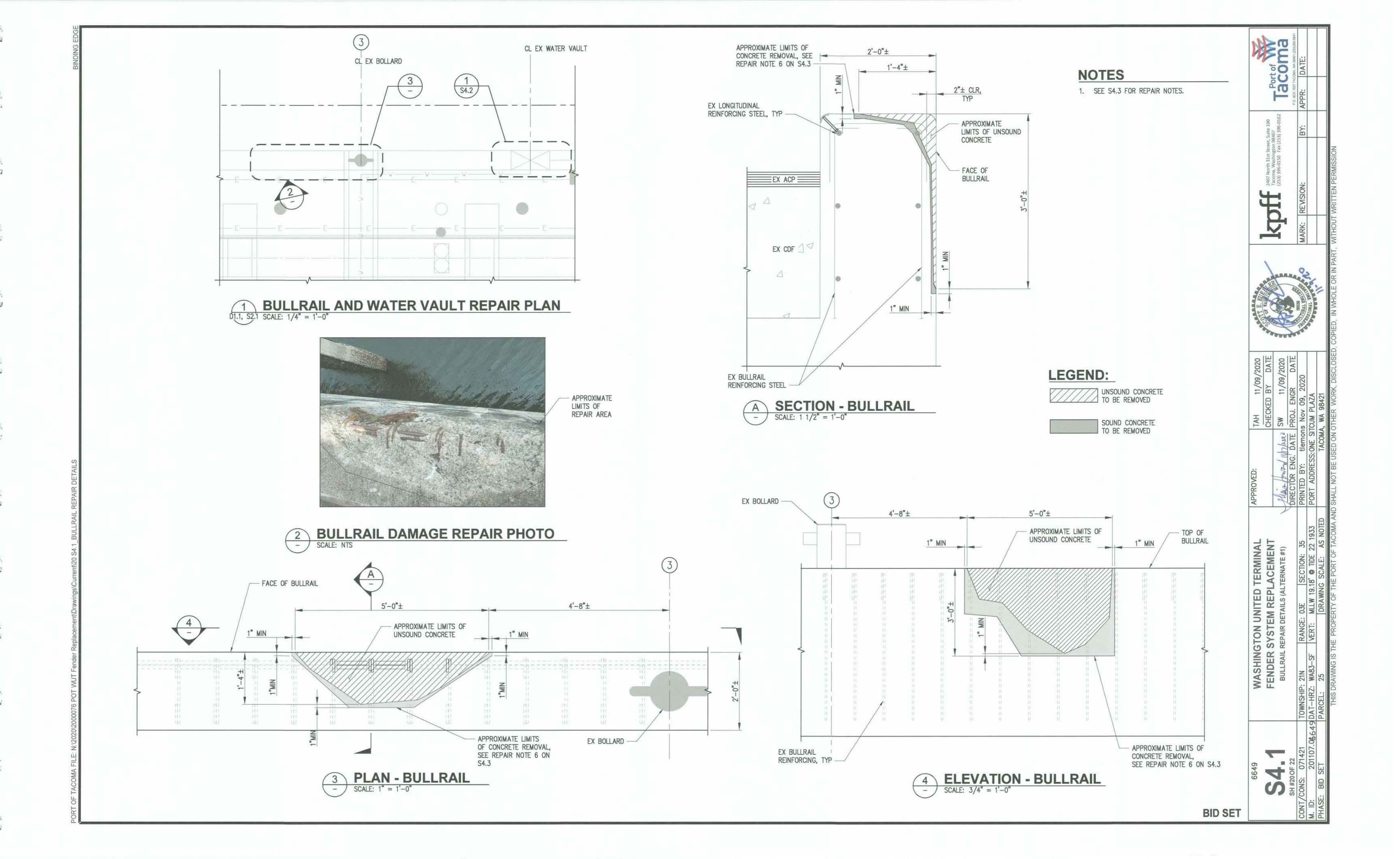


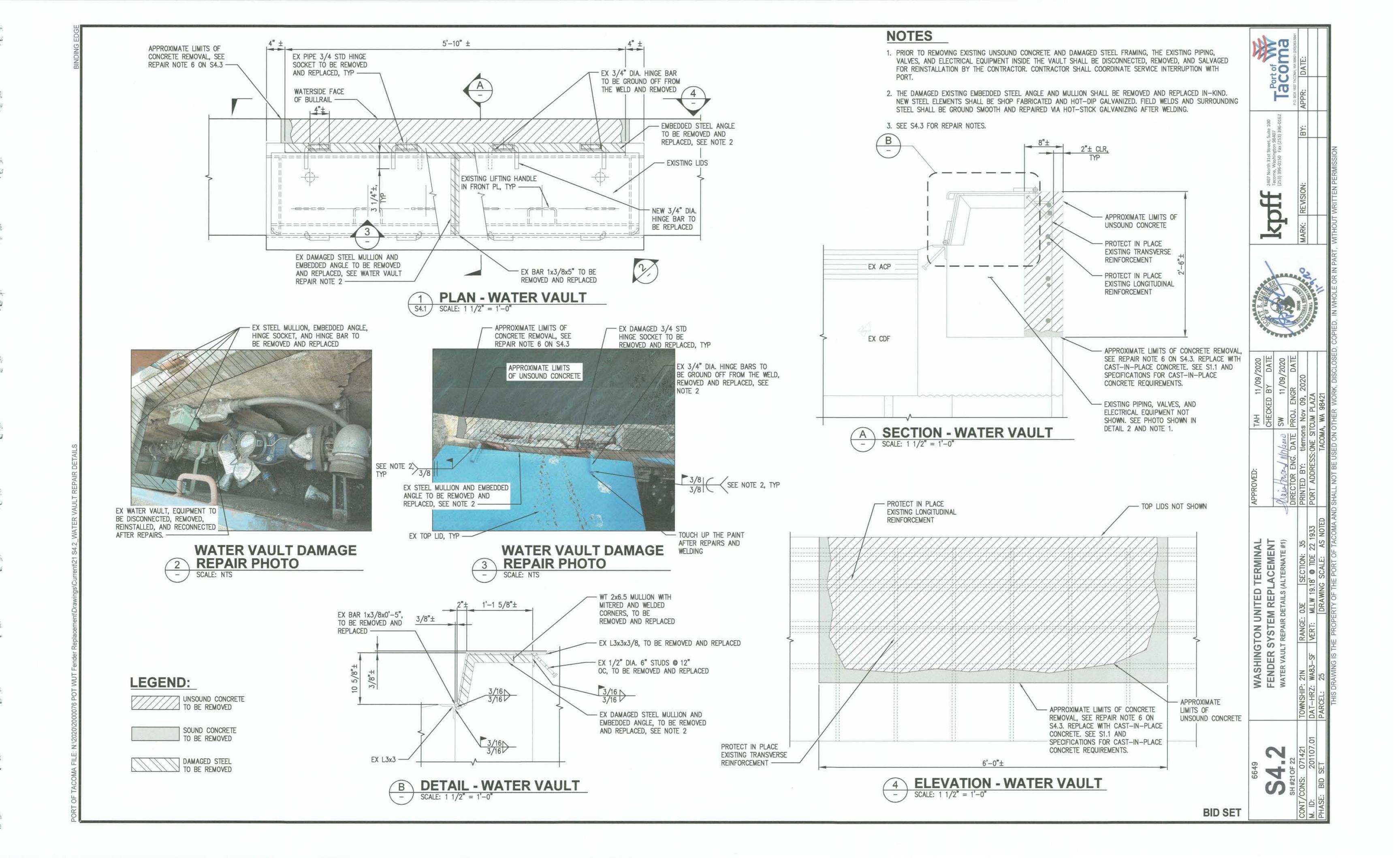


WASHINGTON UNITED TERMINAL FENDER SYSTEM REPLACEMENT FENDER SYSTEM DETAILS - SHEET 2

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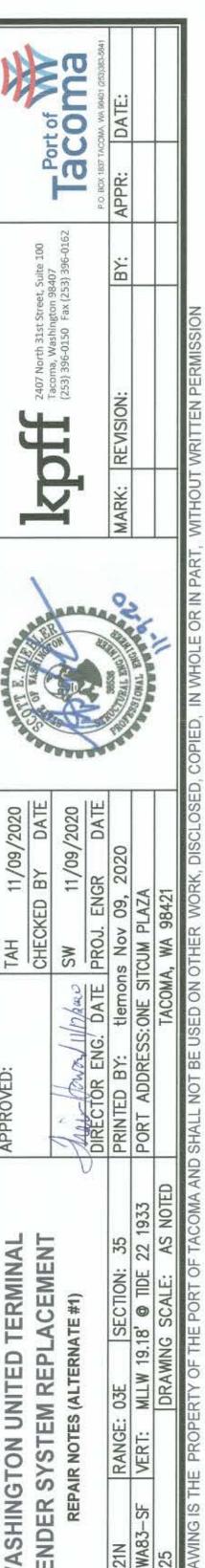




REPAIR NOTES

- CONTRACTOR SHALL PROVIDE APPROPRIATE CATCHMENT DEVICE(S) DURING REPAIR WORK TO PREVENT CONCRETE CHIPS, WASH WATER AND OTHER DEBRIS FROM FALLING INTO THE WATER.
- CONCRETE REPAIRS SHALL BE PERFORMED IN GENERAL CONFORMANCE WITH ACI FIELD GUIDE TO CONCRETE REPAIR APPLICATION PROCEDURES "SURFACE REPAIR USING FORM-AND-POUR TECHNIQUES" (ACI RAP-4), EXCEPT AS MODIFIED
- LOCATE EACH DESIGNATED REPAIR ON THE STRUCTURE AND DETERMINE THE ACTUAL REPAIR PERIMETER BY HAMMER SOUNDING TO DETERMINE IF UNSEEN SUBSURFACE DELAMINATIONS EXIST IN ADDITION TO VISUAL SPALLS. FOLLOWING THE HAMMER SOUNDING, THE CONTRACTOR SHALL MARK THE OUTLINE OF THE ACTUAL REPAIR AREA, NOTIFY THE ENGINEER, AND SHALL PROVIDE ACCESS FOR THE ENGINEER'S INSPECTION PRIOR TO PROCEEDING WITH REPAIRS.
- USING A PACHOMETER OR OTHER NON-DESTRUCTIVE TESTING (NDT) METHOD, DETERMINE THE ACTUAL CONCRETE COVER OVER THE REINFORCING IN THE ESTIMATED REPAIR AREA.
- THE EDGES OF THE REPAIR BOUNDARY SHALL EXTEND AT LEAST 1" BEYOND THE EXTENTS OF SPALLS, DELAMINATIONS, OR OTHERWISE UNSOUND CONCRETE. THE EDGES OF THE ACTUAL REPAIR AREA SHALL BE FORMED BY STRAIGHT BOUNDARY LINES FOLLOWING THE GENERAL SHAPE INDICATED IN THE REPAIR DETAIL.
- EDGES OF THE REPAIR AREA BOUNDARY SHALL BE SAWCUT PERPENDICULAR TO THE CONCRETE SURFACE TO A MINIMUM DEPTH 1" OR 1/4" ABOVE THE SHALLOWEST REINFORCEMENT DEPTH, WHICHEVER IS LESS. USE CARE TO PREVENT DAMAGE TO REINFORCING STEEL. DAMAGE TO REINFORCING STEEL SHALL BE REPAIRED AT NO ADDITIONAL COST TO THE PORT.
- CHIP AND REMOVE SPALLED, DELAMINATED, LOOSE OR OTHERWISE UNSOUND CONCRETE TO SOUND SUBSTRATE WITHIN REPAIR AREA. DO NOT FEATHER THE EDGES OF THE REPAIRS.
- PERFORM INITIAL CONCRETE REMOVAL WITH EITHER 15-LB OR 30-LB JACKHAMMERS. JACKHAMMERS LARGER THAN 30-LBS MAY CAUSE DAMAGE TO REINFORCEMENT, REINFORCEMENT BOND TO SURROUNDING CONCRETE, AND REMAINING CONCRETE, AND SHALL NOT BE USED WITHIN 1" OF EXISTING REINFORCING STEEL. USE 15-LB OR SMALLER JACKHAMMERS FOR FINAL REMOVAL AND DETAILING AROUND THE REINFORCING STEEL
- REMOVE ADDITIONAL CONCRETE TO PROVIDE 1" MIN CLEAR AROUND REINFORCING BARS AT LOCATIONS WHERE HALF OR MORE OF A BAR'S DIAMETER IS EXPOSED.
- 10. CHECK SURFACES TO ENSURE THAT THEY ARE FREE FROM LOOSE AGGREGATE OR ADDITIONAL DELAMINATIONS.
- 11. PREPARE REPAIR CAVITY BY REMOVING ALL CONCRETE FROM WITHIN THE REPAIR AREA BOUNDARY TO THE DEPTH OF THE SAWCUTS.
- 12. AT ALL INTERFACES BETWEEN EXISTING AND NEW CONCRETE, ROUGHEN EXISTING EXPOSED CONCRETE SURFACES TO A 1/4" MINIMUM AMPLITUDE BY LIGHT SCABBLING OR ABRASIVE BLASTING.
- 13. PREPARE EXPOSED CONCRETE, AND REINFORCEMENT FOR PLACING REPAIR MATERIAL. DO NOT CUT, BEND, OR OTHERWISE DAMAGE EXISTING REINFORCING. ALL EXPOSED CONCRETE AND REINFORCING SHALL BE CLEANED OF ALL SCALE. RUST, DIRT, OIL, OR ANY OTHER DELETERIOUS MATERIAL USING MECHANICAL ABRASION.
- 14. PROVIDE NOTIFICATION TO PORT AND PROVIDE ACCESS TO ENGINEER FOR INSPECTION OF REPAIR AREAS PRIOR TO PLACING REPAIR MATERIALS.

- 15. ERECT FORMWORK AROUND REPAIR AREA. FORMWORK AND FALSEWORK MAY BE HUNG FROM BULLRAIL
- 16. CHAMFER ALL CAST-IN-PLACE EXPOSED CORNERS TO MATCH EXISTING CHAMFER SIZES.
- 17. COMPLETED REPAIR SHALL PROVIDE FOR MINIMUM 1 1/2" OF COVER OVER REINFORCING STEEL.
- 18. ENSURE FORMWORK IS RIGID AND WATER TIGHT TO PREVENT LOSS OF MATERIAL. PROPERLY SEAL FACES OF FORMS TO ENSURE SURFACES ABSORB NO WATER FROM REPAIR MATERIAL.
- 19. PROVIDE DRAINAGE OUTLETS IN FORMWORK FOR PRESOAKING. PROVIDE SUITABLE ACCESS POINTS TO POUR OR PUMP MIXED REPAIR MORTAR INTO PLACE.
- 20. USE SUITABLE FORM-RELEASE AGENT TO FACILITATE REMOVAL OF FORMS FROM CAST MATERIAL.
- 21. IN CONDITIONS WHERE FORMWORK CANNOT BE FILLED WITH WATER, SPRAY SUBSTRATE WITH CLEAN WATER TO ACHIEVE SATURATED SURFACE-DRY (SSD) CONDITION IMMEDIATELY BEFORE PLACING REPAIR MORTAR OR CAST-IN-PLACE CONCRETE.
- APPLY BONDING AGENT TO THE EXISTING CONCRETE SURFACE AND EXISTING REINFORCING STEEL. ACCEPTABLE PRODUCT: EUCLID CHEMICAL DURALPREP A.C. BONDING AGENT.
- 23. IMMEDIATELY AFTER MIXING, PUMP OR POUR REPAIR MORTAR OR CAST-IN-PLACE CONCRETE INTO FORMED AREA.
- 24. REPAIR MORTAR FOR BULLRAIL REPAIR SHALL BE A ONE-COMPONENT, LOW DUST, SHRINKAGE-COMPENSATED, SELF-CONSOLIDATING MORTAR CONSISTING OF CEMENT, GRADED AGGREGATE, SHRINKAGE-COMPENSATING AGENTS, AN INTEGRAL CORROSION INHIBITOR, AND ADDITIVES. ACCEPTABLE PRODUCT: MASTEREMACO S 440CI (FORMERLY LA40 PMAC REPAIR MORTAR) BY BASF, OR AN APPROVED EQUAL HAVING SIMILAR PERFORMANCE REQUIREMENTS. ALTERNATE PRODUCTS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO USE. CAST-IN-PLACE CONCRETE SHALL CONFORM TO REQUIREMENTS OF S1.1 AND THE SPECIFICATIONS.
- PLACE REPAIR MORTAR AND CURE PER MANUFACTURER'S INSTRUCTIONS. ENSURE THAT SUBSTRATE SURFACE AND AMBIENT AIR TEMPERATURE ARE MINIMUM OF 40 DEGREES F AND RISING AT APPLICATION TIME AND REMAIN ABOVE 40 DEGREES F FOR AT LEAST 24 HOURS AFTER APPLICATION. ENSURE THAT FROST OR FROZEN SURFACES ARE THAWED AND DRY. CAST-IN-PLACE CONCRETE SHALL BE PLACED AND CURED IN ACCORDANCE WITH THE REQUIREMENTS OF THE SPECIFICATIONS.
- 26. FORMWORK SHALL BE LEFT IN PLACE FOR THE PRESCRIBED CURING PERIOD. AFTER STRIPPING OF FORMWORK, ANY SPACES NOT FILLED SHALL BE TRIMMED. CLEANED, AND DRY-PACKED. TWO COATS OF A MEMBRANE CURING COMPOUND SHALL BE APPLIED IMMEDIATELY AFTER REMOVAL OF FORMWORK.



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