PORT OF TACOMA

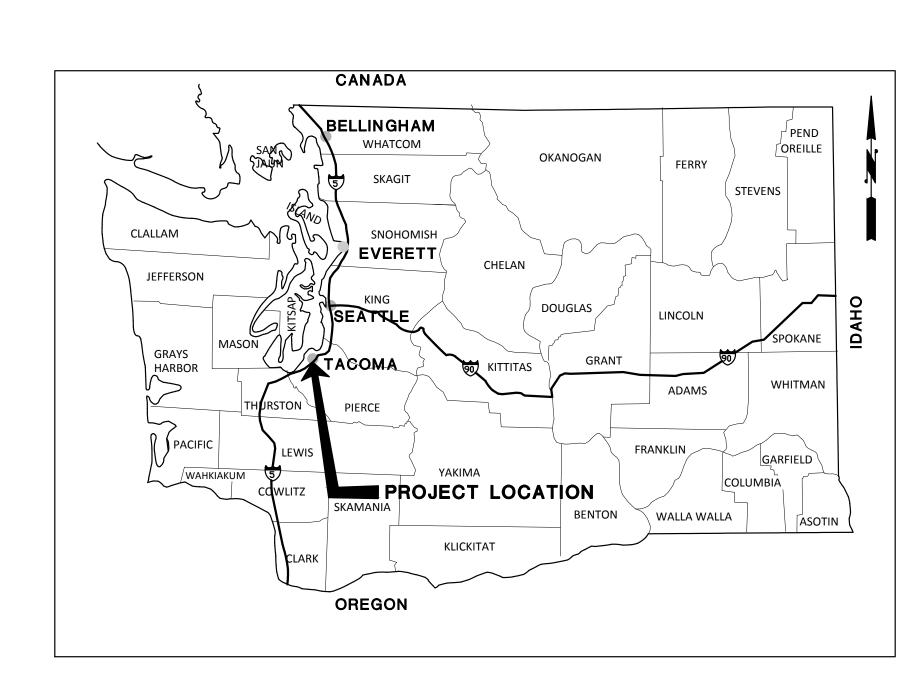
TACOMA, WASHINGTON PIER 3 UPGRADE CONTRACT NO. 069458

PORT COMMISSIONERS:

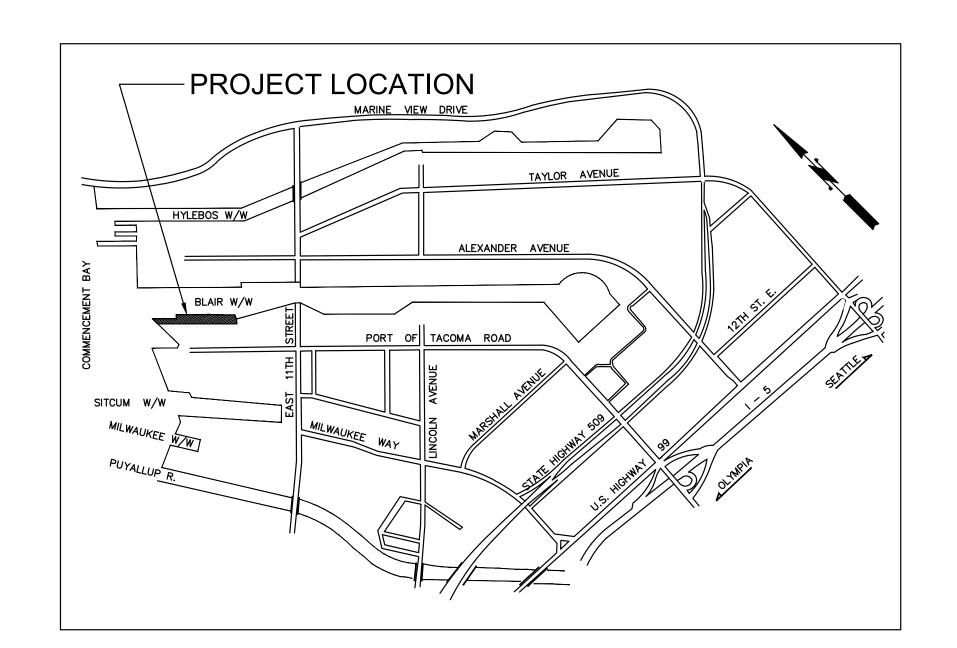
CONSTANCE T. BACON DON JOHNSON RICHARD P. MARZANO **CLARE PETRICH DON MEYER**

PORT STAFF:

JOHN WOLFE Chief Executive Officer SUE MAUERMANN Chief Facilities Development Officer DAKOTA CHAMBERLAIN Director of Engineering TREVOR THORNSLEY Senior Project Manager



AREA MAP WASHINGTON NO SCALE



NO SCALE

VICINITY MAP

PORT OF TACOMA

CONSULTANTS:

(206) 431-2300 Fax: (206) 431-2250







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61	\$8.3	STAGE I - DETAILS, BULKHEAD SUPPLEMENT, & SUBCAPS	127	E7.0	CONDUIT AND CONDUCTOR SCHEDULE
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63	S9.1	STAGE II - PILE CAPS - SHEET 1	129	E7.2	CONDUIT AND CONDUCTOR SCHEDULE
64	S9.2	STAGE II - PILE CAPS - SHEET 2			

N XXXXX.XX

E XXXXX.XX

(xx)

NORTH ARROW EXIST HIGH MAST LIGHTPOLE SECTION, DETAIL, OR ELEVATION CALLOUT XX.X DRAWING WHERE SECTION, DETAIL OR **ELEVATION IS SHOWN OR CALLED FROM** SECTION OR ELEVATION CUT X TITLE XX.X SCALE: X' = X'-XX" PLAN, SECTION, DETAIL, OR ELEVATION WATER **EXIST WATER** FIRE WATER EXIST FIRE WATER SANITARY SEWER **EXIST SANITARY SEWER** NEW/TEMPORARY SECURITY FENCE **EXIST SECURITY FENCE** STORM DRAIN EXIST STORM DRAIN ELECTRICAL EXIST ELECTRICAL **EXIST ELECTRICAL CONDUIT** EXIST CRANE RAIL GROUNDING — · · · — E(G) — · · · —

COORDINATES

WORK POINT

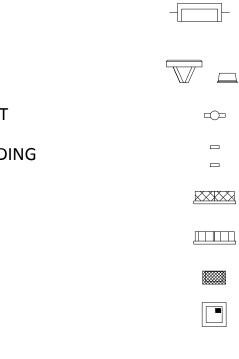
BENT OR GRID NO.

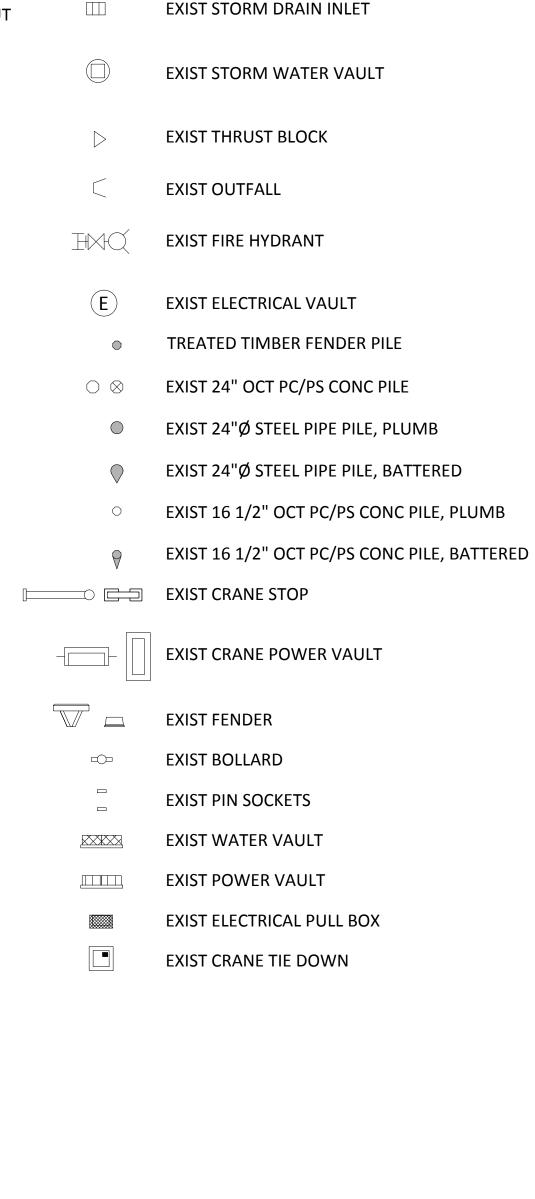
SURVEY CONTROL MONUMENT,

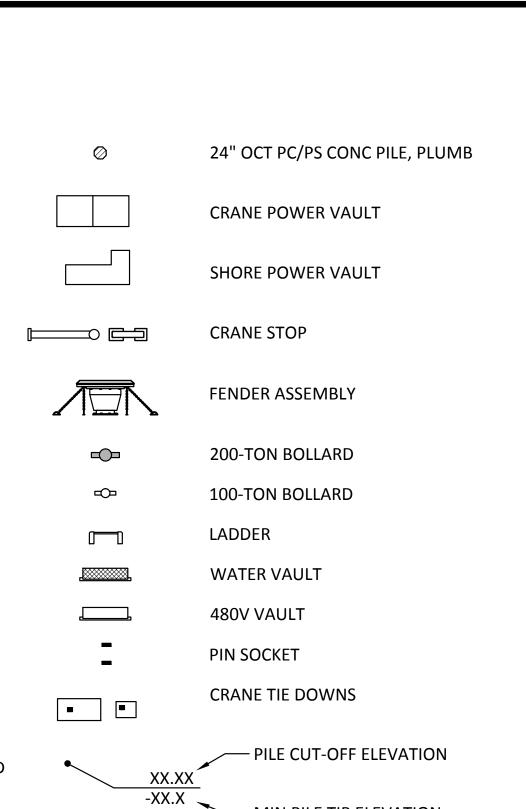
SURVEY CONTROL POINT

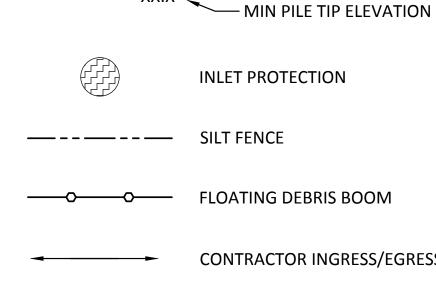
PROJECT CONTROL POINT

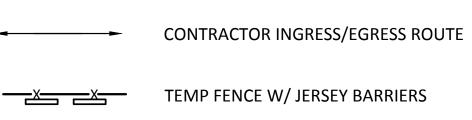
PROJECT CONTROL POINT













DEMOLITION LIMITS

HIGH MAST LIGHT POLE





EPOXY COATED REINFROCEMENT

ABBREVIATIONS:

MAXIMUM

MANHOLE

MINIMUM

MECHANICAL JOINT

MEAN SEA LEVEL

MECHANICAL

MEAN HIGHER HIGH WATER

MEAN LOWER LOW WATER

MECH

MHHW

MLLW

MH

MIN MJ

&	AND	N	NORTH
±	APPROXIMATELY	NAD	NORTH AMERICAN DATUM
<u>-</u> @	AT	NAVD	NORTH AMERICAN VERTICAL DATUM
Ę.		NCD	NEARSHORE CONFINED DISPOSAL
	CENTERLINE	NIC	NOT IN CONTRACT
Ø	DIAMETER	NIM	NORTH INTERMODAL
· ·	DEGREES	NGVD	NATIONAL GEODETIC VERTICAL DATUM
=	EQUALS	NOM	NOMINAL
	INCHES, SECONDS	NO	NUMBER
·	FEET, MINUTES	NSF	NATIONAL SANITATION FOUNDATION
۸۲۱	AMERICAN CONCRETE INSTITUTE	NW	NORTHWEST
ACI ACP	ASPHALT CONCRETE PAVEMENT		
ACP	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	OC	ON CENTER
AMP	AMPLITUDE	OCT	OCTAGONAL
		OD	OUTSIDE DIAMETER
APPROX	APPROXIMAT (-E, -LY)	OHW	ORDINARY HIGH WATER
APWA	AMERICAN SOCIETY FOR TESTING AND MATERIALS	OPP	OPPOSITE
ASTM	AMERICAN MELDING SOCIETY		
AWS	AMERICAN WELDING SOCIETY	Р	POINT LOAD
AWWA	AMERICAN WATER WORKS ASSOCIATION	P/C, PC	PRECAST
DNAD		PC/PS	PRECAST/PRESTRESSED
BMP	BEST MANAGEMENT PRACTICE	PDA	PILE DRIVING ANALYZER
ВОТ	ВОТТОМ	PERP	PERPENDICULAR
CDC	CALIFORNIA DI III DINIC CODE	PL	PLATE
CBC	CALIFORNIA BUILDING CODE	PS	PRESTRESSED
CIP	CAST IN PLACE	PSF	POUNDS PER SQUARE FEET
CJ	CONSTRUCTION JOINT	PSI	POUNDS PER SQUARE INCH
CJP	COMPETE JOINT PENETRATION	PT	POINT
CLR	CLEAR (-ANCE)	PVC	POLY VINYL CHLORIDE
CONC	CONCRETE	1 4 6	TOET VIIVE CHECKIDE
CONT	CONTINUOUS	R, RAD	RADIUS
COMM	COMMUNICATION(S)	R/C	REINFORCED CONCRETE
		REINF	REINFORC (-E,-ED,-ING,-MENT)
DEMO	DEMOLISH OR DEMOLITION	REQ'D	REQUIRED
DIA	DIAMETER	RPM	RADIATION PORTAL MONITOR
DIP	DUCTILE IRON PIPE	NPIVI	RADIATION PORTAL WIGHTOR
DWG(S)	DRAWING (-S)	S	SECONDS SOUTH
		SCHED	SECONDS, SOUTH SCHEDULE
EA	EACH		
EHW	EXTREME HIGH WATER	SD	STORM DRAIN
EL, ELEV	ELEVATION	SE	SOUTHEAST
ELW	EXTREME LOW WATER	SHT	SHEET
EMBED	EMBEDMENT	SIM	SIMILAR
EQ	EQUAL (-LY)	SPA	SPACE
EW	EACH WAY	SPECS	SPECIFICATIONS
EXIST, EX	EXISTING	SQ	SQUARE
EXP JT	EXPANSION JOINT	SS	SANITARY SEWER/STAINLESS STEEL
		SSP	SHIP SHORE POWER
FB	FLAT BAR	STIFF	STIFFENER
f'c	COMPRESSIVE STRENGTH	SWPPP	STORMWATER POLLUTION PREVENTION PLAN
FT	FEET, FOOT	SYMM	SYMMETRICAL
FW	FIRE WATER	70 5	TOD 0 DOTTOM
fy	YIELD STRENGTH	T&B	TOP & BOTTOM
,		TEMP	TEMPORARY
GALV	GALVANIZE (-D)	TESC	TEMPORARY EROSION AND SEDIMENT CONTROL
GPS	GLOBAL POSITIONING SYSTEM	THRU	THROUGH
GR	GRADE	TMC	TACOMA MUNICIPAL CODE
		TOPO	TOPOGRAPHY
HAS	HEADED ANCHOR STUD (-S)	TPU	TACOMA PUBLIC UTILITIES
HDPE	HIGH DENSITY POLYETHYLENE	TWIC	TRANSPORTATION WORKER IDENTIFICATION CREDENTIAL
HMA	HOT MIX ASPHALT	TYP	TYPICAL
HSS	HOLLOW STRUCTURAL SECTION		
HORIZ	HORIZONTAL	•	ULTRA HIGH MOLECULAR WEIGHT POLYETHYLENE
	TIOTALE STATE	UNC	UNIFIED NATIONAL COARSE
IBC	INTERNATIONAL BUILDING CODE	UNO	UNLESS NOTED OTHERWISE
IE	INVERT ELEVATION		
IEBC	INTERNATIONAL EXISTING BUILDING CODE	V	VOLT
IN.	INCH (-ES)		
IIV.	interr (E3)	W/	WITH
JT	JOINT	W/O	WITHOUT
J 1		WSDOT	WASHINGTON STATE DEPARTMENT
KIP,KIPS	KILOPOUND(-S)		OF TRANSPORTATION
NIT, NITS	KILOF GGIND(-3)		
LB, LBS	POLIND(-S)		
LB, LBS LOC	POUND(-S) LOCATION		
LF	LOAD FACTOR		
LF LLH	LONG LEG HORIZONTAL		
LLN	LONG LEG HORIZONTAL LONG LEG VERTICAL		
LLV LT	LONG-TON		
LI	LOING-TOIN		

BergerABAM
33301 9th Avenue South, Suite 300
Federal Way, Washington 98003-2600
(206) 431-2300 Fax: (206) 431-2250

Tacoma Tacoma

UPGRAD

3

PIER

NOTES:

GENERAL

- THESE NOTES CONTAIN GENERAL INFORMATION AND ARE NOT NECESSARILY COMPLETE FOR CONSTRUCTION PURPOSES. THE CONTRACTOR SHALL VERIFY INFORMATION SHOWN ON THE DRAWINGS, IN THE SPECIFICATIONS AND OTHER DOCUMENTS, AND BRING ANY CONFLICTS TO THE ATTENTION OF THE ENGINEER BEFORE BEGINNING THE AFFECTED WORK. THE ENGINEER WILL RESOLVE ANY SUCH CONFLICTS. SEE THE SPECIFICATIONS FOR AVAILABLE RECORD DRAWINGS AND REFERENCE DOCUMENTS.
- 2. THE ADJACENT TERMINAL (PIER 4) WILL BE IN OPERATION DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL KEEP ITS WORKERS, MATERIAL, AND EQUIPMENT CLEAR OF ALL SHIPPING AND CONTAINER HANDLING OPERATIONS AND SHALL NOT IN ANY WAY HINDER OR DISRUPT TERMINAL OPERATIONS.

THE PROJECT SITE IS LOCATED ALONG AN ACTIVE SHIPPING CHANNEL. AT NO TIME SHALL THE CONTRACTOR OBSTRUCT OR HINDER SHIPPING ACTIVITIES WITHIN THE CHANNEL. ALL WATER BORNE CONSTRUCTION EQUIPMENT SHALL BE MOORED OUT OF THE SHIPPING CHANNEL. NO MOORING LINES OR ANCHORS SHALL BE PLACED WITHIN THE CHANNEL.

- PROJECT DATUM AND SURVEY CONTROL: REFER TO DWG C1.0 FOR ESTABLISHED PROJECT CONTROL POINTS. THE CONTRACTOR SHALL PERFORM ALL SURVEYING AND STAKING REQUIRED DURING CONSTRUCTION, SEE SPECIFICATIONS ALSO.
- 4. LOCATIONS OF EXISTING UTILITIES SHOWN HEREIN HAVE BEEN ESTABLISHED BY FIELD SURVEY OR OBTAINED FROM AVAILABLE RECORDS AND SHOULD BE CONSIDERED APPROXIMATE ONLY AND NOT NECESSARILY COMPLETE. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO INDEPENDENTLY VERIFY THE ACCURACY OF ALL UTILITY LOCATIONS SHOWN, AND TO FURTHER DISCOVER AND AVOID ANY OTHER UTILITIES NOT SHOWN HEREON WHICH MAY BE AFFECTED BY THE IMPLEMENTATION OF THESE PLANS. SEE THE SPECIFICATIONS FOR AVAILABLE RECORD DRAWINGS AND REFERENCE DOCUMENTS.
- 5. THE CONTRACTOR SHALL USE A UTILITY LOCATION SERVICE FAMILIAR WITH THE SITE. DOCUMENTATION OF ALL LOCATES SHALL BE REVIEWED AND APPROVED BY THE ENGINEER PRIOR TO THE COMMENCEMENT OF ANY EXCAVATION. INFORM THE ENGINEER OF ANY POTENTIAL CONFLICTS OR INTERFERENCES FOR RESOLUTION PRIOR TO PERFORMING EXCAVATIONS.
- 6. THE CONTRACTOR SHALL PROVIDE AND MAINTAIN TEMPORARY SEDIMENTATION AND EROSION CONTROL (TESC) FACILITIES TO ENSURE THAT SEDIMENT-LADEN WATER DOES NOT ENTER THE NATURAL OR PIPED DRAINAGE SYSTEM. REFER TO THE APPLICABLE DRAWINGS AND THE SPECIFICATIONS FOR PLANS AND DETAILS THAT DESCRIBE THE MINIMUM TESC FEATURES REQUIRED.

AS CONSTRUCTION PROGRESSES AND SEASONAL CONDITIONS DICTATE, THE CONTRACTOR SHALL REVISE TESC FACILITIES AND CONFIGURATIONS AS NECESSARY TO ENSURE COMPLETE SILTATION CONTROL AND THAT NO SEDIMENT LADEN WATER ENTERS THE NATURAL OR PIPED DRAINAGE SYSTEM.

DURING THE COURSE OF CONSTRUCTION, IT SHALL BE THE OBLIGATION AND RESPONSIBILITY OF THE CONTRACTOR TO ADDRESS ANY NEW CONDITIONS THAT MAY BE CREATED BY ITS ACTIVITIES AND TO PROVIDE ADDITIONAL TESC FACILITIES THAT MAY BE NEEDED TO PROTECT THE SITE, ADJACENT LAND AND EXISTING DRAINAGE FEATURES. REMOVE TESC FACILITIES AT COMPLETION OF THE PROJECT.

- 7. THE CONTRACTOR SHALL ENSURE NO DEMOLITION, CONSTRUCTION, OR OTHER DEBRIS FALLS TO THE SLOPE OR IN THE WATER. THE CONTRACTOR SHALL IMMEDIATELY REMOVE ALL DEBRIS THAT INADVERTENTLY FALLS ON THE SLOPE OR IN THE WATER ON AN ONGOING BASIS. SEE ADDITIONAL NOTES UNDER DEMOLITION AND THE SPECIFICATIONS.
- PIER BENT NUMBERING SYSTEM SHALL BE AS SHOWN ON DWGS S3.0 TO S3.4. BENT CENTERLINES SHALL BE DEFINED BY THE CENTER OF THE EXIST STAGE I PILECAP AT FACE OF PIER

GENERAL CONT.

- 8. THE CONTRACTOR SHALL SUPPLY, INSTALL, AND MAINTAIN AT ALL TIMES FLOATING DEBRIS BOOMS AROUND ALL ACTIVE WORK AREAS OVER THE SLOPE AND IN OR OVER THE WATER.
- 9. THE CONTRACTOR SHALL KEEP ALL ON-SITE PROJECT AREAS CLEAN AT ALL TIMES BY SWEEPING. WASHING AND/OR USE OF A WATER TRUCK TO CLEAN PAVED AREAS ARE/IS NOT ALLOWED.
- 10. ALL EXISTING FEATURES AND PAVED SURFACES, INCLUDING TERMINAL YARDS, ROADWAYS, SIDEWALKS, FENCES, AND CURBS DESIGNATED TO REMAIN THAT ARE DAMAGED, AS DETERMINED BY THE ENGINEER, DURING CONSTRUCTION, SHALL BE REPAIRED BY THE CONTRACTOR AT ITS OWN
- 11. THE CONTRACTOR SHALL DISPOSE OF EXCESS OR UNSUITABLE MATERIALS AS INDICATED IN THE SPECIFICATIONS. SEE SPECIFICATIONS FOR ITEMS TO BE SALVAGED.
- 12. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS ASSOCIATED WITH NOTIFICATIONS OF UTILITY OUTAGES. THE CONTRACTOR SHALL COORDINATE AND ARRANGE FOR ALL UTILITY CONNECTIONS, UTILITY RELOCATIONS AND/OR SERVICE INTERRUPTIONS WITH ENGINEER, APPROPRIATE UTILITY OWNER AND WITH THE CITY OF TACOMA. CONNECTIONS TO EXISTING PUBLIC UTILITIES SHALL BE MADE PER CITY OR TACOMA POWER REQUIREMENTS. EXISTING UTILITY LINES IN SERVICE WHICH ARE DAMAGED DUE TO CONSTRUCTION WORK SHALL BE REPAIRED AT CONTRACTORS'S EXPENSE AND INSPECTED AND ACCEPTED BY UTILITY OWNER'S REPRESENTATIVE PRIOR TO BACKFILLING.
- 13. CONTRACTOR AREAS FOR STAGING, LAYDOWN, STORAGE OF MATERIALS AND EQUIPMENT SHALL BE CONFINED TO THE PROJECT SITE, AS SHOWN ON DWG G5.0. REFER TO SECTION 01 14 00 WORK RESTRICTIONS IN THE SPECIFICATIONS FOR ADDITIONAL SITE ACCESS INFORMATION. AT NO TIME SHALL CONTRACTOR EQUIPMENT, MATERIAL OR PERSONNEL ENTER THE SLIP 5 MITIGATION AREA.
- 14. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR DETERMINING THE ADEQUACY OF EXISTING STRUCTURES AND UTILITIES TO SUPPORT CONSTRUCTION EQUIPMENT AND LOADS.
- 15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR AND SHALL INSTALL AND MAINTAIN SHORING AND BRACING NECESSARY TO PROTECT WORKERS, UTILITIES AND OTHER IMPROVEMENTS AND EXCAVATIONS AGAINST LOSS OF GROUND, ROCKS, RIPRAP, GRAVELS, OR CAVING EMBANKMENTS. THE CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR REMOVAL OF TEMPORARY SHORING AND BRACING.

EXISTING CONCRETE PILES THAT ARE TO BE REINCORPORATED INTO STRUCTURE SHALL BE LATERALLY BRACED PRIOR TO SAW CUTTING AND RELEASE AT TOP OF PILE. BRACING SHALL REMAIN IN PLACE UNTIL STAGE II CONCRETE ABOVE PILE HAS CURED FOR 14 DAYS.

- 16. A GEOTECHNICAL REPORT PREPARED BY GEOENGINEERS, DATED JANUARY 25, 2013, IS INCLUDED IN THE SPECIFICATIONS. THE CONTRACTOR SHALL READ AND BE THOROUGHLY FAMILIAR WITH THE CONTENTS THEREOF.
- 17. ALL DEVIATIONS FROM THESE PLANS SHALL BE RECORDED ON A SET OF "AS-BUILT" OR RECORD DRAWINGS. THE CONTRACTOR SHALL SUBMIT "AS-BUILT" DRAWINGS TO THE ENGINEER IN ACCORDANCE WITH THE SPECIFICATIONS, SEE SECTION 01 70 00, EXECUTION AND CLOSEOUT REQUIREMENTS.

CIVIL

- 1. THE WASHINGTON STATE 2012 STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION, AS PREPARED BY THE WASHINGTON STATE DEPARTMENT OF TRANSPORTATION (WSDOT), SHALL APPLY TO WORK AS IDENTIFIED IN THE SPECIFICATIONS.
- 2. ALL PIPING, FITTINGS & SUPPORTS SHALL CONFORM TO AMERICAN WATER WORKS ASSOCIATION (AWWA) GUIDELINES, AND SHALL BE NSF APPROVED.
- 3. FILL MATERIAL SHALL COMPLY WITH CONTRACT DOCUMENTS AND SHALL BE APPROVED BY THE
- SOILS IN THE SLIP 1 NCD BOUNDARY, AS DEPICTED ON DWG G5.0, MAY BE CONTAMINATED. THE CONTRACTOR SHALL COMPLY WITH APPLICABLE SECTIONS OF THE SPECIFICATIONS DURING DEMOLITION AND EXCAVATION OPERATIONS.
- THERE SHALL BE NO CHANGE IN TOPO FOR THIS PROJECT. TOP OF FINISH GRADE SHALL EQUAL EXISTING GRADE. PAVEMENT PATCH WORK IS DISTURBED AREAS THAT IS REMOVED SHALL LATER BE REPLACED WITH SAME SURFACE ELEVATIONS. PERFORM SURVEYS AS REQ'D TO ENSURE TOPO REMAINS THE SAME. SURFACE IS FLUSH WITH TOP OF CRANE RAILS AND SLOPES DOWN TO DECK DRAINS THAT CREATE A VALLEY BETWEEN THE RAILS ALONG THE ALIGNMENT OF THE DRAINS. PAVEMENT DEMO ON THE PIER AND ADJACENT AREAS IS SHOWN ON THE STRUCTURAL DRAWINGS. LANDSIDE PAVEMENT DEMO FOR ELECTRICAL WORK IS NOT SHOWN. REFER TO TRENCH CROSS-SECTIONS.
- PAVEMENT SECTIONS TO BE USED FOR ALL LANDSIDE REPLACEMENT PAVEMENT (AREAS WEST OF THE BULKHEAD STRUCTURE) SHALL BE 10 INCHES OF COMPACTED HMA OVER 12 INCHES OF COMPACTED BASE COURSE AGGREGATE OVER COMPACTED SELECT BACKFILL MATERIAL.
- 7. PAVEMENT SECTION TO BE USED FOR ALL PIER REPLACEMENT PAVEMENT SHALL BE 6 INCHES OF COMPACTED HMA OVER COMPACTED BALLAST (BASE COURSE AGGREGATE). ONLY BALLAST MATERIAL SHALL BE USED BETWEEN TOP OF CONCRETE AND HMA PAVEMENT UNLESS OTHERWISE SHOWN ON DRAWINGS.

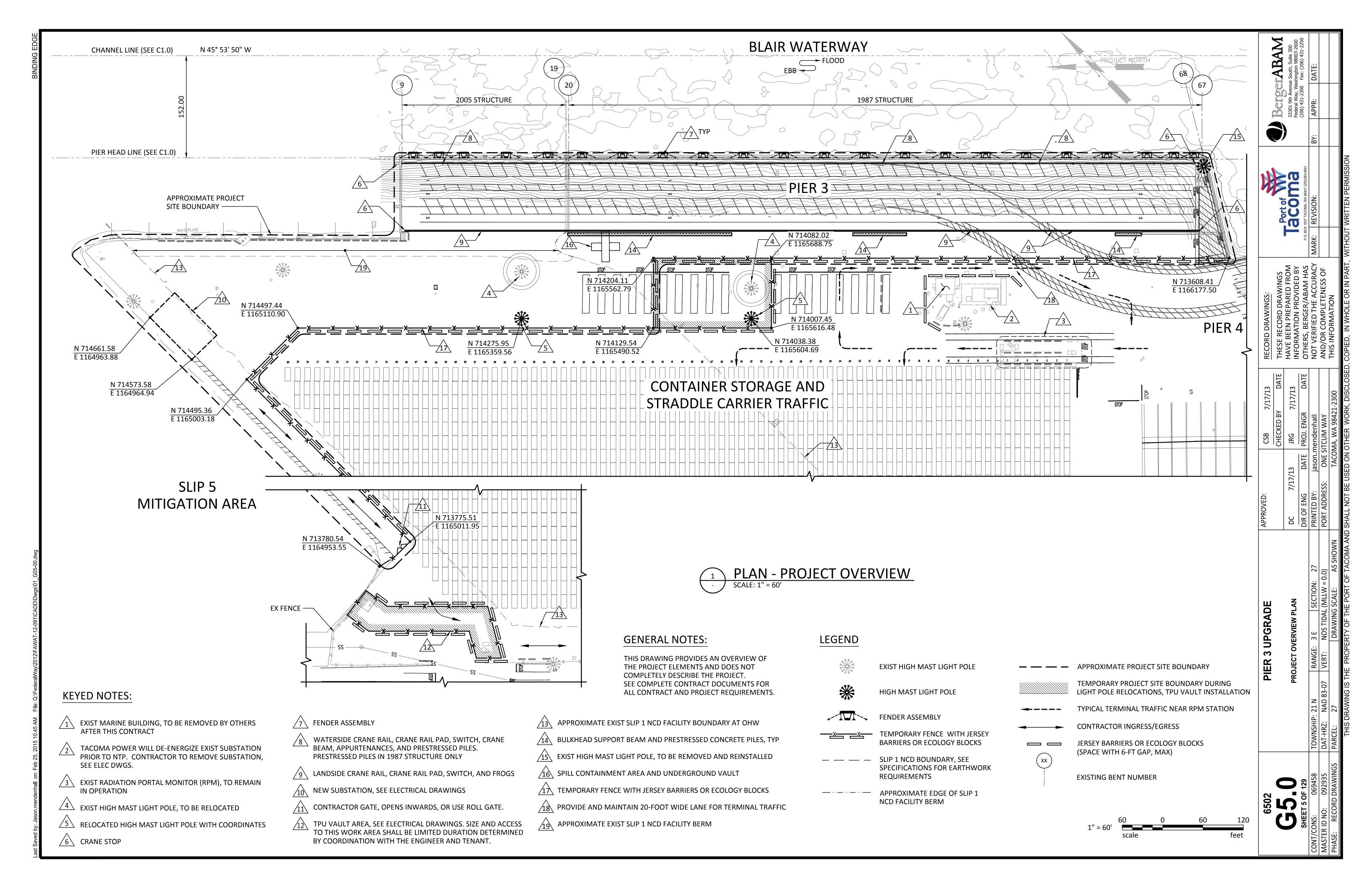
DEMOLITION

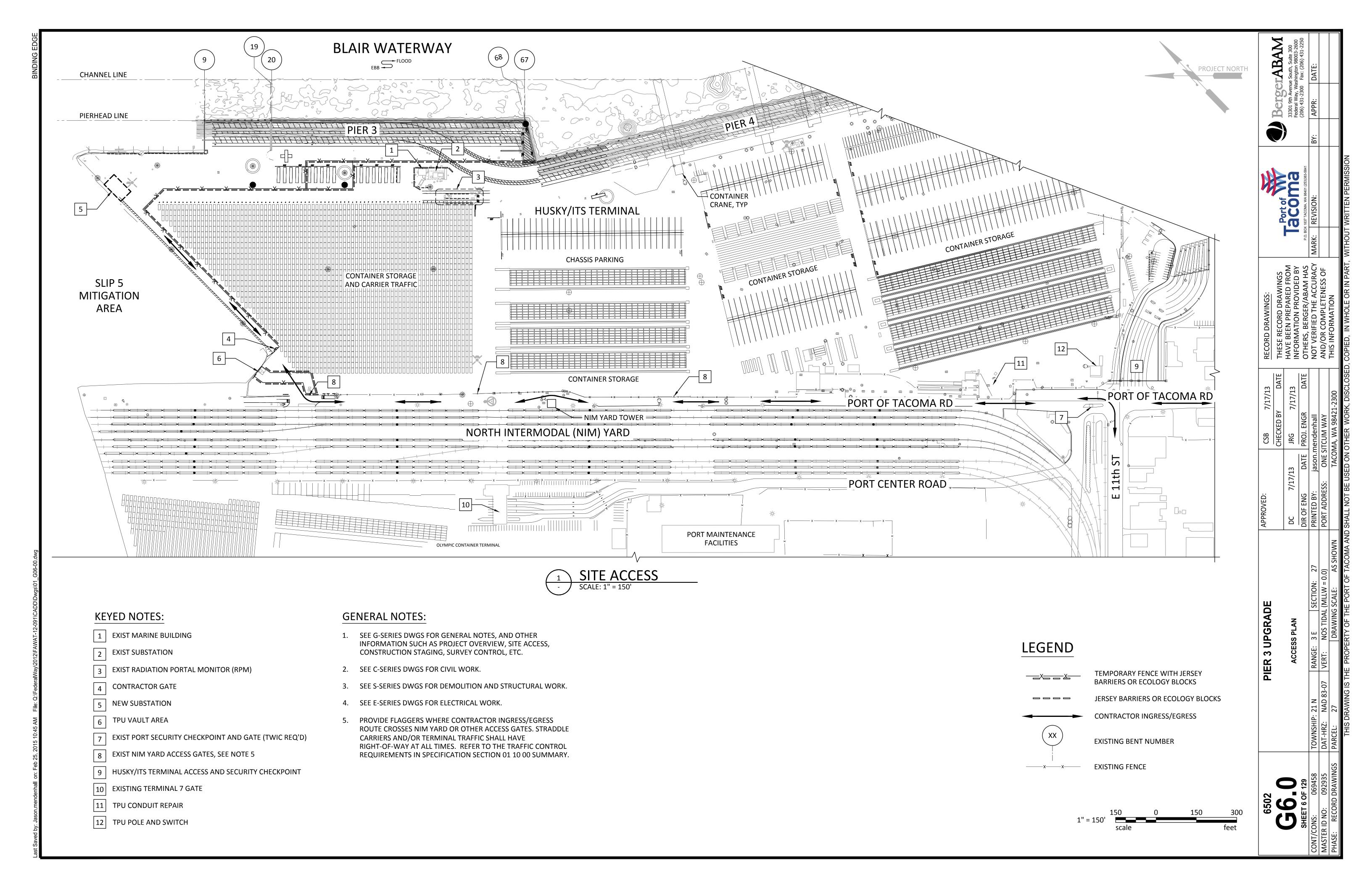
- DEMOLITION DRAWINGS ARE INTENDED TO SHOW OVERALL SCOPE AND COMPLEXITY OF DEMOLITION. IN ADDITION, THE CONTRACTOR SHALL PERFORM ALL OTHER INCIDENTAL DEMOLITION REQUIRED TO COMPLETE THE WORK.
- 2. PRIOR TO COMMENCING DEMOLITION OPERATIONS, IMPLEMENT EROSION AND SEDIMENT CONTROL PLAN.
- PRIOR TO DEMOLITION, THE CONTRACTOR SHALL SAWCUT WHERE NOTED OR OTHERWISE PROVIDE A SMOOTH CLEAN CUT FROM ALL CONNECTING LINKS TO THE REMAINING STRUCTURES OR ADJACENT YARD AREAS.
- THE CONTRACTOR SHALL CONTAIN THE DEMOLITION WITHIN THE LIMITS DESIGNATED, TO PREVENT DAMAGE TO EXISTING STRUCTURES, UTILITIES, OR FACILITIES, AND KEEP ALL DEBRIS FROM FALLING INTO THE WATER OR ON THE SLOPE. THE CONTRACTOR SHALL PROVIDE FLOATS AND FALSEWORK TO CATCH ALL FALLING DEBRIS IN OVER-WATER AREAS.
- 5. ANY DAMAGE INCURRED TO ANY PART OF THE SITE OR BOUNDARY NOT SPECIFICALLY DESIGNATED FOR DEMOLITION SHALL BE REPAIRED, REPLACED, AND/OR RECONSTRUCTED BY THE CONTRACTOR, AT IT'S OWN EXPENSE, TO THE PREDISTURBED CONDITION AS DIRECTED BY THE ENGINEER.
- 6. ALL DEMOLITION MATERIAL, EXCEPT AS NOTED AND/OR SPECIFIED, SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND BE COMPLETELY REMOVED AND DISPOSED OF BY THE
- FOR BASIS OF BID FOR SAW-CUTTING AND FOR PAVEMENT DEMO, CONTRACTOR SHALL ASSUME EXISTING ASPHALT PAVEMENT THICKNESS ON PIER IS NOMINALLY 4-INCHES THICK. 6-INCHES WHERE THERE IS CONCRETE STRUCTURE DIRECTLY BELOW PAVEMENT, AND ASSUME EXISTING ASPHALT PAVEMENT THICKNESS LANDSIDE IS 10 INCHES.
- 8. SEE ELECTRICAL PLANS FOR DEMOLITION OF ELECTRICAL UTILITIES

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3 **PIER**





STRUCTURAL NOTES

CODES AND STANDARDS

- 1. ALL METHODS AND MATERIALS SHALL CONFORM TO BOTH THE INTERNATIONAL BUILDING CODE (IBC) (2009 EDITION) AND THE INTERNATIONAL EXISTING BUILDING CODE (IEBC) (2009 EDITION) AS AMENDED AND ADOPTED BY THE CITY OF TACOMA
- 2. REINFORCED CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301) AND "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318).
- 3. STRUCTURAL AND MISCELLANEOUS STEEL FABRICATION AND ERECTION THEREOF SHALL CONFORM TO THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (AISC 303) AND "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" (AISC 360).
- 4. WELDING OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO "STRUCTURAL WELDING CODE - STEEL" (AWS D1.1).
- 5. WELDING OF REINFORCING STEEL SHALL CONFORM TO "STRUCTURAL WELDING CODE" REINFORCING STEEL" (AWS D1.4).
- 6. ALL METHODS AND MATERIALS SHALL CONFORM TO THE WASHINGTON STATE 2012 STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION AS PREPARED BY THE WASHINGTON STATE DEPARTMENT OF TRANSPORTATION (WSDOT) AND THE AMERICAN PUBLIC WORKS ASSOCIATION (APWA) WASHINGTON STATE CHAPTER.

GENERAL

- 1. THESE NOTES CONTAIN GENERAL INFORMATION ONLY. THE CONTRACTOR SHALL VERIFY INFORMATION PROVIDED WITH THE SPECIFICATIONS AND OTHER DOCUMENTS AND BRING ANY CONFLICTS TO THE ATTENTION OF THE ENGINEER BEFORE BEGINNING AFFECTED WORK. THE ENGINEER WILL RESOLVE ANY SUCH CONFLICT.
- 2. ALL DIMENSIONS AND DETAILS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND CONSTRUCTION.
- 3. ALL SHOP DRAWINGS FOR PRECAST CONCRETE ELEMENTS, REINFORCING STEEL, MISCELLANEOUS STEEL, AND FENDER SYSTEM SHALL BE SUBMITTED TO THE ENGINEER AND WILL BE REVIEWED BY THE ENGINEER PRIOR TO FABRICATION.
- 4. ALL ELEVATIONS REFER TO MEAN LOWER LOW WATER (MLLW) EL 0.00 FT AS DEFINED BY THE U.S. ARMY CORPS OF ENGINEERS. SEE DRAWING C1.0 FOR HORIZONTAL AND VERTICAL SURVEY CONTROL.
- 5. EXISTING FEATURES SHOWN ON THESE DRAWINGS ARE BASED ON AS-BUILT DATA AND RECENT SURVEYS. THE LOCATIONS OF EXISTING UTILITIES AND OTHER FEATURES SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO PERFORMING THE ASSOCIATED WORK. USE A LOCATOR SERVICE AND EXCAVATE TO EXPOSE UTILITY LINES.
- 6. THE CONTRACTOR SHALL BRING ANY CONFLICTS BETWEEN EXISTING UTILITIES AND THE WORK TO THE ENGINEER'S ATTENTION PRIOR TO START OF CONSTRUCTION ON THE AFFECTED WORK.
- 7. ANY DAMAGE CAUSED BY THE CONTRACTOR'S OPERATIONS TO EXISTING FEATURES, STRUCTURES, AND UTILITIES THAT ARE TO REMAIN, SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.

REINFORCED CONCRETE

1. REINFORCING STEEL

- A. PRESTRESSING STEEL SHALL BE UNCOATED LOW-RELAXATION SEVEN-WIRE STRAND CONFORMING TO ASTM A 416, GRADE 270.
- ALL REINFORCING STEEL SHALL BE DEFORMED STEEL BARS CONFORMING TO ASTM A 615, GRADE 60, UNLESS NOTED OTHERWISE. SEE SPECIFICATIONS.
- C. ALL DOWELS FOR PILING SHALL BE OF WELDABLE QUALITY AND SHALL CONFORM TO ASTM A 706, GRADE 60.
- D. EPOXY COATED REINFORCEMENT SHALL CONFORM TO ASTM A 934.
- WIRE FOR SPIRAL REINFORCEMENT SHALL CONFORM TO ASTM A 82.
- SEE SPECIFICATIONS FOR HIGH-STRENGTH BAR, WELDED HEADED STUD, MECHANICAL COUPLER, AND HEADED REINFORCEMENT REQUIREMENTS.
- SPLICING OF LONGITUDINAL REINFORCEMENT OVER 40 FT IN LENGTH, EXCEPT AS SPECIFICALLY NOTED ON THE DRAWINGS, WILL BE PERMITTED UNLESS NOTED OTHERWISE. SPLICES SHALL BE CLASS "B" (ACI 318) STAGGERED THREE LAP LENGTHS WITH NO MORE THAN 50% OF THE BARS BEING SPLICED AT ANY ONE LOCATION.
- H. PROVIDE CORNER BARS AT ALL CORNERS. CORNER BARS SHALL MATCH THE NUMBER/SPACING AND DIAMETER OF ALL HORIZONTAL REINFORCEMENT AT THE CORNER. TERMINATED STRAIGHT BARS SHALL EXTEND THE FULL AVAILABLE LENGTH INTO ADJOINING MEMBERS. SPLICE CORNER BAR TO TERMINATED STRAIGHT BAR WITH MINIMUM SPLICE LENGTH BELOW.
- SPLICES SHALL CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED

SCHEDULE OF LAP SPLICE LENGTHS (f'c = 5000 PSI)					
BAR SIZE	TOP BARS	BOTTOM BARS			
4	2'-5"	1'-10"			
5	3'-0"	2'-4"			
6	3'-7"	2'-9"			
7	5'-3"	4'-0"			
8	6'-0"	4'-7"			
9	6'-9"	5'-2"			
10	7'-6"	5'-9"			
11	8'-3"	6'-4"			

NOTES:

2. PRECAST CONCRETE

- 1. VALUES ARE BASED ON CLASS "B" SPLICES (MAX OF 50% BAR SPLICED AT ONE LOCATION).
- 2. TOP BARS ARE DEFINED AS ANY HORIZONTAL BAR PLACED SUCH THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE BAR IN ANY SINGLE POUR.
- 3. INCREASE LAP SPLICE LENGTH OF EPOXY COATED REINFORCEMENT BY 50%.
- DETAIL ALL REINFORCING STEEL IN ACCORDANCE WITH ACI 315, LATEST EDITION. ALL REINFORCING STEEL BENDS SHOWN ARE OUT TO OUT UNLESS OTHERWISE NOTED. USE SEISMIC HOOK DETAILS FOR ALL TIES AND STIRRUPS UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
- K. ALTERNATE ENDS OF HORIZONTAL TIES (CROSS TIES WITH 135 DEGREE HOOK AND 90 DEGREE HOOK) EXCEPT WHEN PLACED AGAINST HARDENED CONCRETE WHERE THE 90 DEGREE HOOK SHALL BE PLACED AT THE HARDENED CONCRETE FACE.

MINIMUM COMPRESSIVE

STRENGTH AT 28 DAYS PRECAST PRESTRESSED CONCRETE PILES 8000 PSI MINIMUM COMPRESSIVE 3. CAST-IN-PLACE CONCRETE STRENGTH AT 28 DAYS

A. UNLESS NOTED OTHERWISE 5000 PSI B. CONCRETE IN PILE BUILDUP 6000 PSI

C. DRILLED SHAFTS CLASS 4000W PER WSDOT

- 4. ALL EXPOSED CORNERS SHALL BE CHAMFERED 3/4", UNLESS OTHERWISE NOTED.
- 5. CONSTRUCTION JOINTS SHALL BE PROVIDED ONLY AS NOTED ON THE DRAWINGS AND AS SPECIFICALLY PERMITTED BY THE ENGINEER. ROUGHEN EXISTING CONCRETE RECEIVING NEW CONCRETE TO A MINIMUM 1/4" AMPLITUDE IN ACCORDANCE WITH THE SPECIFICATIONS. CLEAN AND REMOVE LAITANCE, THEN DAMPEN FOR AT LEAST 12 HOURS BEFORE PLACING NEXT POUR.

GROUT, MORTAR, DOWEL ADHESIVE, AND CRACK REPAIRS

1. SEE SPECIFICATIONS.

MISCELLANEOUS STEEL

- MISCELLANEOUS AND STRUCTURAL STEEL SHAPES AND PLATES, EXCEPT AS NOTED OTHERWISE, SHALL CONFORM TO THE SPECIFICATIONS.
- 2. BOLTS AND NUTS SHALL CONFORM TO ASTM A 307, UNLESS NOTED OTHERWISE.
- ANCHOR BOLT OR ANCHOR RODS SHALL CONFORM TO ASTM F 1554, GRADE 55, UNLESS NOTED OTHERWISE.
- FOR ITEMS TO BE COATED, GALVANIZED, OR GALVANIZED AND COATED, REFER TO THE SPECIFICATIONS. REPAIR DAMAGE TO COATINGS AND GALVANIZING IN ACCORDANCE WITH THE SPECIFICATIONS.
- PROVIDE BLEED HOLES IN EMBEDDED PLATES AND SHAPES AT 24-IN. ON CENTER MAXIMUM.
- 6. ALL MISCELLANEOUS PIPE SHALL CONFORM TO ASTM A 53 GRADE B OR ASTM A 106 GRADE B OR C UNLESS NOTED OTHERWISE. PIPE BOLLARDS SHALL CONFORM TO ASTM A 106 GRADE C.

PILING

1. PILES SHALL BE DRIVEN TO THE REQUIRED TIP ELEVATIONS AND MINIMUM CAPACITIES INDICATED. SEE SPECIFICATIONS.

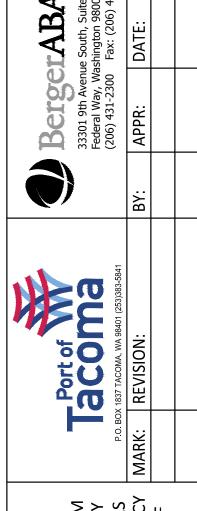
LOAD PATH

VERTICAL LOADS:

LOADS ARE DISTRIBUTED THROUGH CONCRETE CRANE BEAMS AND BALLAST, BALLAST TO DECK SYSTEM, DECK SYSTEM TO PILE CAPS, PILE CAPS AND CRANE BEAMS TO PILES, PILES TO SOIL IN FRICTION AND END BEARING.

HORIZONTAL LOADS:

LOADS, INCLUDING SEISMIC FORCES, SOIL PRESSURE, SHIP IMPACT, AND MOORING FORCES ARE TRANSMITTED THROUGH THE DECK ACTING AS A DIAPHRAGM TO THE BATTER PILES. HORIZONTAL LOADS PARALLEL TO THE WHARF ARE RESISTED BY SOIL FRICTION AGAINST THE **BULKHEAD AND SHEET PILES.**



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3 PIER

GRA

DESIGN CRITERIA

DESIGN LOADS:

- 1. UNIFORM LIVE LOAD ON PIER (MATCH EXISTING):
 - A. 750 PSF ON ANY COMBINATION OF SPANS
 - B. 2000 PSF AT HEAVY LOAD ZONE
- 2. CONCENTRATED LIVE LOAD (MATCH EXISTING):

P = 100 KIPS, INCLUDING IMPACT WHEEL/OUTRIGGER TRANSVERSE SPACING > 8 FEET WHEEL/OUTRIGGER LONGITUDINAL SPACING > 18 FEET CONTACT PRESSURE: 100 PSI

3. CONTAINER STORAGE LOAD (MATCH EXISTING):

100 KIPS ON 8x20 FT FOOTPRINT

BERTHING:

A. VESSELS

CONTAINER SHIP 1	
LENGTH OVERALL (LOA):	1380 FEET
BEAM:	195 FEET
DRAFT:	50 FEET
DISPLACEMENT:	230,000 LT
APPROACH ANGLE:	6 DEGREES
PERPENDICULAR VELOCITY:	0.16 KNOTS (0.26 FT/S)

2. CONTAINER SHIP 2

LENGTH OVERALL (LOA):	1200 FEET
BEAM:	160 FEET
DRAFT:	50 FEET
DISPLACEMENT:	180,000 LT
APPROACH ANGLE:	6 DEGREES
PERPENDICULAR VELOCITY:	0.16 KNOTS (0.26 FT/S)
	•

CONTAINER SHIP 3	
LENGTH OVERALL (LOA):	1145 FEET
BEAM:	149 FEET
DRAFT:	45 FEET
DISPLACEMENT:	140,000 LT
APPROACH ANGLE:	6 DEGREES
PERPENDICULAR VELOCITY:	0.16 KNOTS (0.26 FT/S)

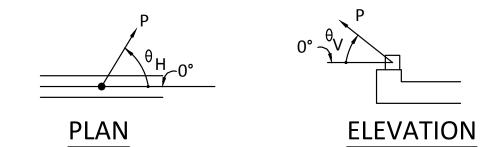
4. CONTAINER SHIP 4

LENGTH OVERALL (LOA):	965 FEET
BEAM:	106 FEET
DRAFT:	40 FEET
DISPLACEMENT:	80,000 LT
APPROACH ANGLE:	10 DEGREES
PERPENDICULAR VELOCITY:	0.24 KNOTS (0.40 FT/S)

B. FENDER REQUIREMENTS

- 1. MINIMUM BERTHING ENERGY = 600 KIP-FT
- 2. MAXIMUM BERTHING REACTION = 300 KIPS
- 3. VESSEL HULL REACTION PRESSURE ≤ 5.0 KSF

5. LINE PULL ON MOORING BOLLARDS:



LOCATION	LINE PULL P	RANGE OF HORIZONTAL ANGLE ⁰ H	RANGE OF VERTICAL ANGLE $\theta_{ m V}$
EXIST BOLLARD (BENT 9 - 19) BOLLARD (BENT 20 - 65) BOLLARD (BENT 66 & 67)	150 TONS 100 TONS 200 TONS	0° TO 180° 0° TO 180° 0° TO 180°	0° TO 30° 0° TO 60°

CONTAINER CRANE LOADS:

WHEEL LOADS ARE EXPRESSED AS EQUIVALENT UNIFORM DISTRIBUTION LOADS BASED ON EIGHT WHEELS PER CORNER SPACED AT 4'-9" ON CENTER.

LAND SIDE

CLop =

40

OPERATING SERVICE (KIPS/FT):

	Wcr =	1.0	Wcr =	1.0
В.	OVERLOAD	SERVICE (KIPS	5/FT):	
		WATER SIDE		LAND SIDE
	CLov =	60	CLov =	50
	LATG =	0.5	LATG =	0.5

WATER SIDE

STOWED SERVICE (KIPS/FT):

WATER SIDE I	LAND SIDE
CLst = 50	50
W = 2.5 W =	2.5

EXTREME SERVICE (KIPS/FT):

	WATER SIDE	ļ	AND SIDE
CLeq =	65	CLeq =	40
EQ =	5.5	EQ =	5.5

- E. PIN SOCKETS (HORIZONTAL LOAD):
 - P = 150 KIPS PER SOCKET (SERVICE, LF = 1.6)
- F. CRANE STOPS (HORIZONTAL LOADS):
- P = 330 KIPS @ 4'-6" ABOVE RAIL (LF = 1.6)
- G. CRANE TIE-DOWNS (UPLIFT LOAD):
 - P = 400 KIPS (SERVICE, LF = 1.6)

LOAD COMBINATIONS:

PER TMC, LOAD COMBINATIONS ARE FROM THE 2009 IEBC AND 2009 IBC. VESSEL LOADS (BERTHING, MOORING) HAVE BEEN INCLUDED PER CHAPTER 31F OF 2010 CBC.

LRFD (STRENGTH DESIGN & STRUCTURAL CAPACITY CHECK)

- U1 = 1.2D + 1.6L + 1.5CLop + 1.2B + 1.6W + 1.2C + 1.6H (OPERATING)
- U2 = 1.2D + 1.6L + 1.3CLst + 1.2B + 1.6Wst + 1.2C + 1.6H (STOWED, VESSEL NOT MOORED)
- U3 = 1.2D + 1.6L + 1.0CLov + 1.2B + 1.0LATG + 1.2C + 1.6H (OVERLOAD)
- U4 = 1.2D + 1.2CLd + 1.2B + 1.6W + 1.2C + 1.6H + 1.6 BE (BERTHING)
- U5 = 1.2D + 1.6L + 1.2B + 1.6W + 1.5CLop + 1.2C + 1.6H + 1.6M (MOORING)
- U6 = 1.4(D+L+EQ) (EXTREME, 1987 STRUCTURE)
- $U7 = 0.9D \pm 1.4EQ$ (EXTREME, 1987 STRUCTURE)

ASD (GEOTECHNICAL PILE CAPACITY & STRUCTURE SERVICEABILITY)

- S1 = 1.0D + 1.0L + 1.0CLop + 1.0B + 1.0W + 1.0C + 1.0H (OPERATING)
- S2 = 1.0D + 1.0L + 1.0CLst + 1.0B + 1.0Wst + 1.0C + 1.0H (STOWED, VESSEL NOT MOORED)
- S3 = 1.0D + 1.0L + 1.0CLov + 1.0B + 1.0LATG + 1.0C + 1.0H (OVERLOAD)
- S4 = 1.0D + 1.0CLd + 1.0B + 1.0W + 1.0C + 1.0H + 1.0BE (BERTHING)
- S5 = 1.0D + 1.0L + 1.0B + 1.0W + 1.0CLop + 1.0C + 1.0H + 1.0M (MOORING)
- S6 = D + L + EQ (1987 STRUCTURE)
- $S7 = 0.85D \pm EQ (1987 STRUCTURE)$

LEGEND:

- B BOUYANCY LOAD BE - BERTHING LOAD C - CURRENT LOAD
- D STRUCTURE DEAD LOAD CLd - CONTAINER CRANE DEAD LOAD
- EQ EARTHQUAKE LOAD (INCLUDES CLeq)
- H SOIL LOAD
- L UNIFORM LIVE LOAD OR CONCENTRATED LIVE LOAD
- LATG LATERAL GANTRY LOAD
- CLop CONTAINER CRANE OPERATING LOAD (INCLUDES CLd)
- CLov CONTAINER CRANE OVERLOAD (INCLUDES CLd)
- CLst CONTAINER CRANE LOAD UNDER STOWED (WIND) CONDITIONS (INCLUDES CLd)
- M MOORING LOAD
- W WIND LOAD (55 MPH)
- Wst STOWED WIND LOAD (85 MPH)
- CLeq CONTAINER CRANE DEAD LOAD INCLUDING VERTICAL EQ

CONTAINER CRANE WHEEL LOAD DIAGRAM

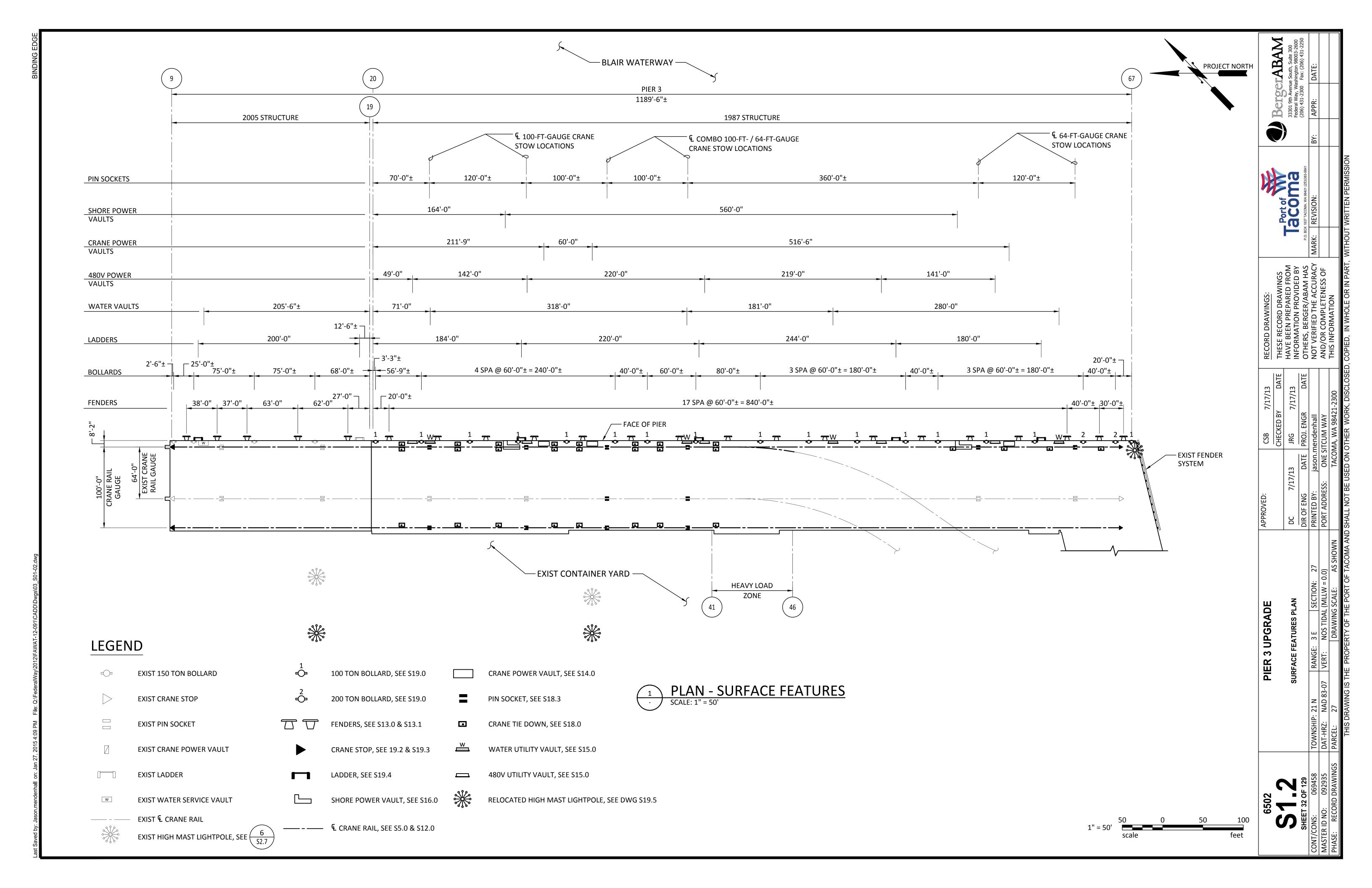
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9. SPECIAL INSPECTION:

- A. SPECIAL INSPECTION WILL BE PROVIDED BY THE PORT. COORDINATE SPECIAL INSPECTION WITH THE ENGINEER.
- THE ELEMENTS REQUIRING SPECIAL INSPECTION PER IEBC & IBC CHAPTER 17 ARE AS FOLLOWS:
 - CONCRETE
 - BOLTS AND ANCHORS INSTALLED IN CONCRETE
 - DRILLED-IN DOWELS
 - REINFORCING STEEL
- STRUCTURAL WELDING PILING

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UPGRAD ${\mathfrak C}$ **PIER**



PILE NO	MUDLINE EL	FINAL TIP EL	ULT CAPACITY
4 **	(ft)	(ft)	(Kips)
1**	-40	-114.5	950
2**	-40	-114.5	850
3	-40	-114.5	850
4	-40	-114	1000
5*	-40	-114	1000
6*	-40	-114	975
7	-40	-114	900
8	-40	-113	925
9	-40	-114	875
10	-40	-114	975
11	-40	-114	925
12	-40	-114	950
13	-40	-114	875
14	-40	-114	950
15**	-40	-114	875
16	-40	-114	975
17**	-40	-114.5	925
18	-40	-114.5	825
19**	-40	-114.5	950
20	-40	-114.5	825
21	-40	-114.5	950
22	-40	-114.5	950
23**	-43	-114	950
24	-43	-114.5	925
25	-43	-114.5	925
26	-43	-114.5	975
27	-43	-114.5	950
28	-43	-115	950
29	-43	-115	950
30	-43	-115	950
31	-43	-115	950
32	-43	-115	950
33**	-43	-115.5	850
34	-43	-115	900
35	-44	-115	875
36	-44	-115	950
37	-43	-115	875
38	-42.5	-115	900
39	-41	-114.5	825
40**	-42	-114.5	800
41	-45	-115	875
42	-41	-115	875
43	-42	-115	800
44	-41	-115.5	750
45	-40	-115	700
46	-40	-114	675
47*	-40	-114	700
ч,	-10	117	700

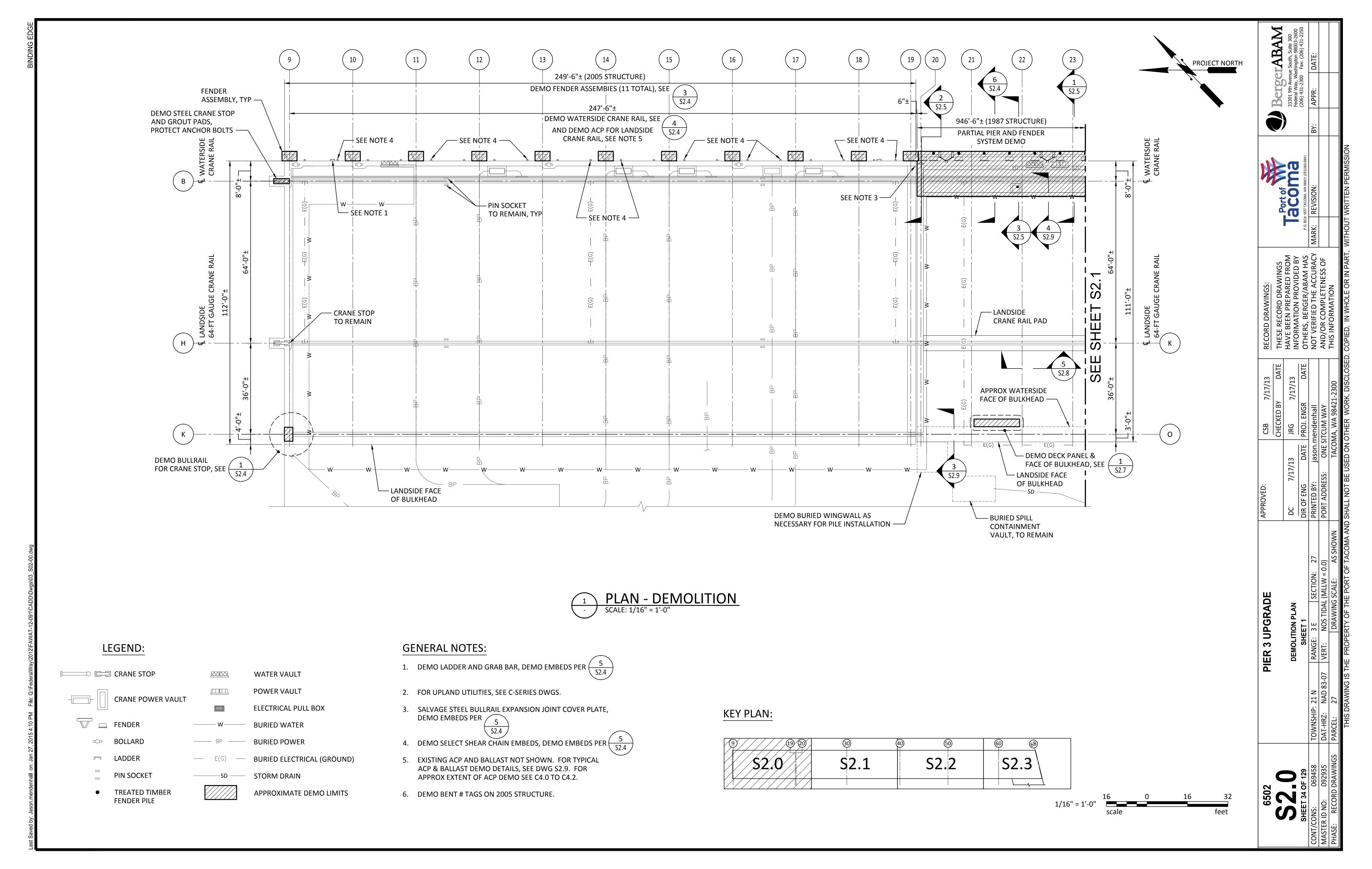
PILE NO	MUDLINE EL (ft)	FINAL TIP EL (ft)	ULT CAPACITY (Kips)
48	-40	-116	825
49	-40	-115.5	850
50	-39	-114.5	825
51	-42.5	-115	800
52	-39	-114.5	800
53	-39	-115	875
54	-39	-114.5	750
55	-39	-114.5	850
56	-39	-114.5	825
 57	-39	-114.5	775
58	-41	-114.5	850
59	-41	-114.5	825
60	-41	-114.5	775
61**	-41	-115	725
62	-43	-113.5	725
63	-42.5	-115	850
64	-43	-115	850
65	-42	-115	850
66	-41	-114.5	850
67	-42	-113.5	725
68	-42	-113.5	800
69	-42	-115.5	850
70	-42	-115.5	825
70	-41	-115	850
72	-42	-114.5	775
73	-42	-114.5	950
73 	-42	-114.5	950
75**	-40	-114.5	900
75 76	-42	-114.5	975
	-42 -42	-114.5	725
	-39	-115.5	800
	-40	-114.5	800
80	-42	-114.5	800
81	-42	-114.5	750
82	-39	-114.5	
83	-39	-114.5	775 775
84	-38	-114.5	775
85*	-36 -40	-110.5	810
86	-40 -40	-114.5	825
87	-40 -42	-114.5	850
88	-42 -42		850
89		-114.5 -103.5	
	-35		800
90	-35	-115	800
91	-34	-115	800
92	-36	-114.5	800
93	-36	-114.5	700
94	-37	-114.5	675

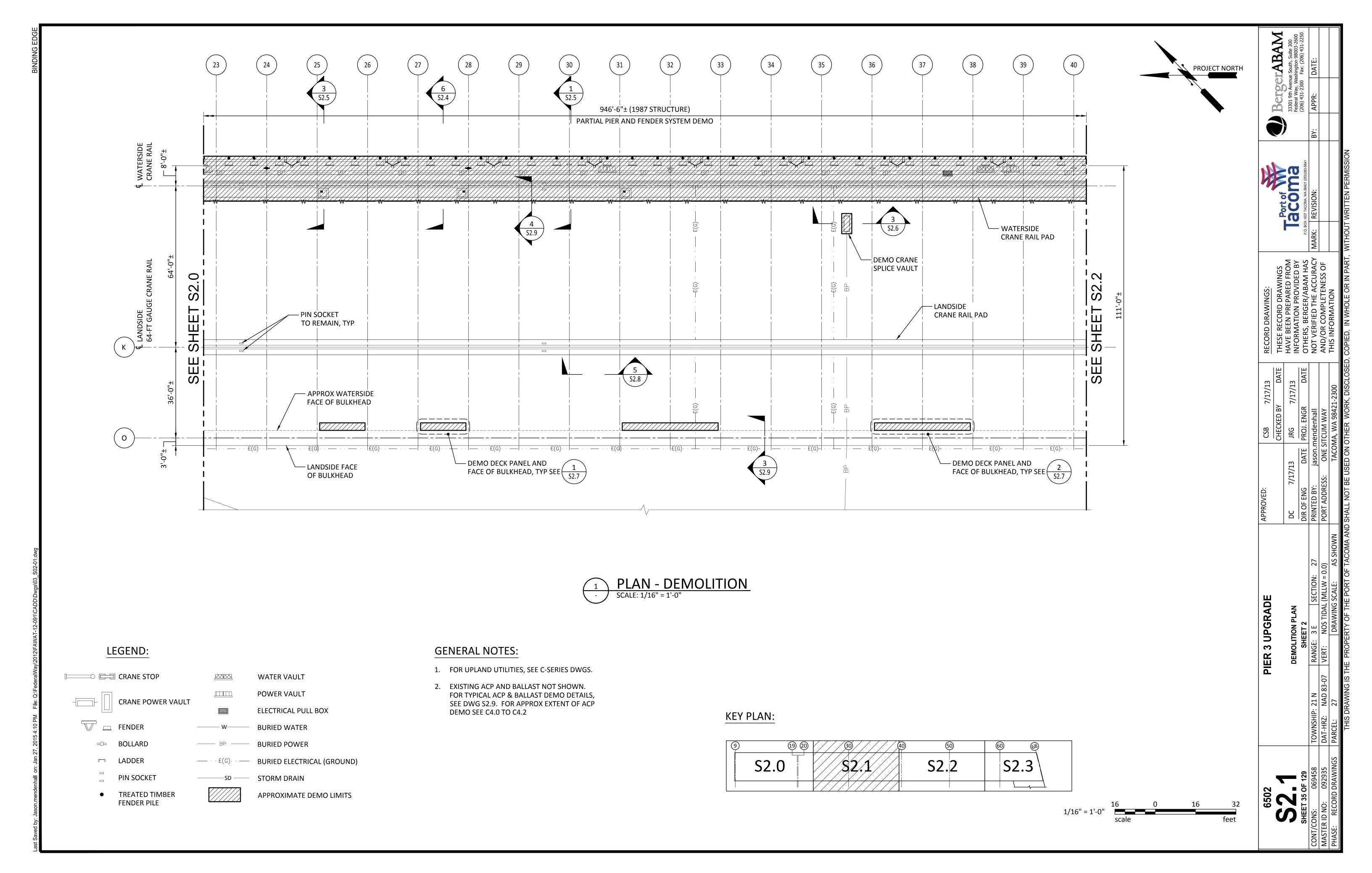
PILE NO	MUDLINE EL	FINAL TIP EL	ULT CAPACITY
PILE NO	(ft)	(ft)	(Kips)
95	-31.5	-118.5	900
96**	-29.5	-119.5	775
97	-13	-110	975
98	-15	-111	875
99	-8.5	-118	750
100	-7.5	-118	725
101	-0.5	-119	825
102	-0.5	-119	750
103	7	-75.5	1150
104*	7	-75.5	1075
105	7	-75.5	1150
106	7	-75.5	1150
107	14	-75.5	790
108	14	-75.5	790
109	14	-75.5	790
110	14	-75.5	790
111*	14	-75.5	790
112	14	-75.5	790
113	14	-75.5	790
114	14	-75.5	790
115	14	-75.5	960
116	14	-75.5	885
117	14	-75.5	985
118	14	-75.5	885
119	14	-75.5	935
120	14	-75.5	1010
121	14	-75.5	1010
122	14	-75.5	885
123	14	-75.5	960
124	14	-75.5	885
125	14	-75.5	935
126	14	-75.5	885
127	14	-75.5	810
128	14	-75.5	835
129	14	-75.5	760
130	14	-75.5	835
131	14	-75.5	835
132	14	-75.5	885
133	14	-75.5	860
134	14	-75.5	860
135*	14	-75.5	835
136*	14	-75.5	835
137	14	-75.5	810

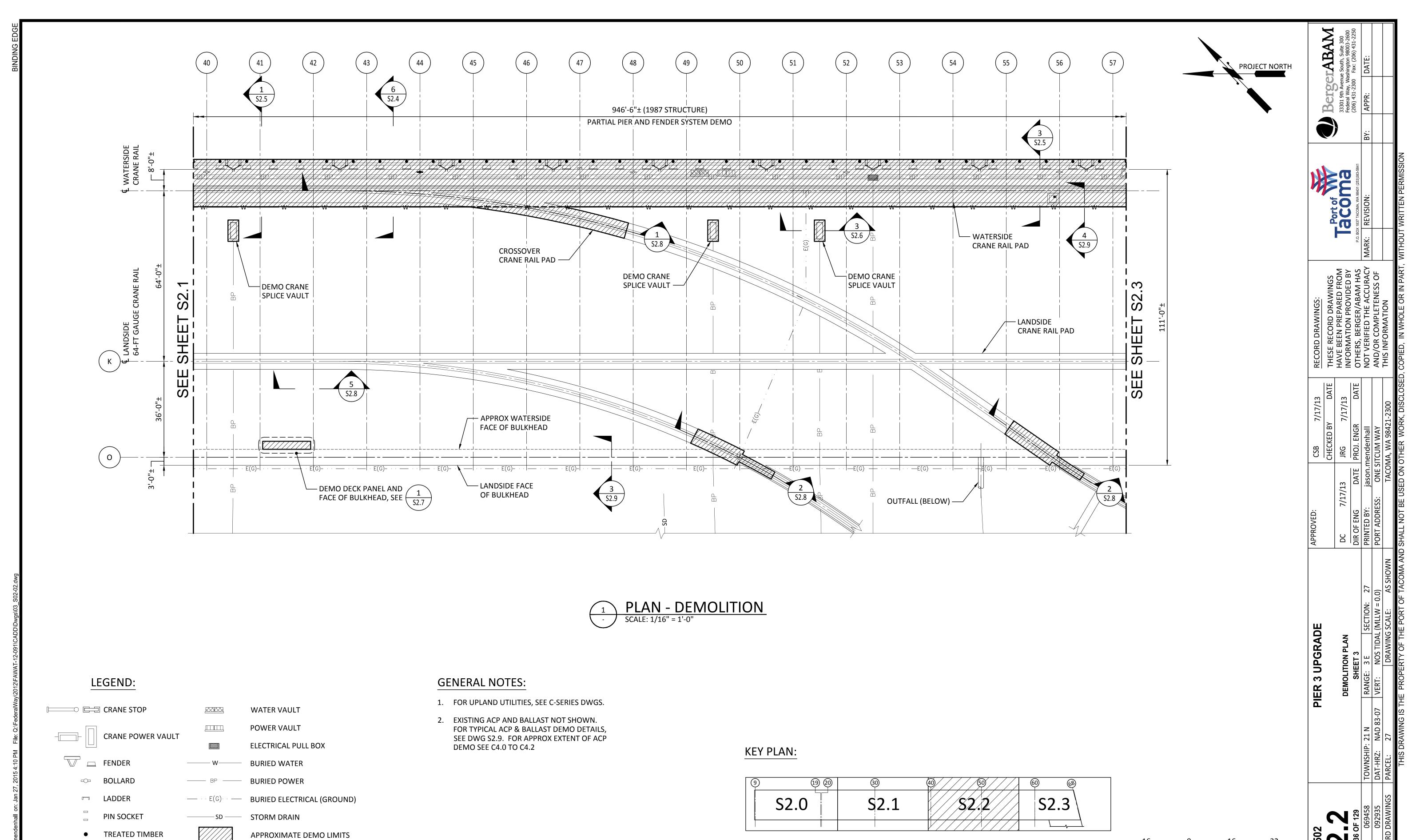
^{*} PDA PILE W/ RESTRIKE

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SHEET 33 OF 129					DIR OF ENG	DATE PROJ. ENGR		DATE OTHERS, BERGER/ABAM HAS	BAM HAS	P.O. BOX 183	P.O. BOX 1837 TACOMA, WA 98401 (253)383-5841		(206) 431-2300	(206) 431-2300 Fax: (206) 431-2250	
001/CONS: 069458	TOWNSHIP: 21 N	RANGE: 3 E	3 E SECTION: 27	27	PRINTED BY:	jason.mendenhall	nhall	NOT VERIFIED THE ACCURACY		MARK: REVISION:	EVISION:	BY:	APPR:	DATE:	
ASTER ID NO: 092935	DAT-HRZ: NAD 83-07 VERT:	VERT:	NOS TIDAL (MLLW = 0.0)	(0	PORT ADDRESS:	ONE SITCUM WAY	1 WAY	AND/OR COMPLETENESS OF	ENESS OF						
IASE: RECORD DRAWINGS	/INGS PARCEL: 27		DRAWING SCALE:	AS SHOWN		TACOMA, W.	TACOMA, WA 98421-2300	THIS INFORMATION							
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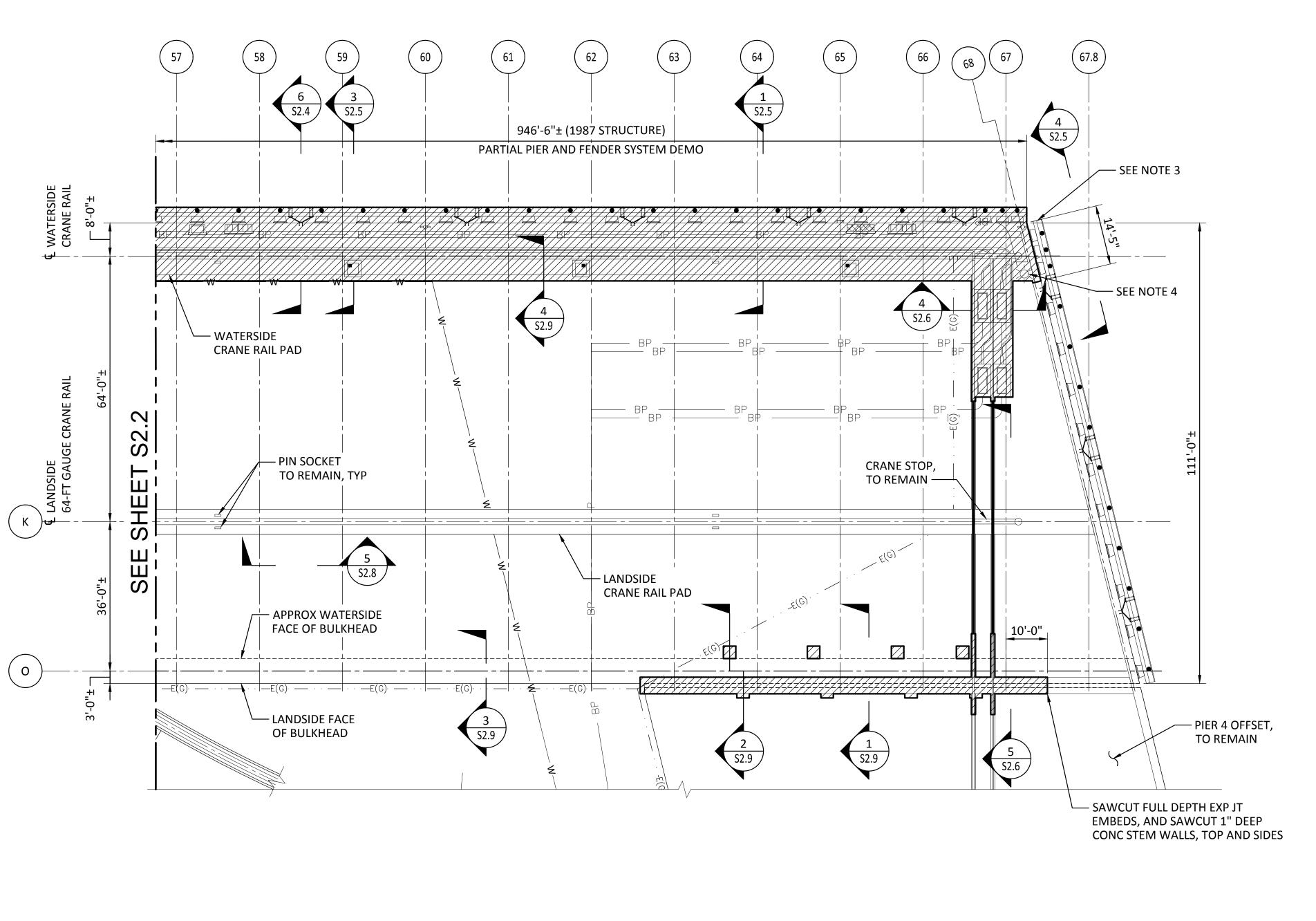
^{**} RESTRIKE PILE

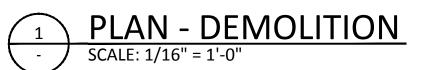




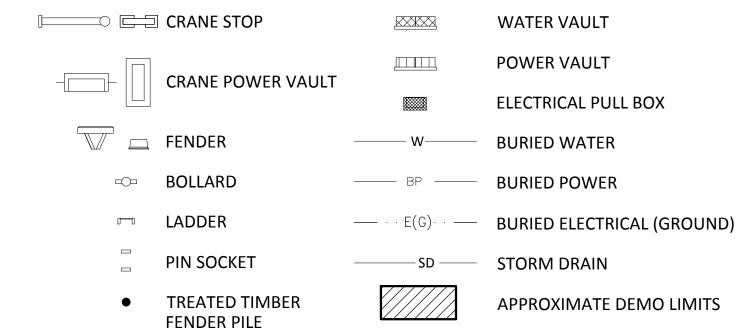


FENDER PILE





LEGEND:

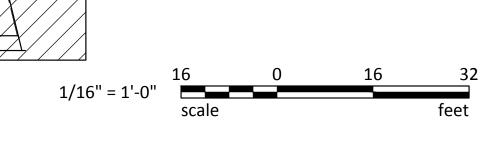


GENERAL NOTES:

- 1. FOR UPLAND UTILITIES, SEE C-SERIES DWGS.
- EXISTING ACP AND BALLAST NOT SHOWN. FOR TYPICAL ACP & BALLAST DEMO DETAILS, SEE DWG S2.9. FOR APPROX EXTENT OF ACP DEMO SEE C4.0 TO C4.2
- B. DETACH FENDER SYSTEM AS REQUIRED AND LEAVE INTACT. SALVAGE ALL FENDERS AND HARDWARE FOR RE-INSTALLATION.
- 4. SALVAGE HIGH MAST LIGHT FOR REINSTALLATION.

KEY PLAN:

9	19 20	30 (40 50	(6) / (8) / / / /
	S2.0	S2.1	S2.2	\$2.3



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PIER 3 UPGRADE

DEMOLITION PLAN
SHEET 4

SHEET 4

SHEET 4

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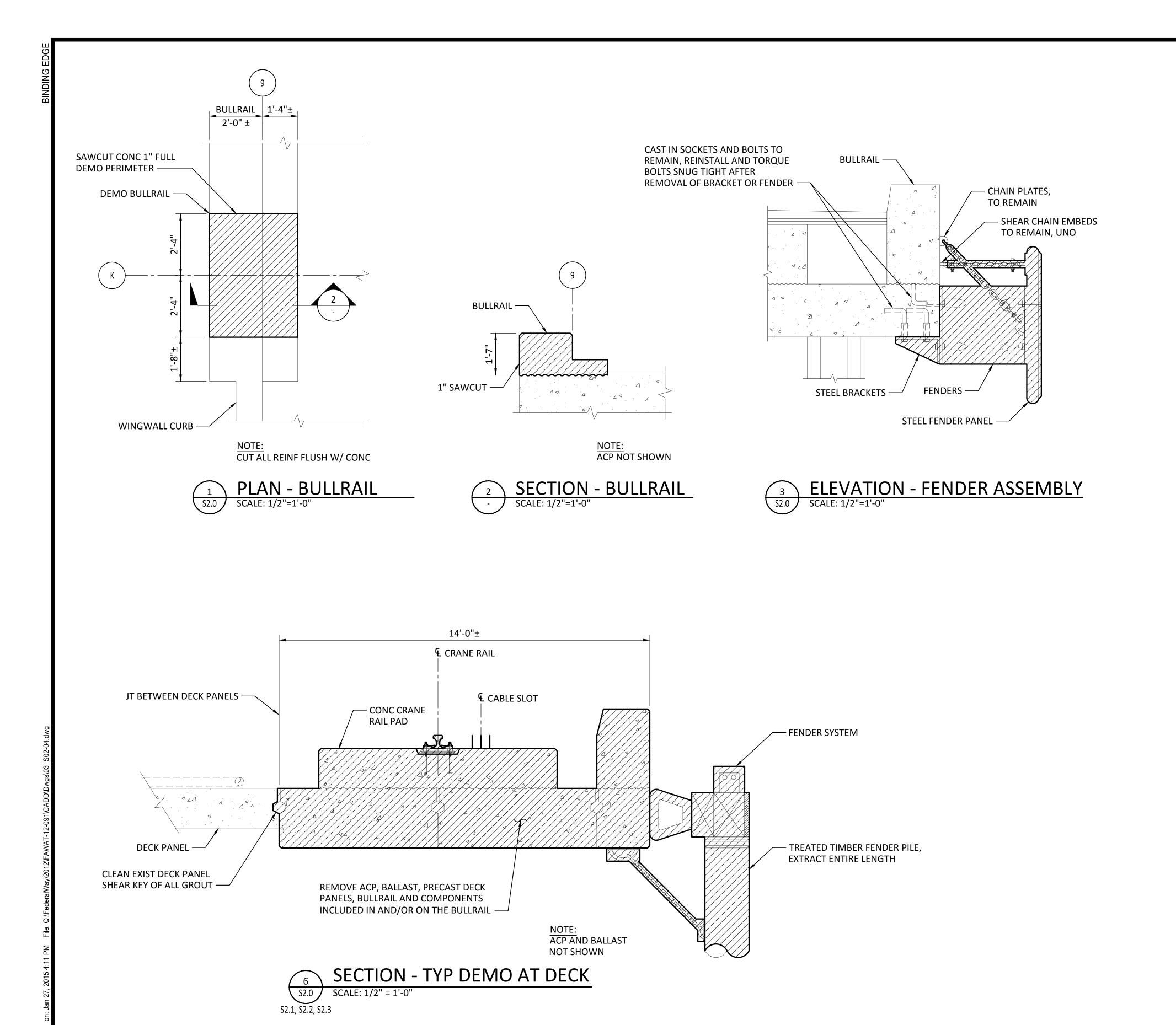
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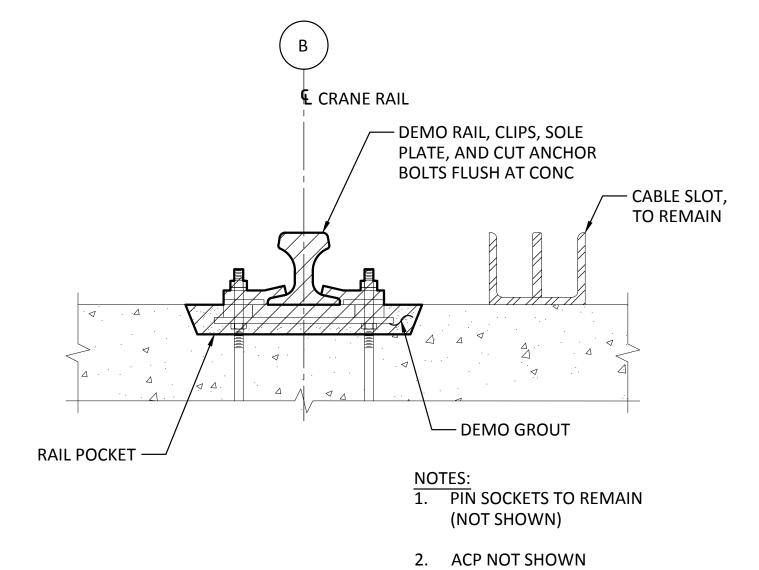
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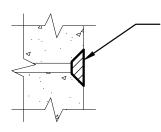
DRAWING SCALE: AS SHOWN

6502 **S2.3**SHEET 37 OF 129





SECTION - WATERSIDE CRANE RAIL
S2.0 SCALE: 1 1/2" = 1'-0" 2005 STRUCTURE



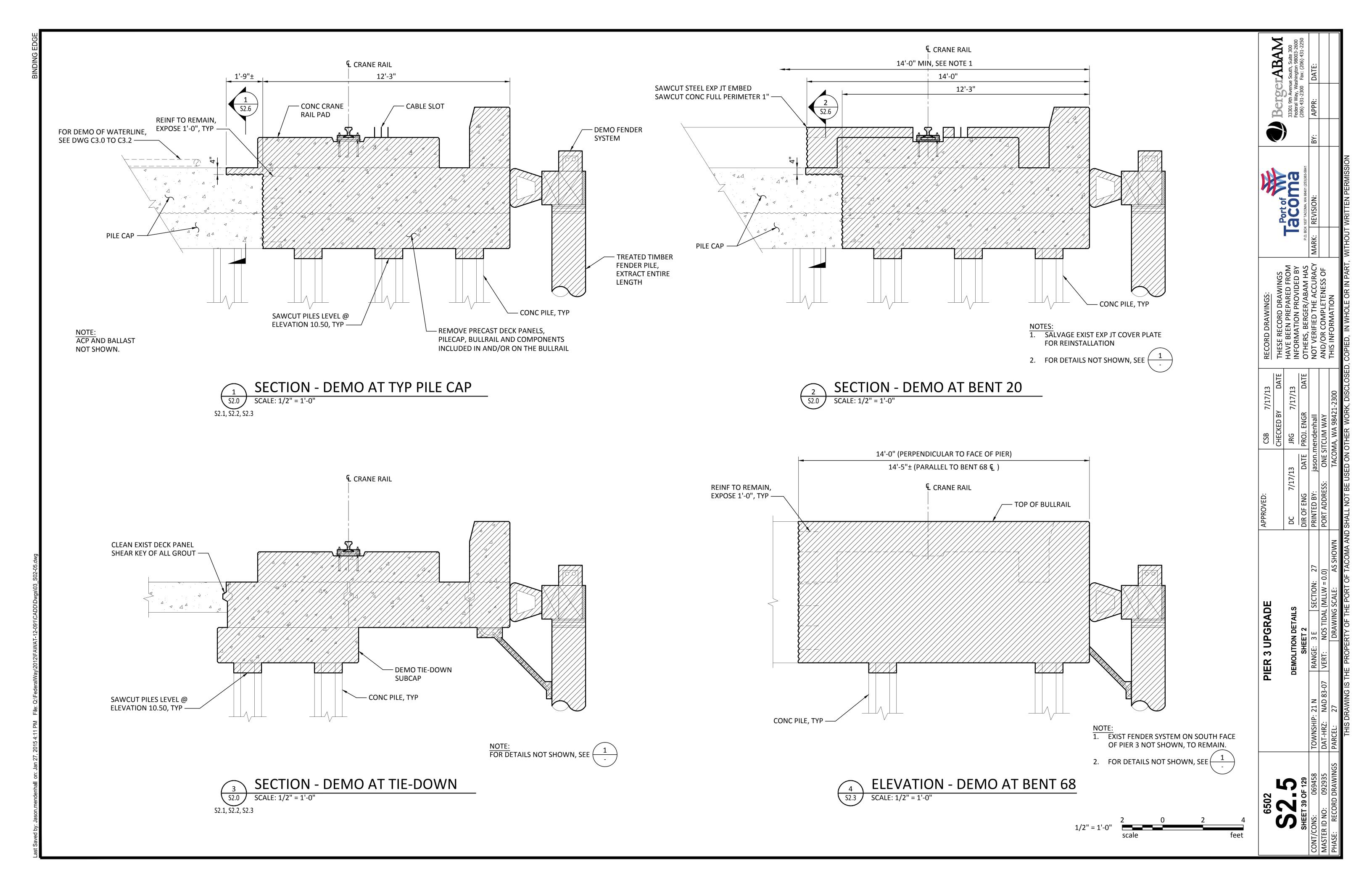
- DRILL OR GRIND EXPOSED ENDS
OF REINF, ANCHOR BOLT, OR
EMBED TO 1" BELOW FINISH
SURFACE AND FILL RESULTING
RECESS WITH EPOXY GROUT

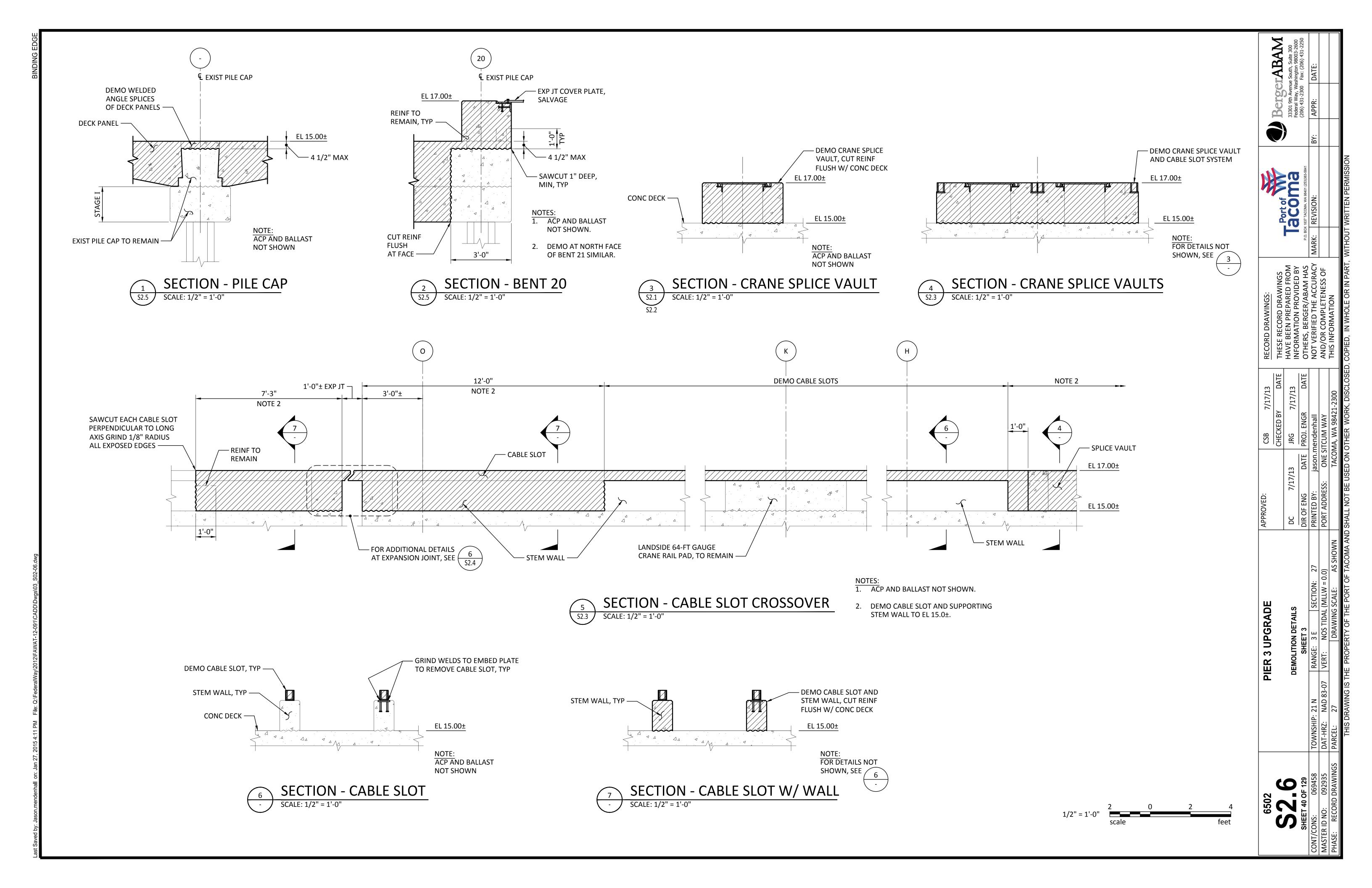


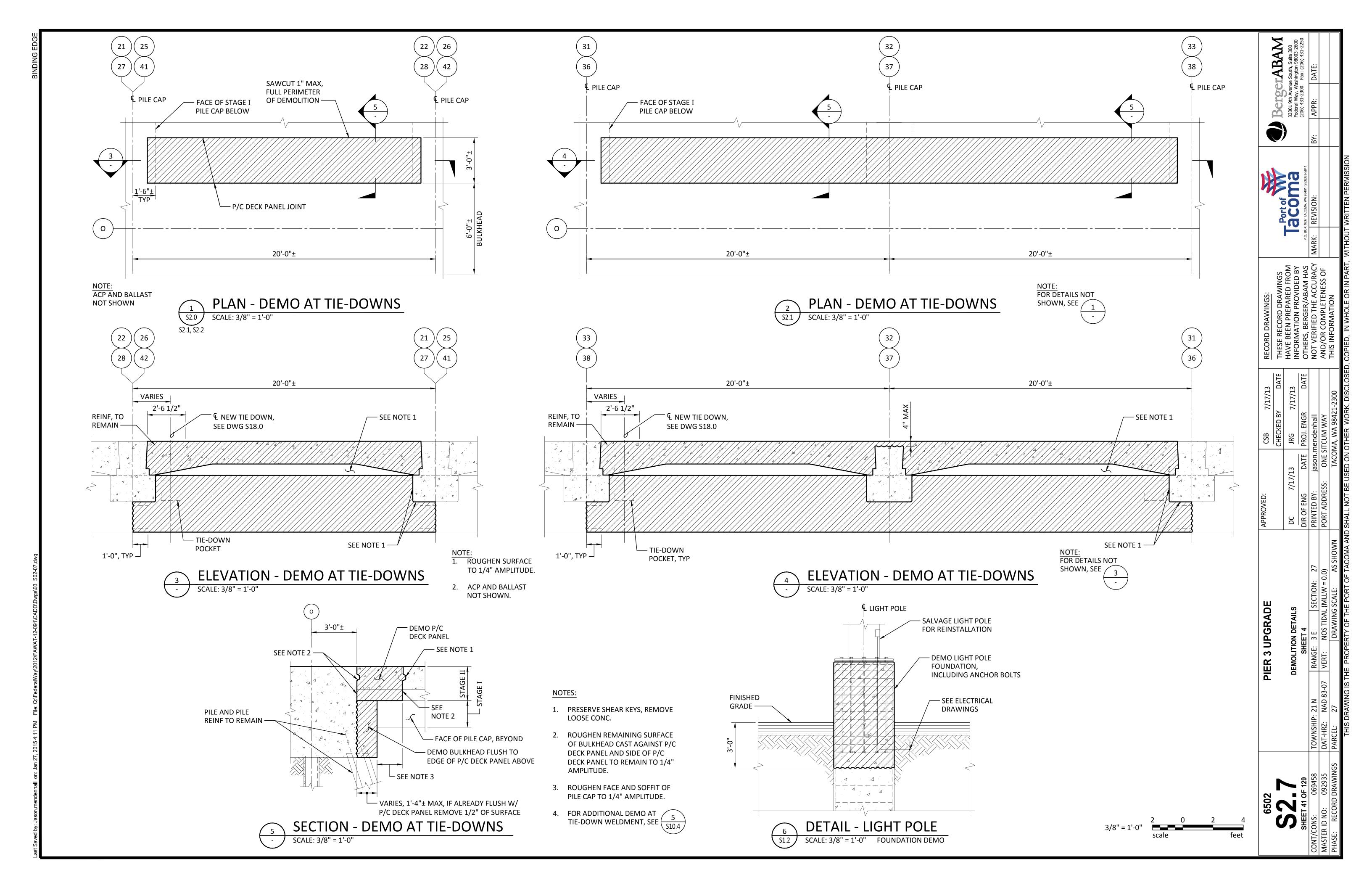
1-1/2" = 1'-0"	8 scale	0	8	16 inches
1/2" = 1'-0"	2 scale	0	2	4 feet

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7								CHECKED BY	DATE	THESE RECORD DI
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ET 38 OF 129		SHE	SHEET 1			DIR OF ENG	DATE	DATE PROJ. ENGR	DATE	OTHERS, BERGER
069458	TOWNSHIP: 21 N	RANGE:	RANGE: 3 E SECTION: 27	SECTION:	27	PRINTED BY:	jason.n	jason.mendenhall		NOT VERIFIED THE
0: 092935	DAT-HRZ: NAD 83-07 VERT: NOS TIDAL (MLLW = 0.0)	VERT:	NOS TIDAL	(MLLW = (0.0)	PORT ADDRESS:		ONE SITCUM WAY		AND/OR COMPLE
CORD DRAWINGS PARCEL: 27	PARCEL: 27		DRAWING	SCALE:	DRAWING SCALE: AS SHOWN		TACON	TACOMA, WA 98421-2300	-2300	THIS INFORMATIC
	THIS DRAWING IS T	HE PROPE	RTY OF TH	E PORT C	F TACOMA AND	SHALL NOT BE	USED ON (THER WOR	K. DISCLOSEI	THIS DRAWING IS THE PROPERTY OF THE PORT OF TACOMA AND SHALL NOT BE USED ON OTHER WORK, DISCLOSED, COPIED. IN WHOLE

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SECTION - DEMO AT CROSSOVER

SAWCUT 1" DEEP TOP AND SIDES OF CRANE RAIL PAD.

3. ACP AND BALLAST NOT SHOWN.

⊈ BULK HEAD DEMO RAIL AND CRANE RAIL PAD - LANDSIDE FACE OF BULKHEAD 25'-0" 5'-9"± 8'-0" SEE NOTE 1 — — SEE NOTE 1 3'-0" 3'-0" SEE NOTE 2 — — SEE NOTE 2 EL 17.0± EL 15.0± REINF TO REMAIN, TYP NOTE: FOR DETAILS NOT PIER 3 SHOWN, SEE CRANE BEAM

SECTION - DEMO AT CROSSOVER (BULKHEAD)

SCALE: 1/2" = 1'-0"

CRANE RAIL PAD

EL 17.0±

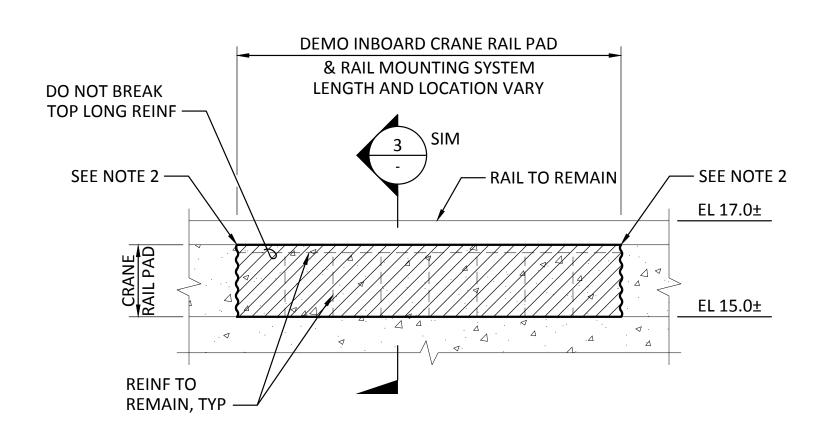
DEMO CRANE RAIL AND CRANE RAIL AND CRANE RAIL PAD FLUSH W/ TOP OF CONC DECK

NOTE:
ACP AND BALLAST NOT SHOWN

SCALE: 1/2" = 1'-0"

SECTION - DEMO AT CROSSOVER

SCALE: 1/2" = 1'-0"



NOTES:

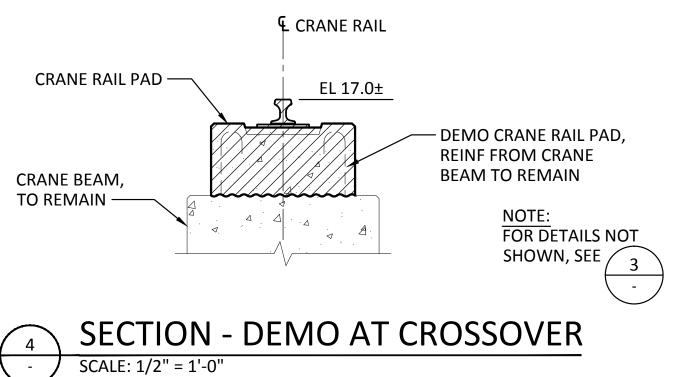
1. FOR DETAILS NOT SHOWN, SEE (

2. FOR LOCATIONS OF DEMO, SEE ELECTRICAL DWGS.

ELEVATION - DEMO AT INBOARD CRANE RAIL

S2.0 SCALE: 1/2" = 1'-0"

S2.1, S2.2, S2.3



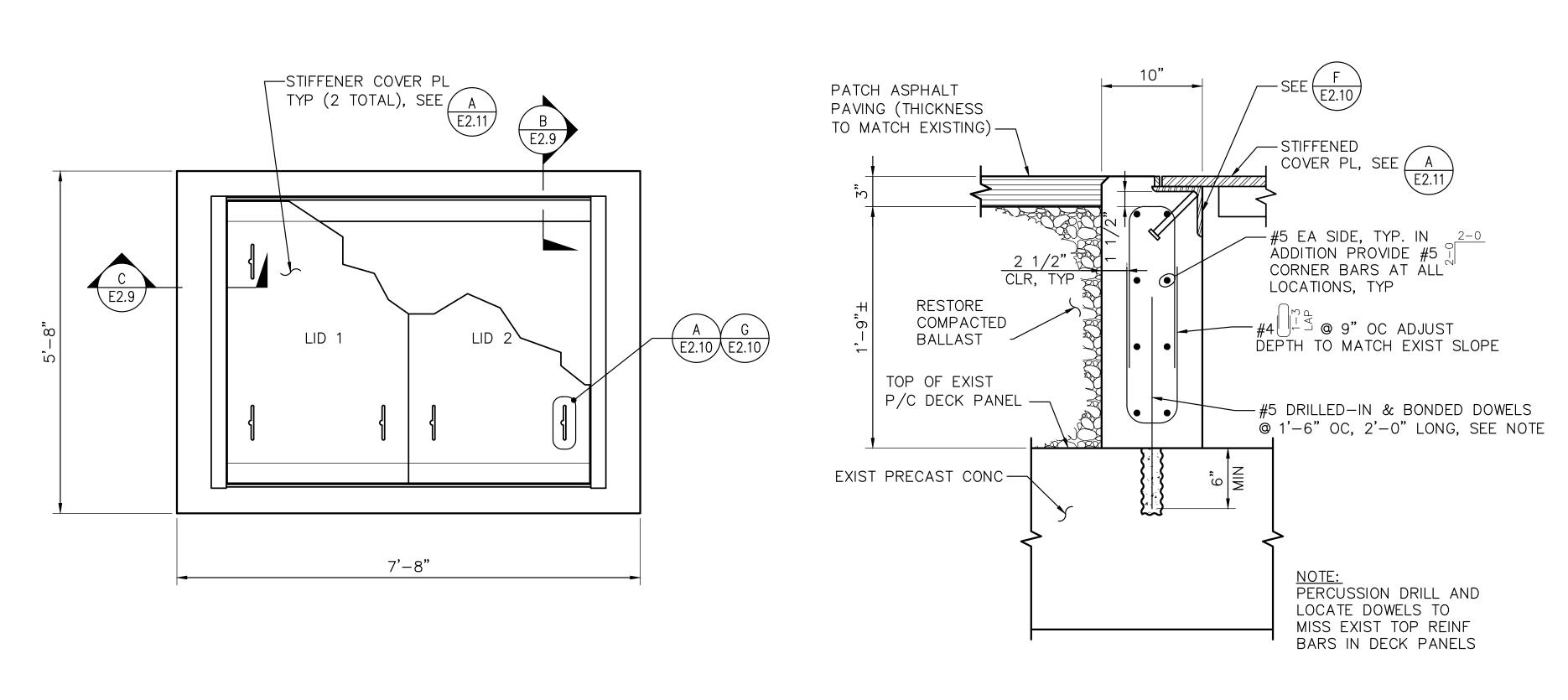
6502PIER 3 UPGRADEAPPROV328DEMOLITION DETAILSDCIEET 42 OF 129SHEET 5DCUS: 069458TOWNSHIP: 21 NRANGE: 3 ESECTION: 27DIR OF ENO: 092935DAT-HRZ: NAD 83-07VERT: NOS TIDAL (MILLW = 0.0)PORT ACRECORD DRAWINGSPARCEL: 27DRAWING SCALE: AS SHOWNPORT AC

BergerABAM

EMO AT CROSSOVER

CRANE POWER TEMPORARY CONNECTION VAULT ELECTRICAL LAYOUT

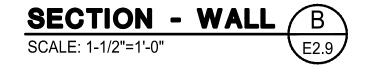
SCALE: 1"=1'-0"



PLAN - TEMP CRANE POWER VAULT

SCALE: 3/4"=1'-0"

E2.9

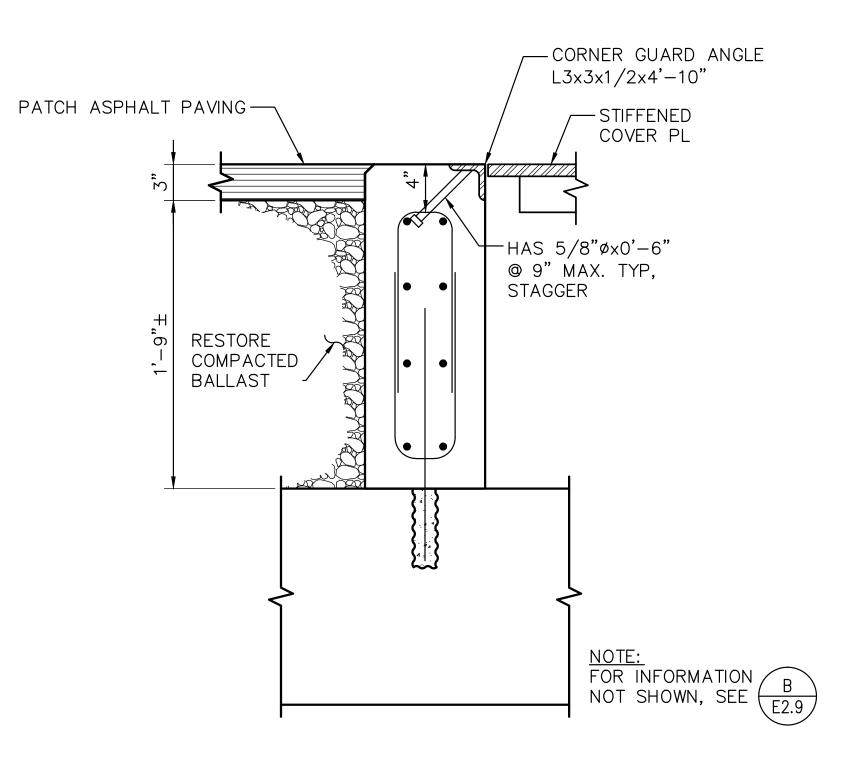


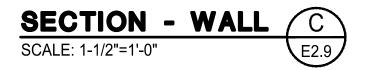
GENERAL NOTES:

1. SEE SHEET E2.0 FOR GENERAL NOTES.

CONSTRUCTION NOTES:

- CONTRACTOR SHALL COORDINATE WITH ENGINEER AND FIELD VERIFY EXACT LABELING AND IDENTIFICATION FOR EXISTING 5KV CRANES AND CABLES. PROVIDE NEW LABELS ON EXISTING 5KV SWITCHGEAR, CABLES (EXISTING AND NEW) AND JUNCTIONS TO MATCH FROM EXISTING 5KV SWITCHGEAR TO EXISTING CRANES.
- PROVIDE TWO (2) POSITION LOAD-BREAK JUNCTIONS MOUNTED TO CONCRETE VAULT WALLS AND ROTATE TO A 45 DEGREE POSITION FOR MOUNTING 5KV, 200A LOAD-BREAK ELBOWS. PROVIDE TWO (2) 5KV, 200A LOAD-BREAK ELBOWS AT EACH JUNCTION (TOTAL OF 12). CONTRACTOR WILL NEED TO INSTALL LOAD-BREAK ELBOWS ON NEW 5KV CABLES AND ON EXISTING 5KV CRANE CABLES. COOPER PRODUCTS OR EQUAL.
- PROVIDE GROUND BAR(S) WITH MULTIPLE SIZES GROUND HOLES MOUNTED TO VAULT WALL BELOW THE 5KV LOAD-BREAK JUNCTIONS. ALL GROUNDS WITHIN THE VAULT SHALL BE NEATLY ROUTED ALONG VAULT WALLS AND FLOOR.





	GLW/SLH 7/17/13	13	RECORD DRAWINGS:		1			
	CHECKED BY	DATE	THESE RECORD DRAWINGS		Port of		CROSS EN	CROSS ENGINEERS, INC
7/13	GLW 7/17/13	13	HAVE BEEN PREPARED FROM INFORMATION PROVIDED BY		acoma		6509 6th Avenue Tacoma, WA 98406 info@crossengineers.com	Phone: (253) 759-0118 Job Number: 11-071.1 m
DATE	DATE PROJ. ENGR	DATE	OTHERS, CROSS ENGINEERS HAS	P.O. BOX	P.O. BOX 1837 TACOMA, WA 98401 (253)383-5841			
jason.	jason.mendenhall		NOT VERIFIED THE ACCURACY	MARK:	MARK: REVISION:	BY:	APPR:	DATE:
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DETAILS

SECTION: 27

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DRAWING SCALE: AS SHOWN

ERTY OF THE PORT OF TACOMA AND SHALL NOT BE USED ON OTHER WOR

PIER 3 UPGRADE

TEMPORARY CRANE CONNECTION

VAULT DETAILS

TOWNSHIP: 21 N RANGE: 3 E SECTION
DAT-HRZ: NAD 83-07 VERT: NOS TIDAL (MI

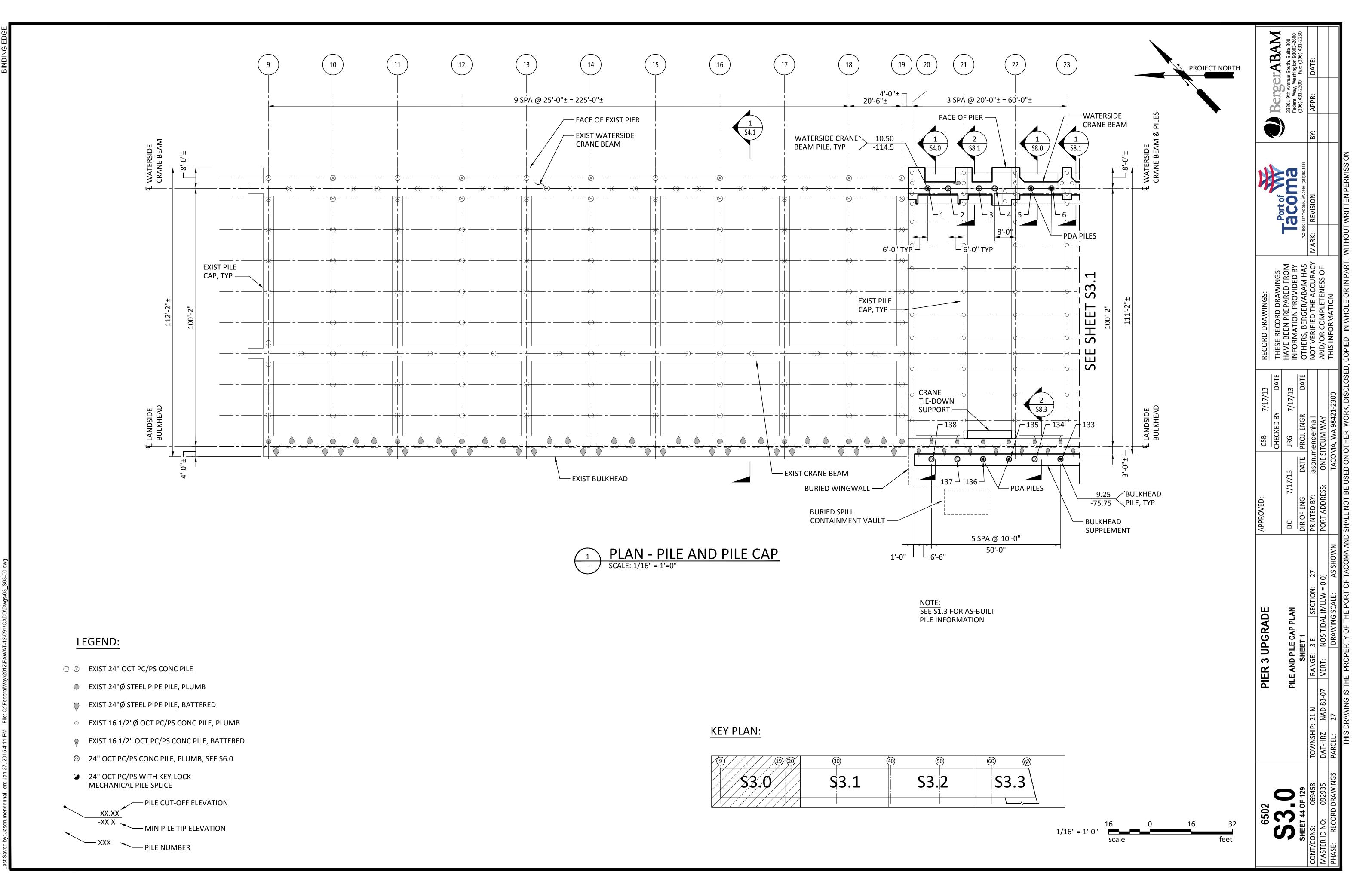
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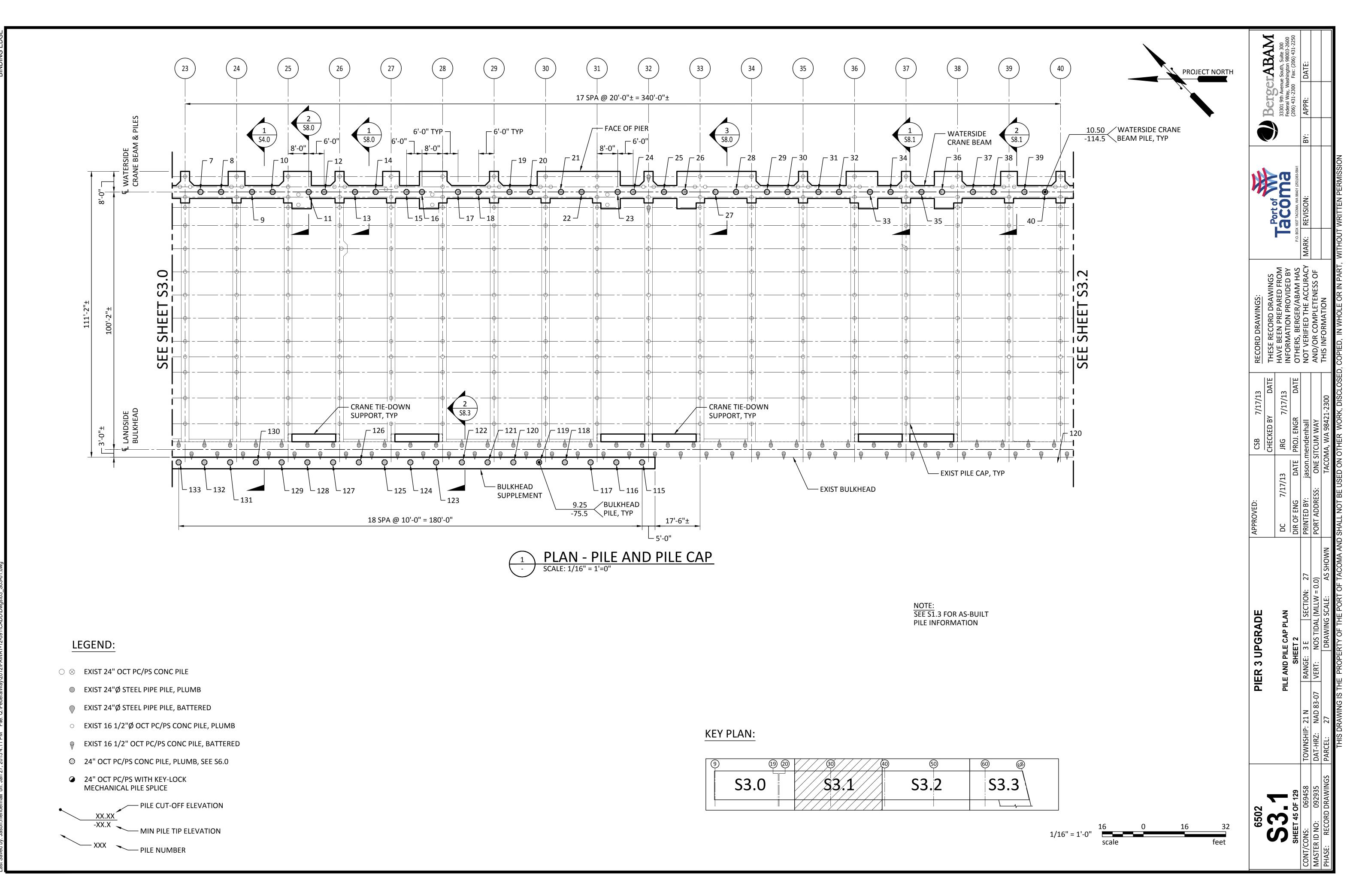
E2_9

SHEET 109 OF 129

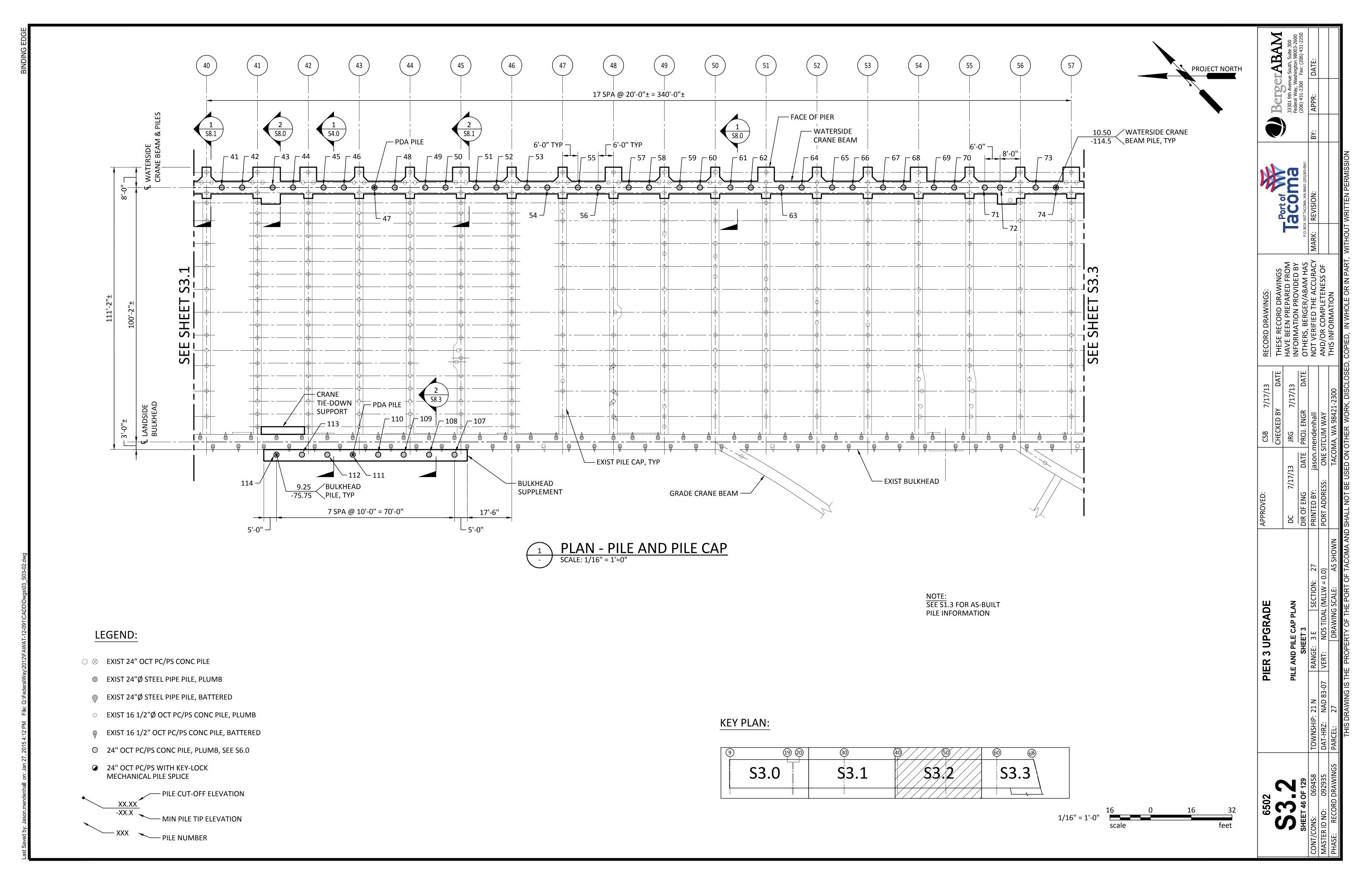
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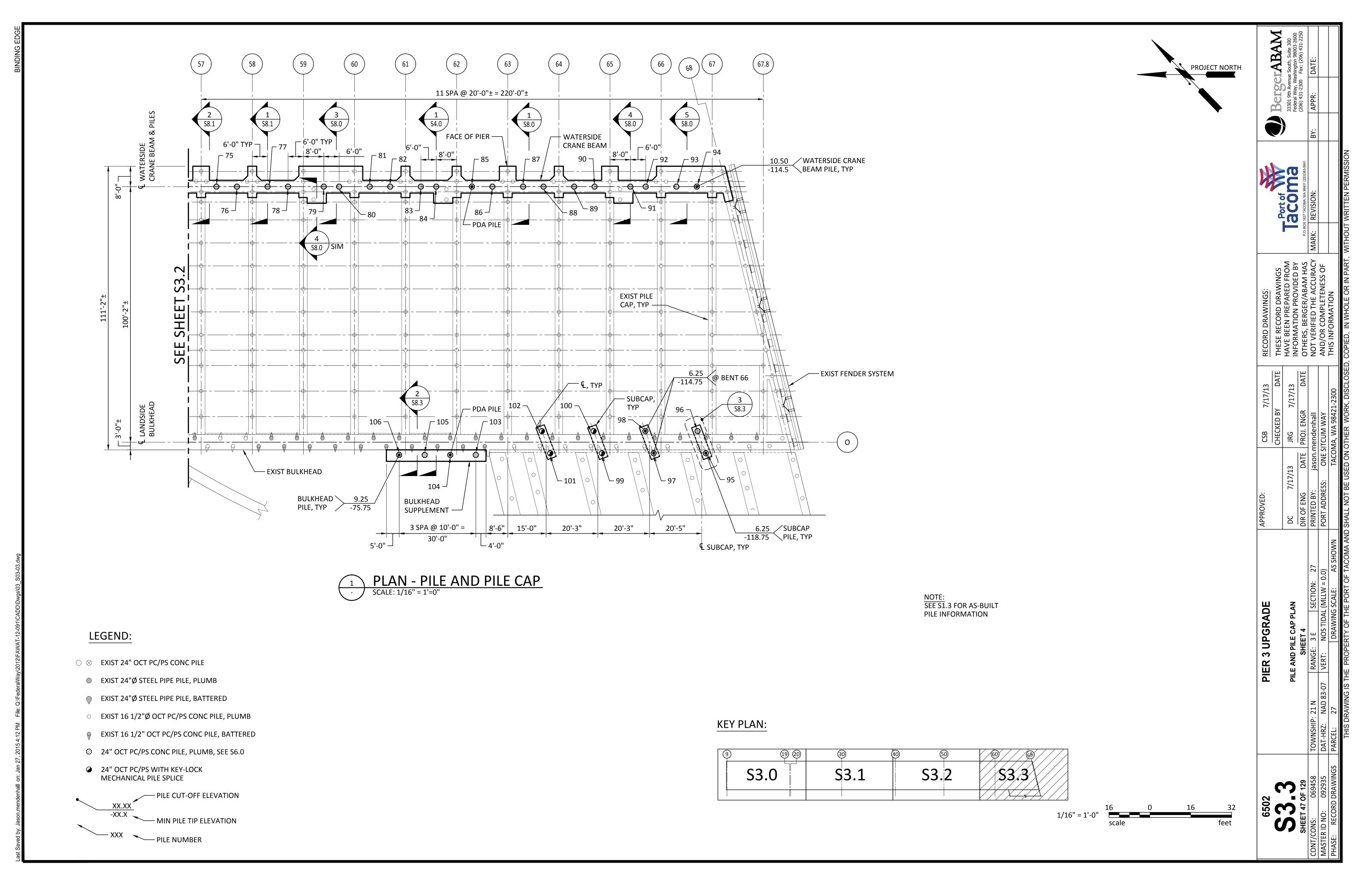
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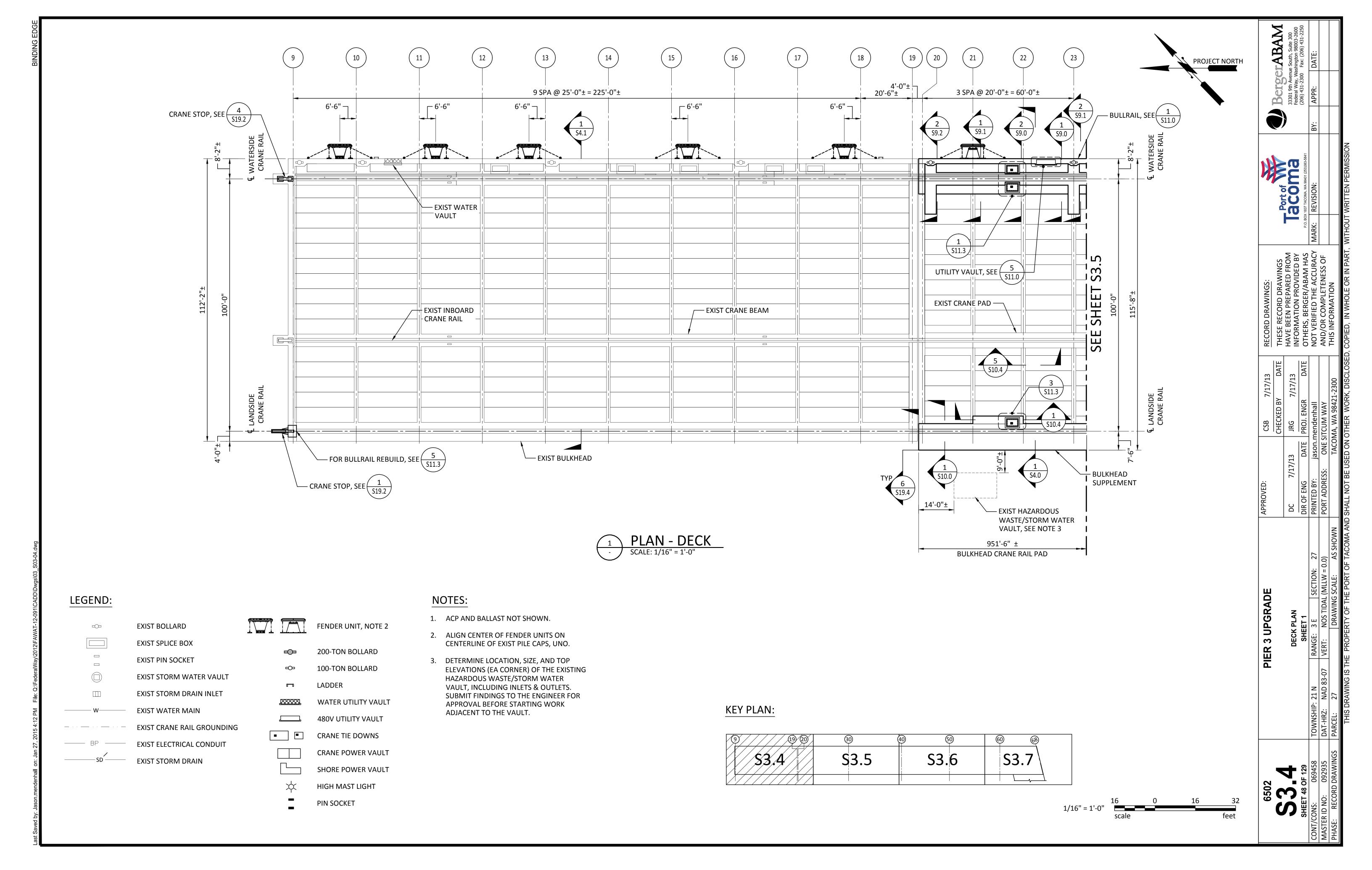


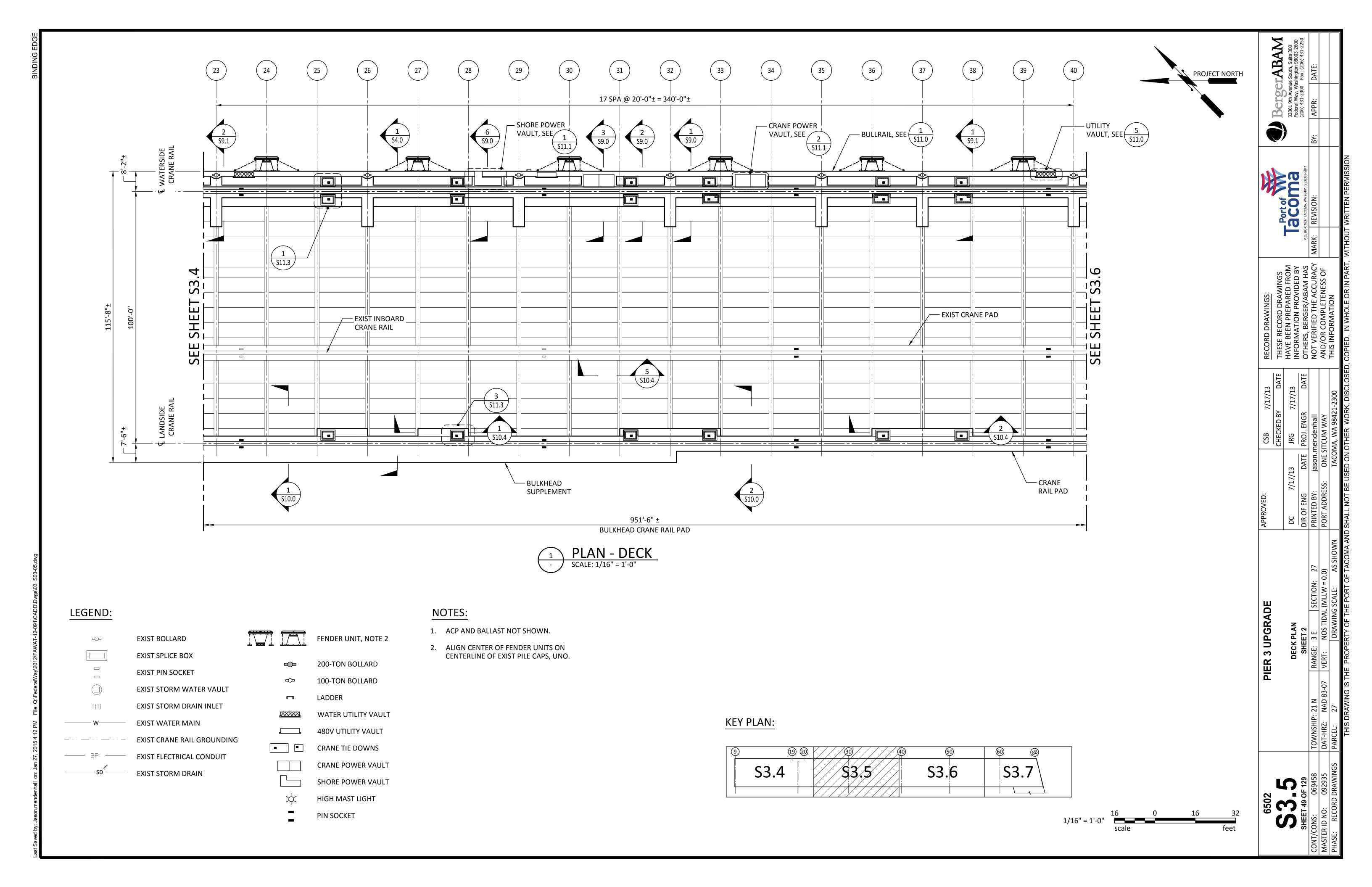


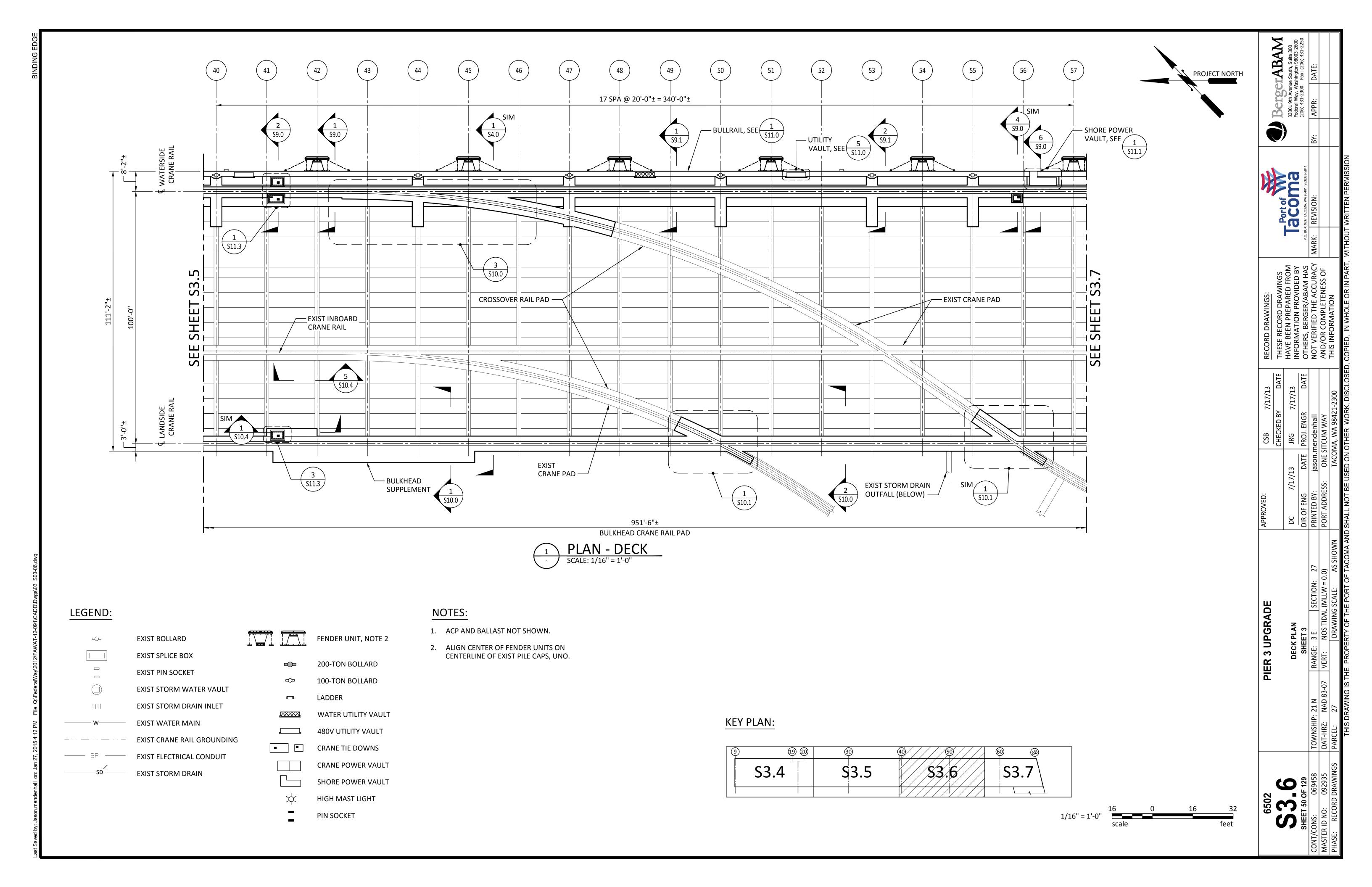
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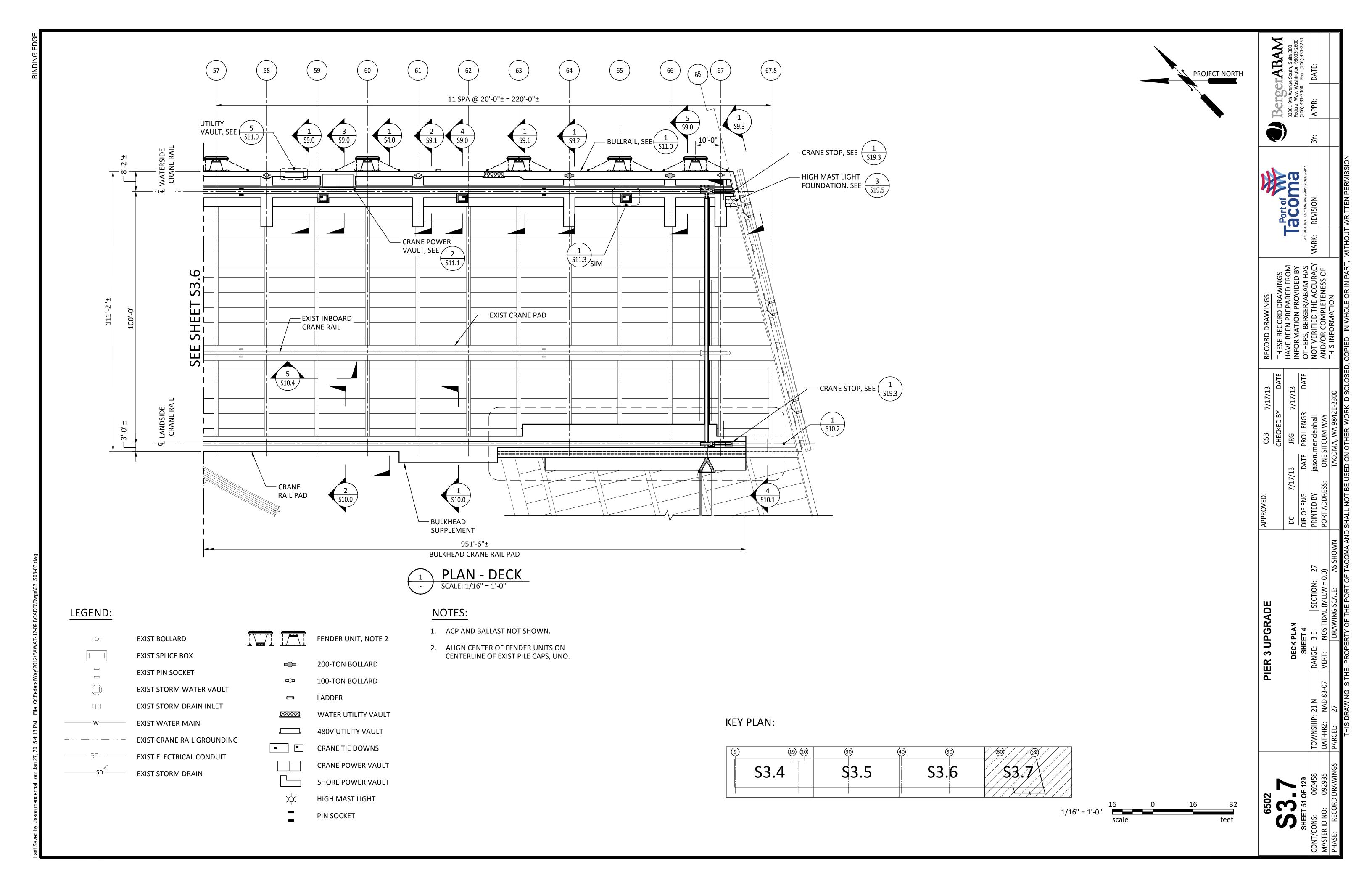


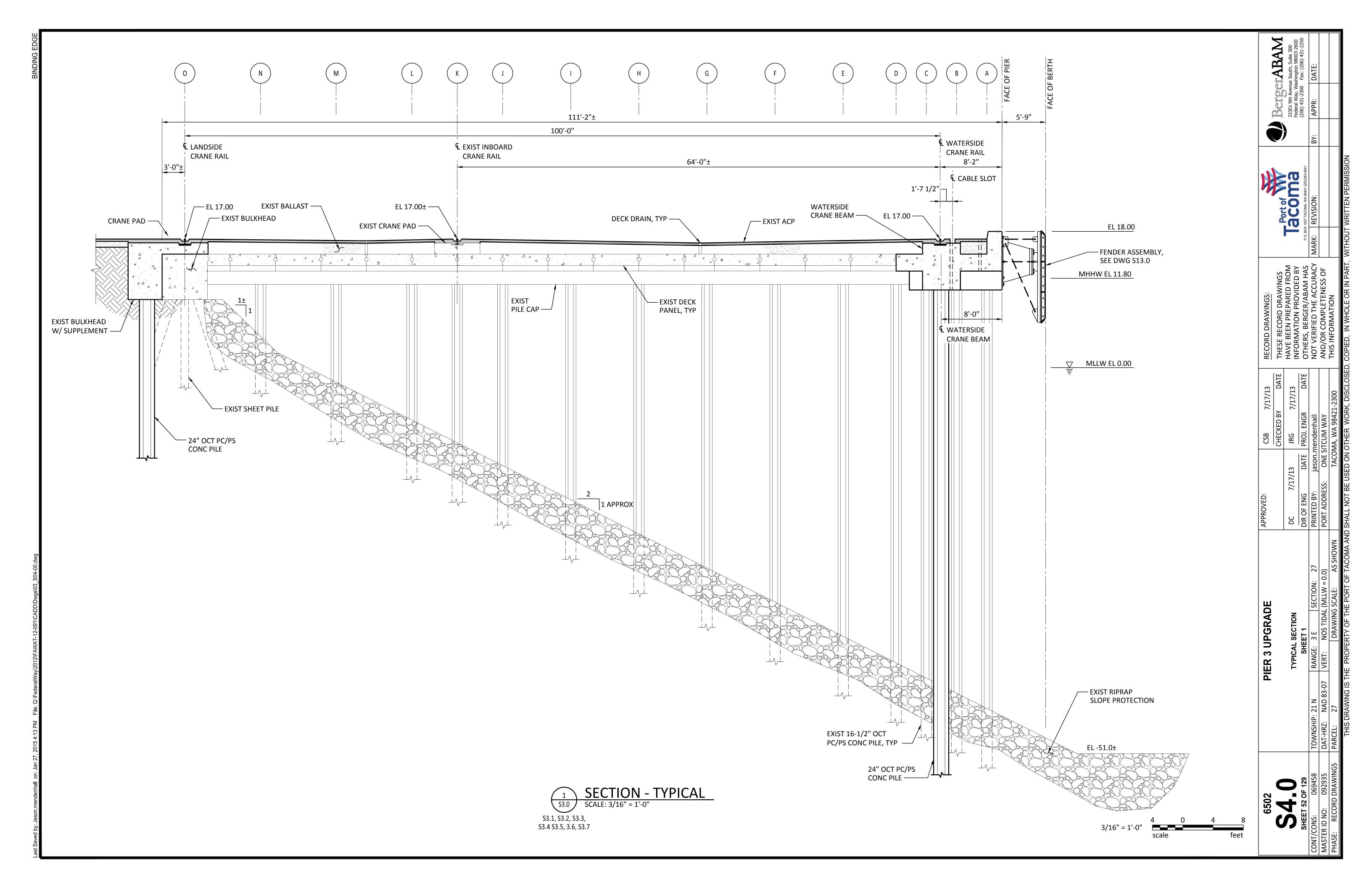


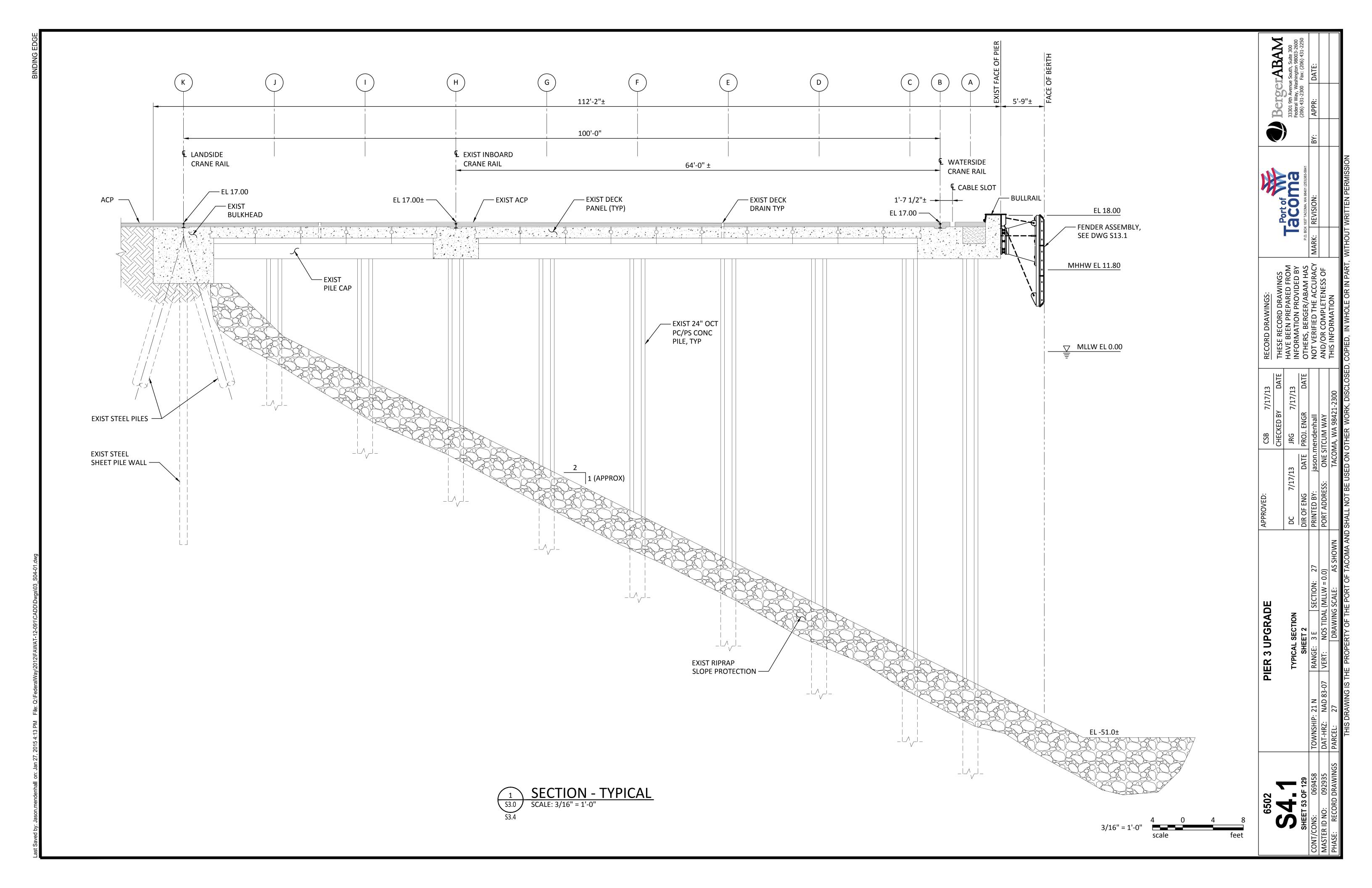


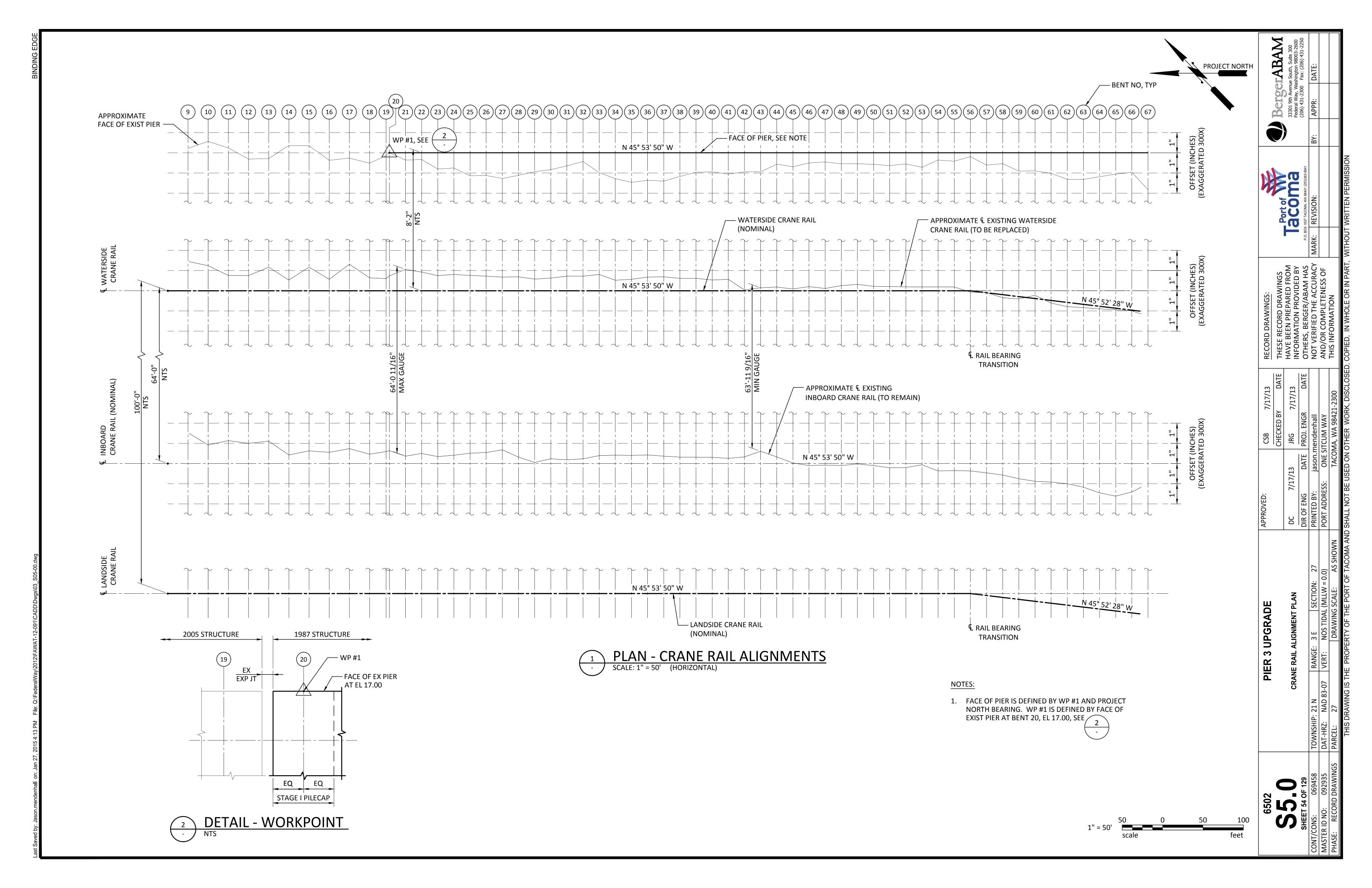


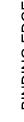


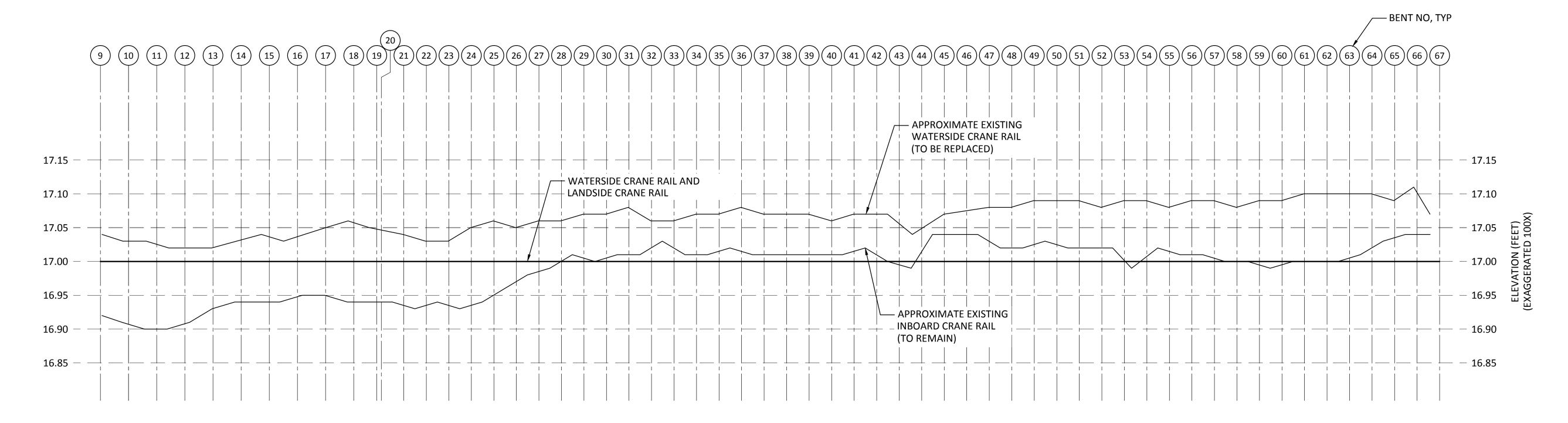












NOTE:

1. ELEVATIONS ARE TO TOP OF RAIL

PLAN - CRANE RAIL PROFILES

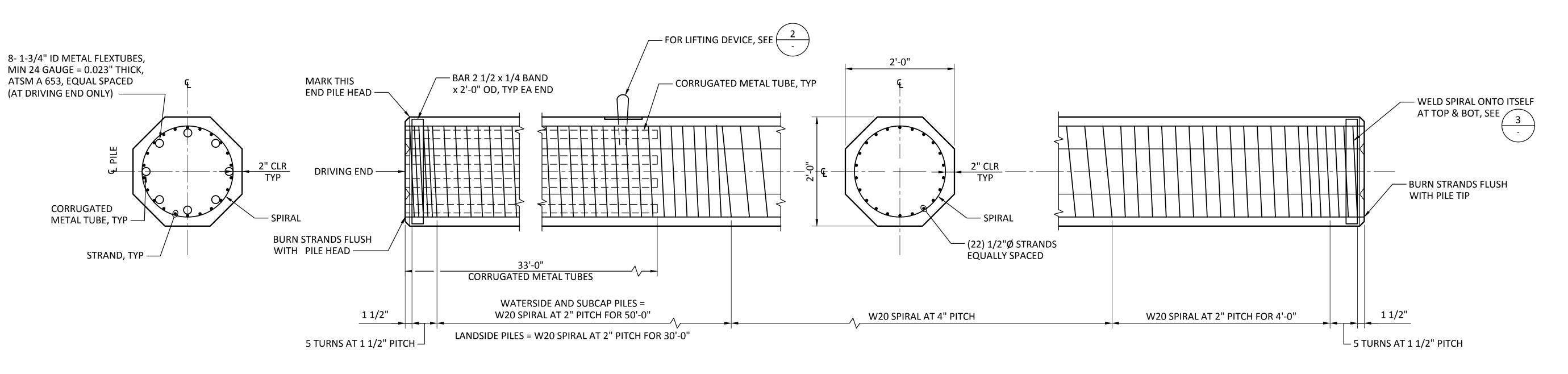
SCALE: 1" = 50' (VERTICAL)

1" = 50' 50 10 scale feet

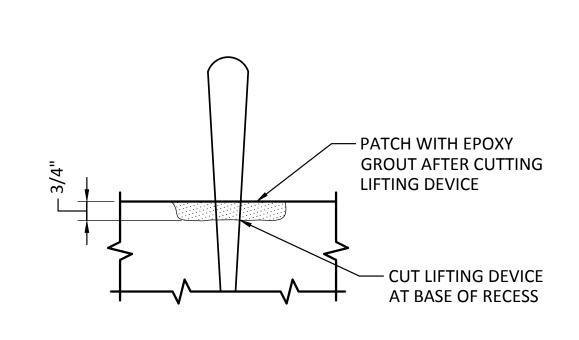
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	7									CHECKED BY	DATE	
りつ			_อ	CRANE RAIL PROFI	L PROFILES	S		DC 7/17/13	/13	JRG	7/17/13	
SHEET 5	SHEET 55 OF 129							DIR OF ENG	DATE	DATE PROJ. ENGR	DATE	
CONT/CONS:	069458	TOWNSHIP: 21 N	21 N	RANGE: 3 E		SECTION: 27	27	PRINTED BY:	jason.m	jason.mendenhall		
MASTER ID NO:	092935	DAT-HRZ:	DAT-HRZ: NAD 83-07	VERT:	NOS TIDAL (MLLW = 0.0)	(MLLW = 0)	(0)	PORT ADDRESS:	ONE SI	ONE SITCUM WAY		
PHASE: RECORD DRAWINGS	RD DRAWINGS	PARCEL:	27		DRAWING SCALE:	SCALE:	AS SHOWN		TACON	TACOMA, WA 98421-2300	1-2300	_
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33301 9th Avenue South, Suite 300
Federal Way, Washington 98003-2600
(206) 431-2300 Fax: (206) 431-2250

Tacoma

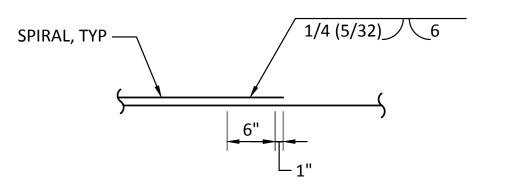


24" SOLID OCTAGONAL PRESTRESSED CONCRETE PILE SCALE: 1" = 1'-0"

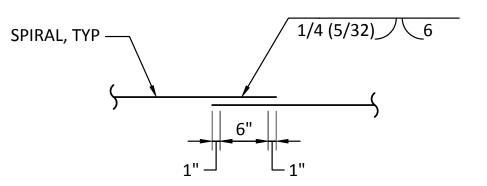


NOTE: LIFTING DEVICE QUANTITY AND LOCATIONS SHALL BE DETERMINED BY THE CONTRACTOR CONSIDERING ALL LOADS IMPARTED TO THE PILE (NORMAL AND PARALLEL TO THE LONGITUDINAL AXIS) BY THE RIGGING BEING USED. ALL MODES OF LIFTING (FABRICATION AND FIELD) SHALL BE CONSIDERED.





DETAIL - WELD DETAIL AT END SCALE: NTS



WELD ALL SPIRAL SPLICES IN THE TOP 1/3 OF THE PILE. LAPPED SPLICES WITH 2 OVER-LAPPING TURNS AND 135° HOOKS EXTENDING COMPLETELY ACROSS THE CORE MAY BE USED IN THE BOTTOM 2/3 OF THE PILE.



PRECAST PRESTRESSEI (SOLID OCTAGONAL PIL	
SIZE SECTION AREA WEIGHT SECTION MODULUS	24" SOLID 477 SQ IN 525 LBS/FT 1515 CUBIC IN
PRETENSION: NUMBER OF STRANDS DESIGN FORCE PRESTRESS IN CONCRETE	24" SOLID 22 555 KIPS 1160 PSI
MIN CONCRETE COMPRESSIVE STRENGTH: AT RELEASE AT DRIVING AT 28 DAYS	4800 PSI 8000 PSI 8000 PSI
PRESTRESSING STEEL	1/2" DIA 7-WIRE LOW RELAXATION STRAND
SPIRAL	ASTM A82 SPACING/DRAWING

MARK DRIVING END OF ALL PILES CLEARLY IN PRECAST PLANT.

PILING S	SCH	EDULE	
(24" SOLID OCTAGO	ONAL (CONCRETE PILE	S)
INSTALLED LOCATIONS	QTY	SUPPLIED LENGTH (FT)	ULTIMATE DEMAND
WATERSIDE CRANE BEAM	93	130'-0"	650 KIPS
SUBCAP @ 66	2	130'-0"	650 KIPS
SUBCAP @ 67	2	138'-0"	650 KIPS
WATERSIDE CRANE BEAM NEAR BENT 67	1	138'-0"	650 KIPS
BULKHEAD SUPPLEMENT	36	90'-0"	500 KIPS

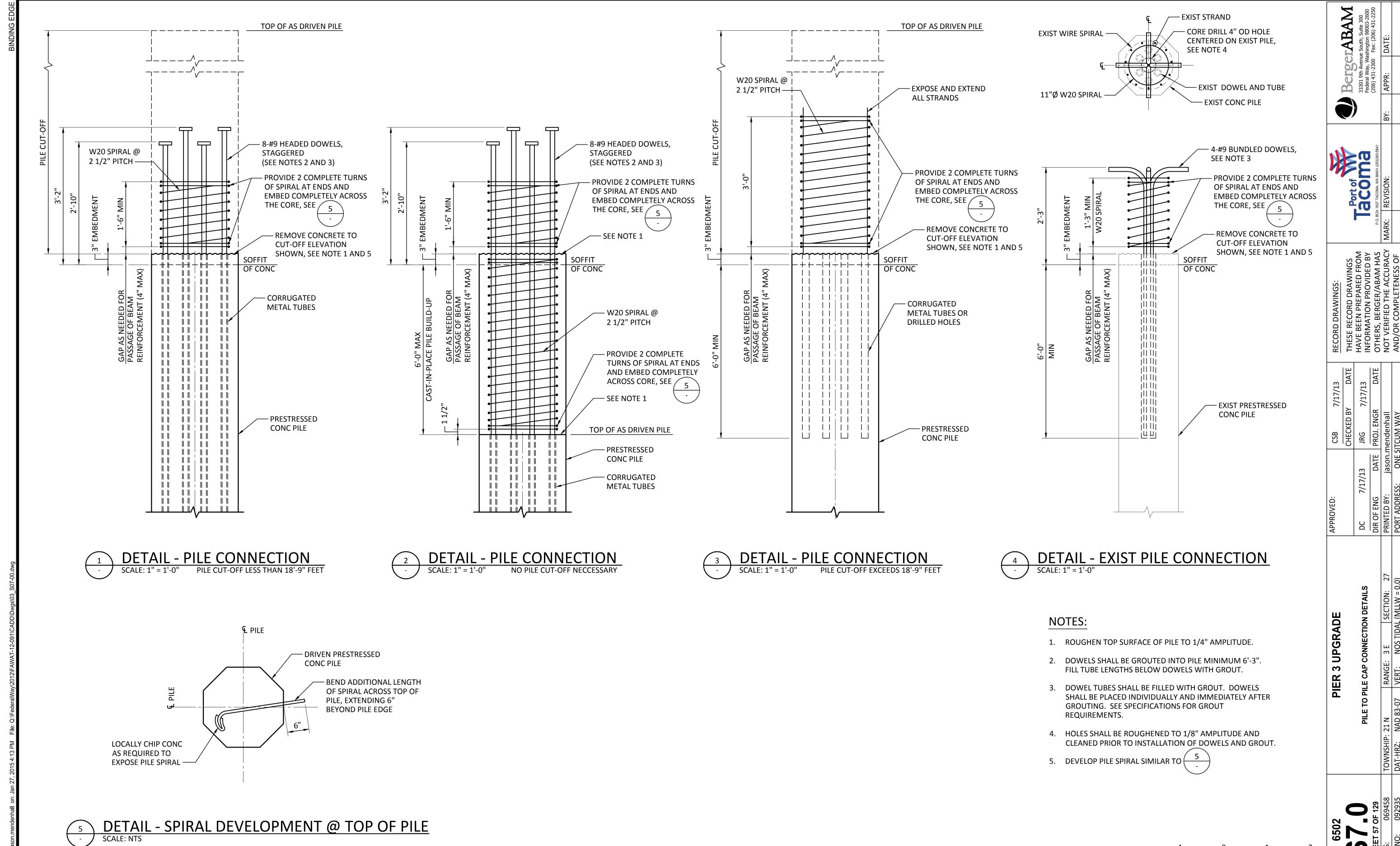
PILING :	SCH	EDULE	
(24" SOLID OCTAG	ONAL (CONCRETE PILE	S)
INSTALLED LOCATIONS	QTY	SUPPLIED LENGTH (FT)	ULTIMATE DEMAND
WATERSIDE CRANE BEAM	93	130'-0"	650 KIPS
SUBCAP @ 66	2	130'-0"	650 KIPS
SUBCAP @ 67	2	138'-0"	650 KIPS
WATERSIDE CRANE BEAM NEAR BENT 67	1	138'-0"	650 KIPS
BULKHEAD SUPPLEMENT	36	90'-0"	500 KIPS

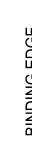
= 1'-0"	6502 S660 SHEET 56 OF 128 CONT/CONS: 06943 MASTER ID NO: 09293 PHASE: RECORD DRAW
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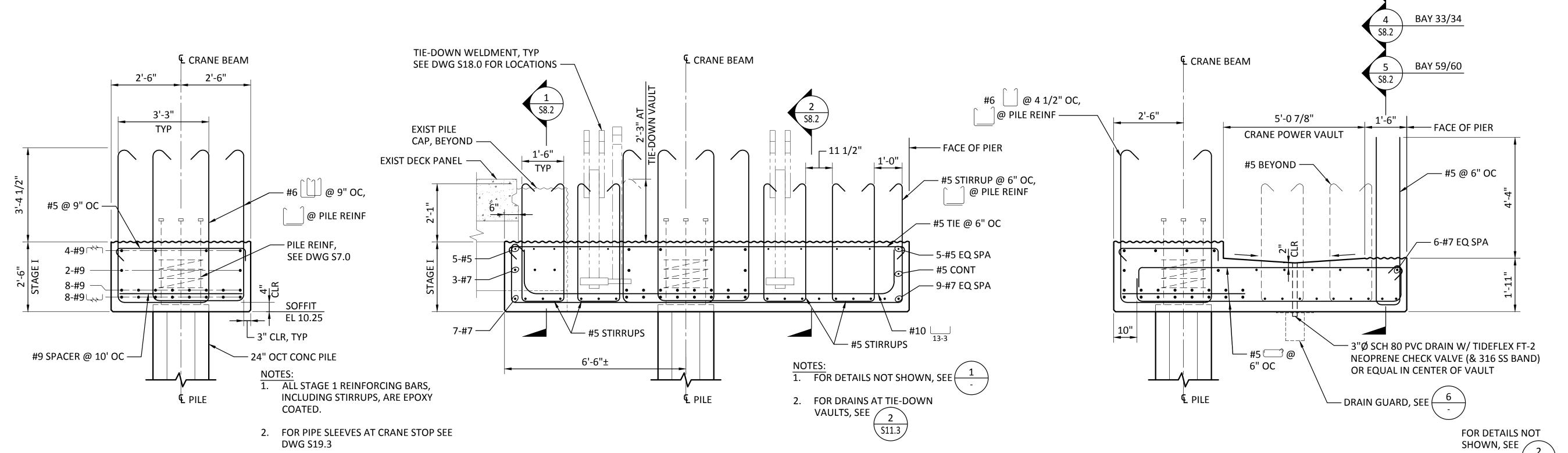
PIER 3 UPGRADE

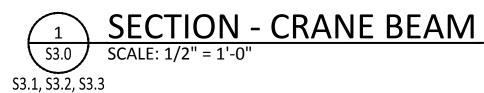
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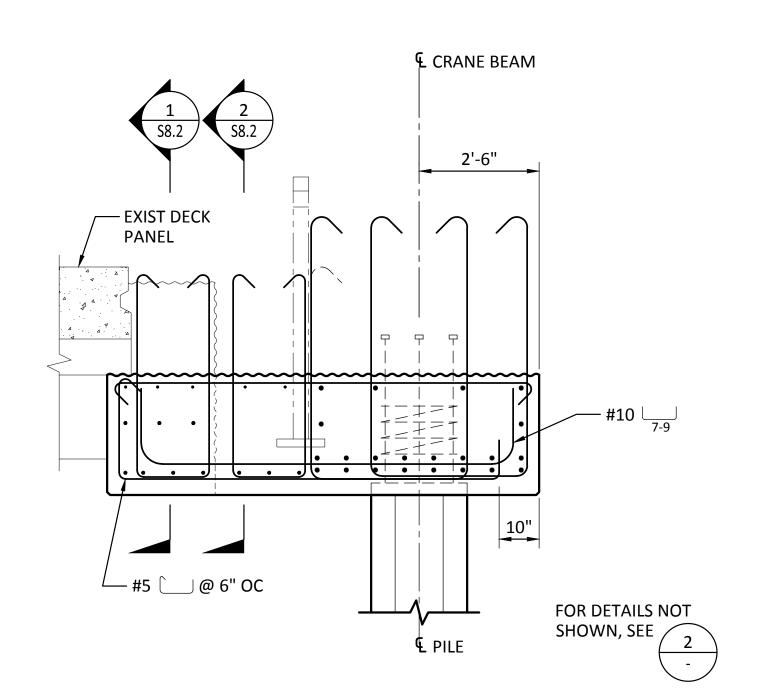
Tacoma







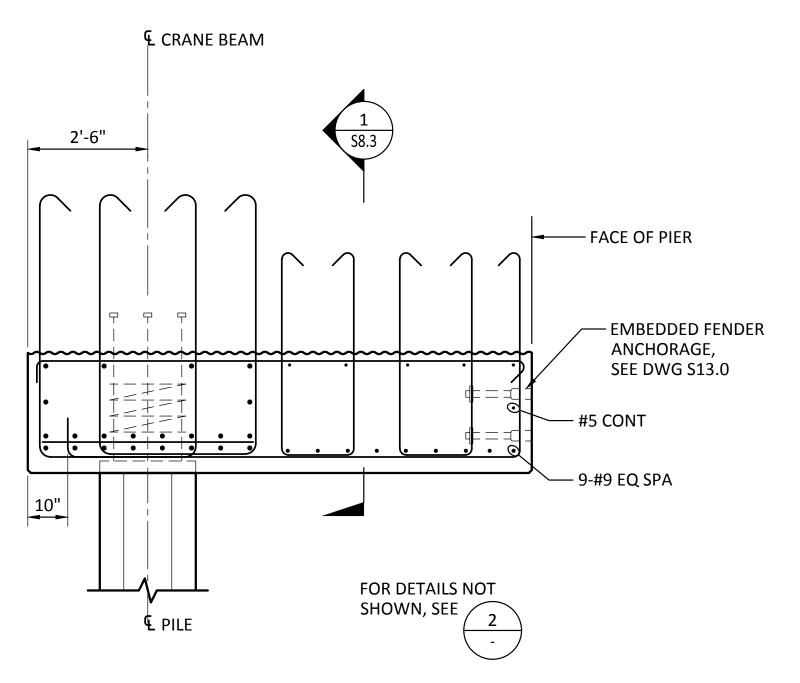




SECTION - TIE-DOWN (SINGLE)

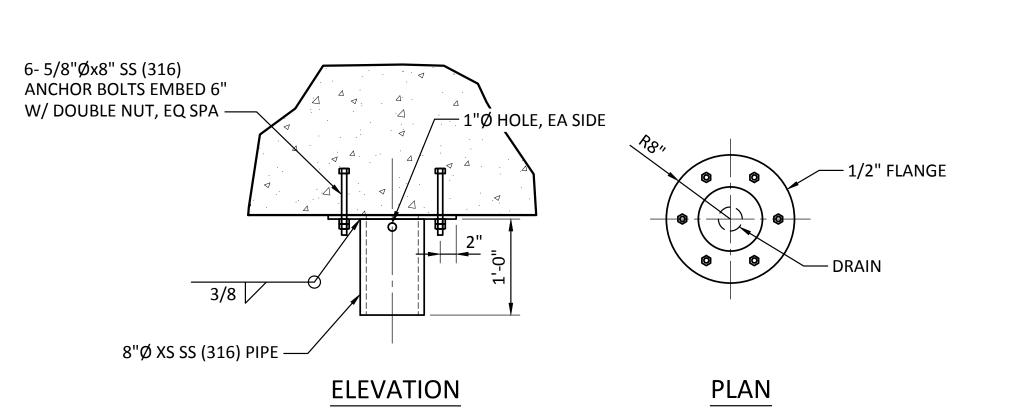
SCALE: 1/2" = 1'-0"

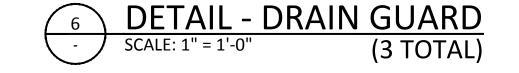
SECTION - TIE-DOWN (PAIR) SCALE: 1/2" = 1'-0"

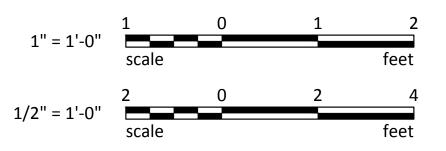


5 SECTION - BAY 66 - 68 SCALE: 1/2" = 1'-0"

SECTION - CRANE POWER VAULT S3.1 SCALE: 1/2" = 1'-0"

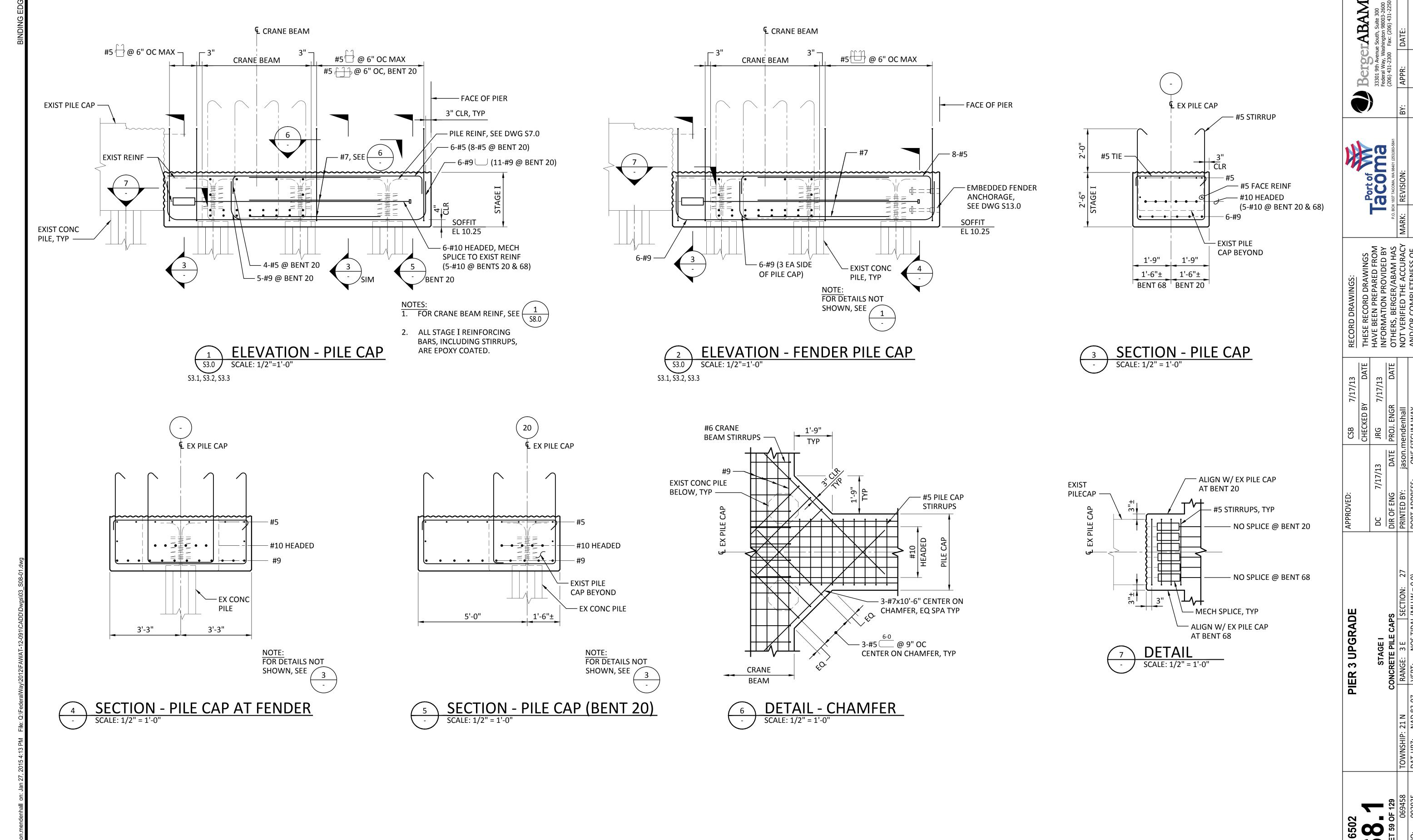




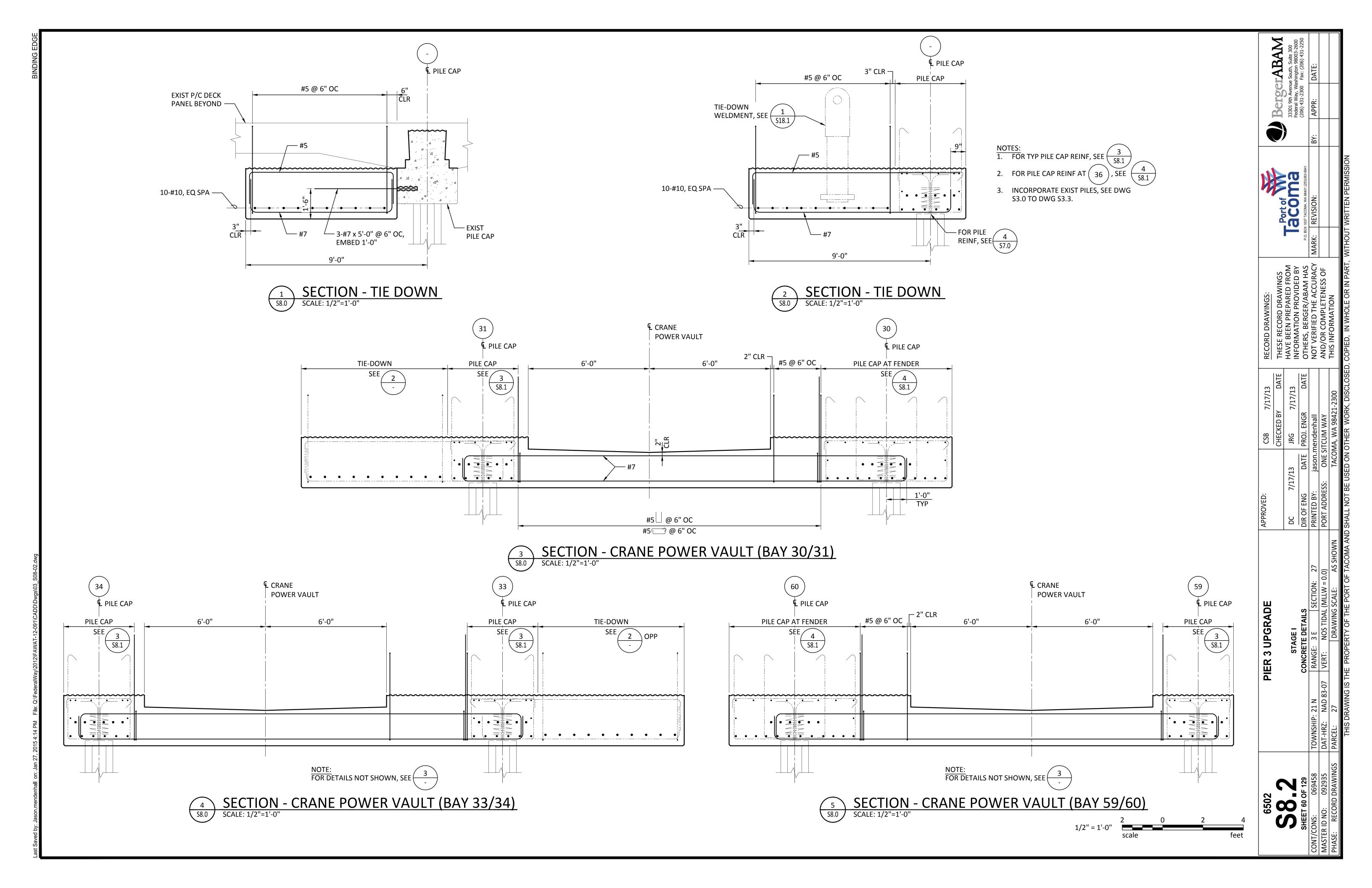


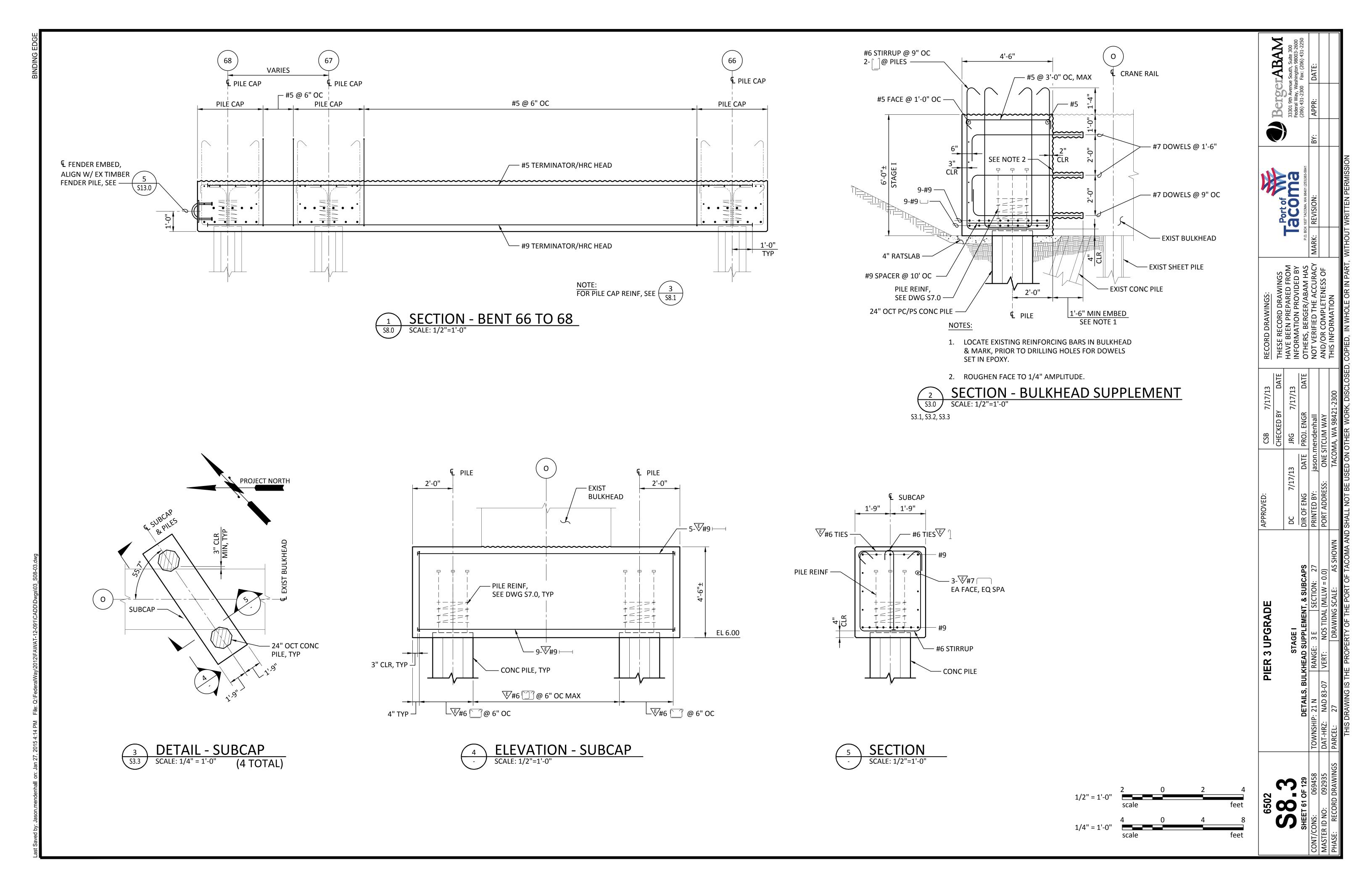
BAY 30/31

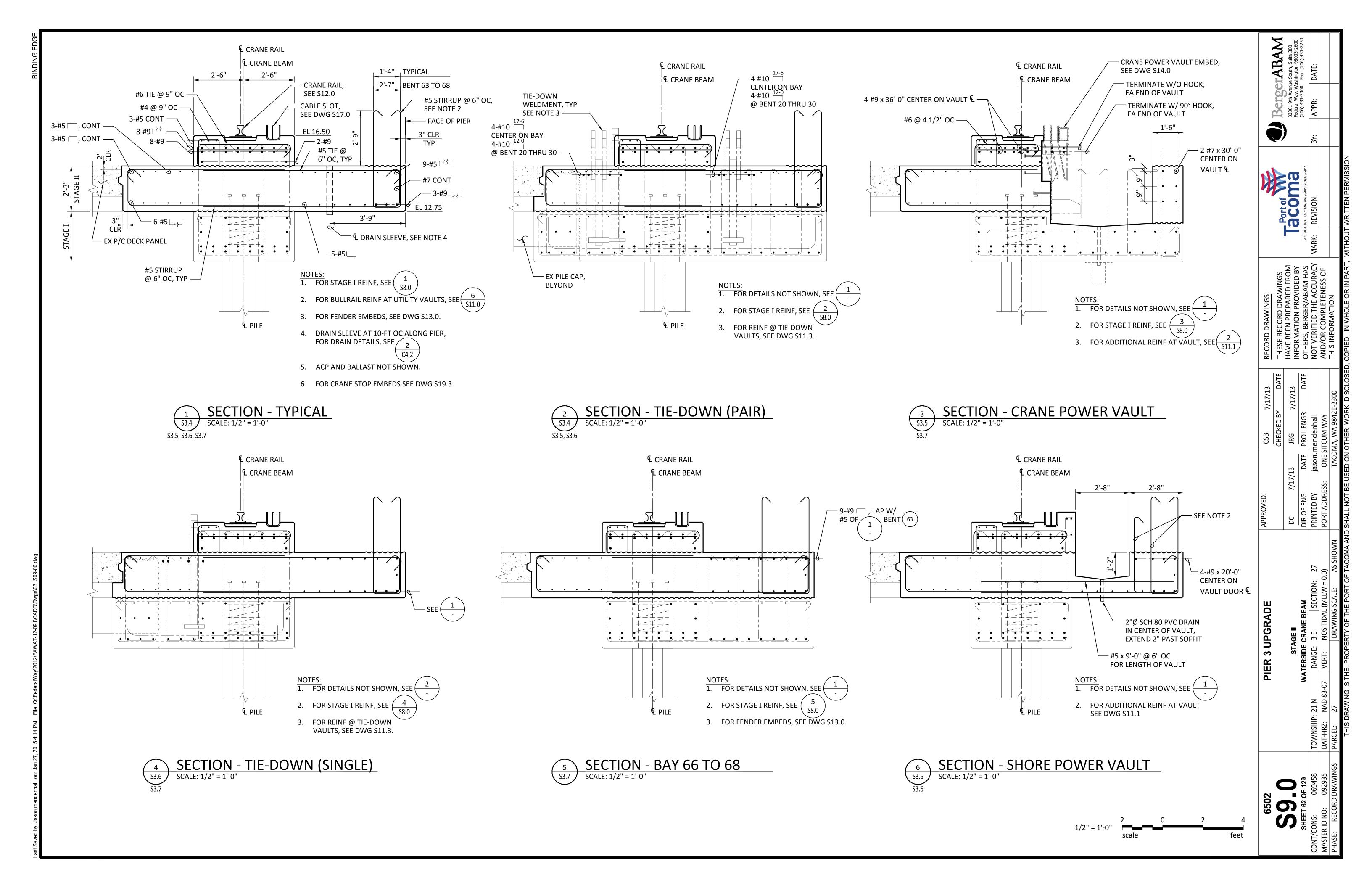
STAGE I CONCRETE WATERSIDE CRANE BEAM TOWNSHIP: 21 N DAT-HRZ: NAD 83-07 VERT: NOS TIDAL (MLLW = 0.0)

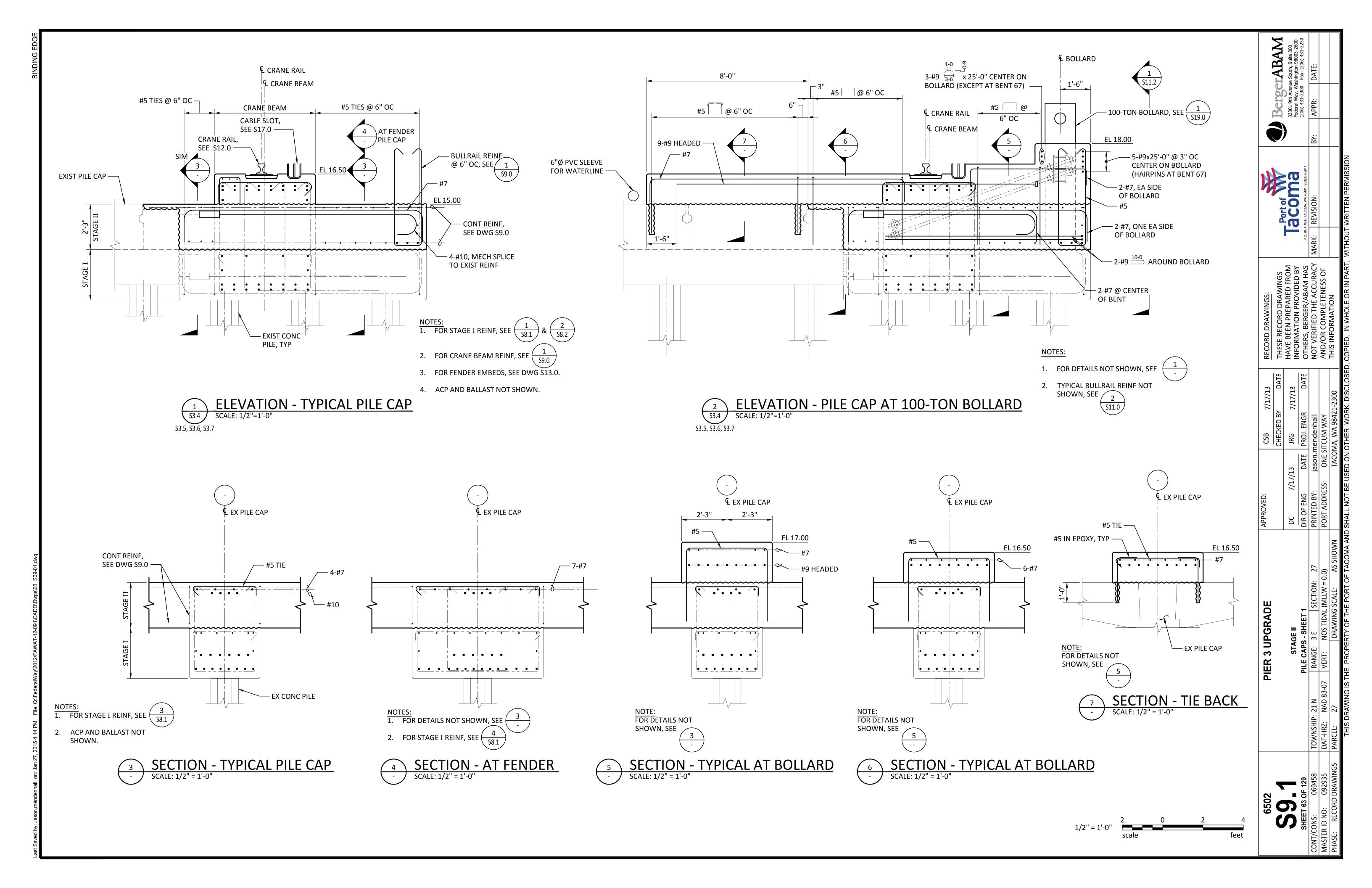


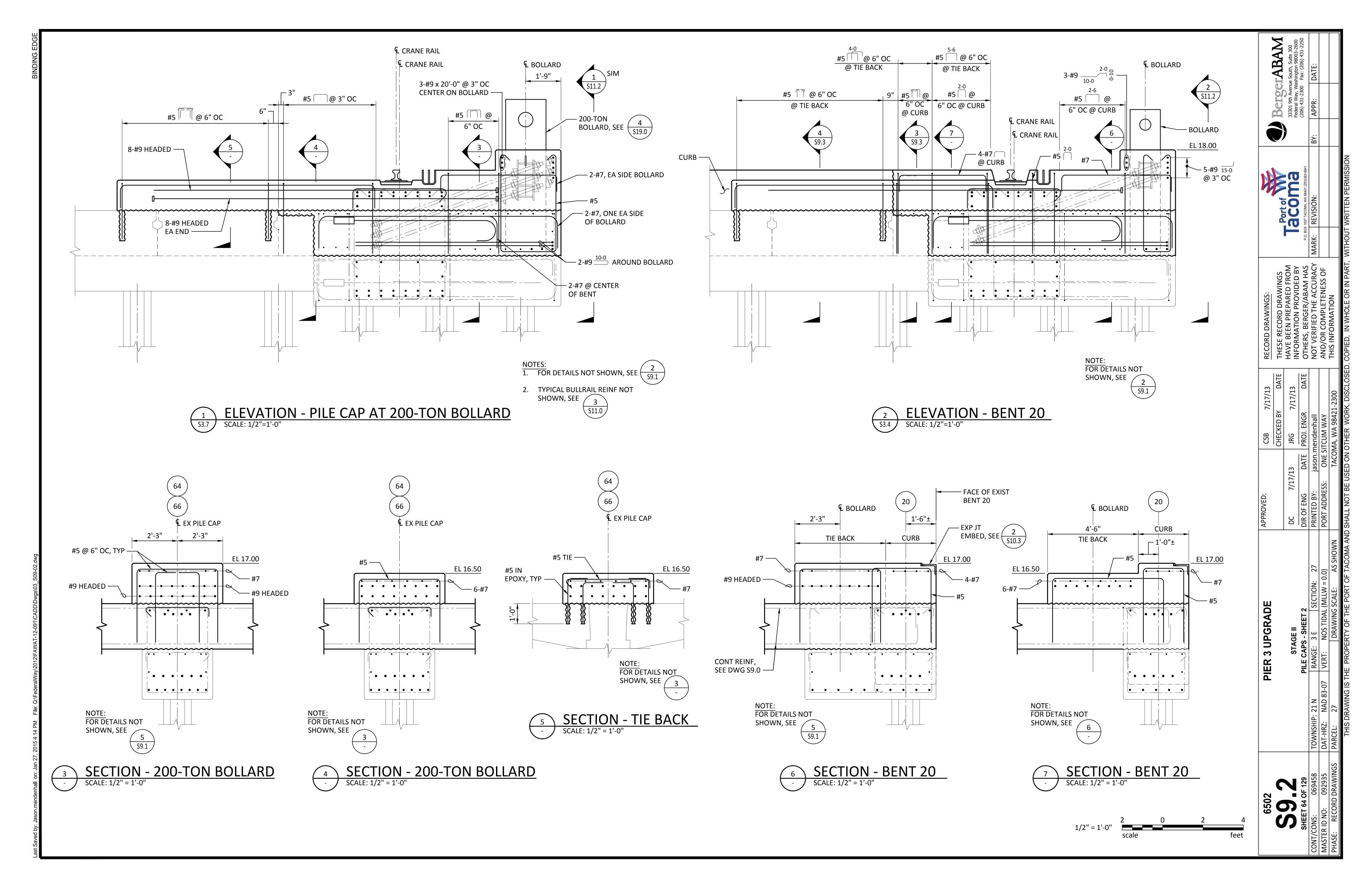
1/2" = 1'-0"

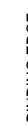


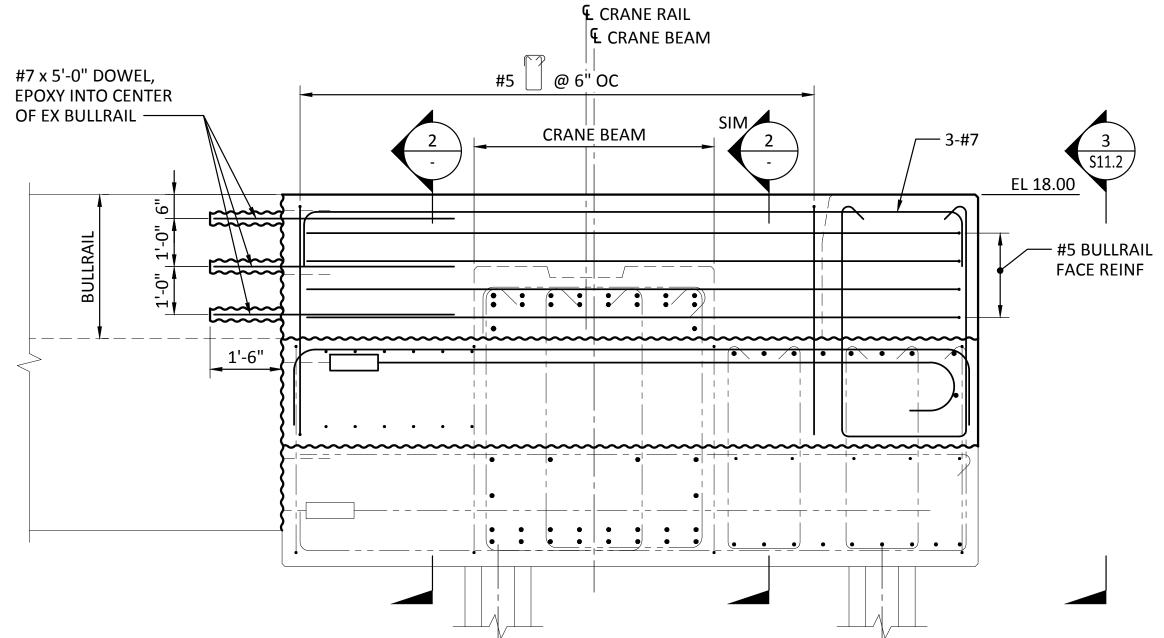












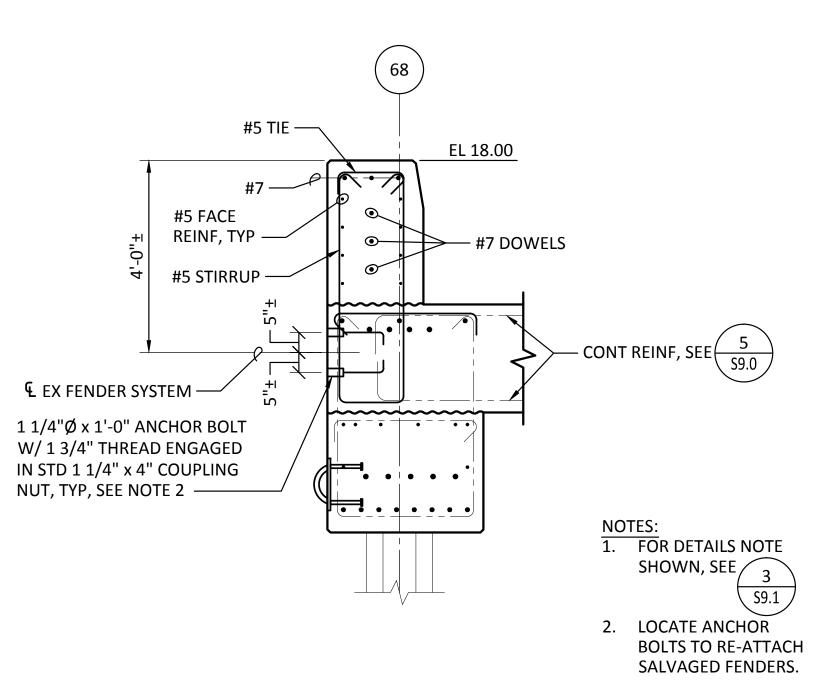
ELEVATION - BENT 68
S3.7 SCALE: 1/2"=1'-0"

NOTES: 1. FOR STAGE I REINF, SEE $\begin{pmatrix} 1 \\ 58.1 \end{pmatrix}$

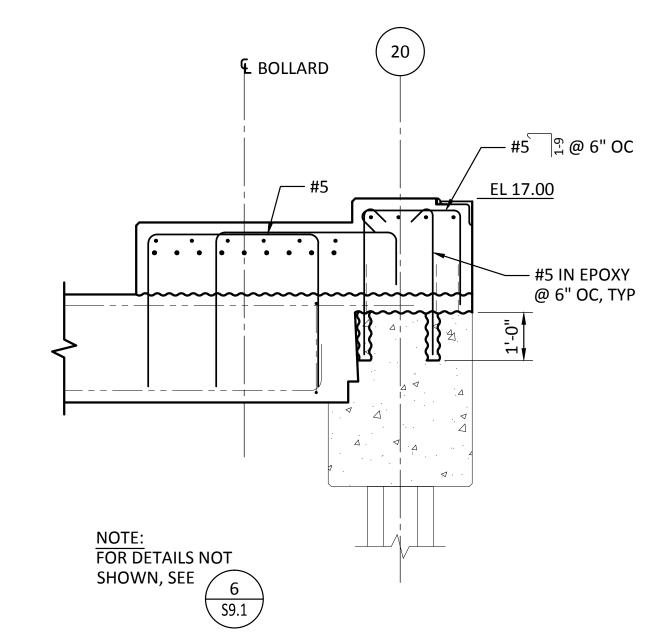
2. FOR CRANE BEAM REINF, SEE

3. FOR DETAILS NOTE SHOWN, SEE $\frac{1}{(S9.1)}$ 4. FOR TYP BULLRAIL REINF, SEE

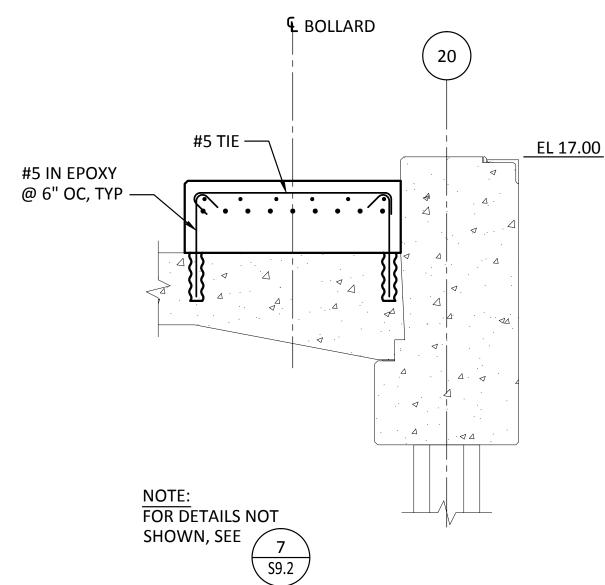
5. CRANE STOP AND LIGHTPOLE NOT SHOWN.



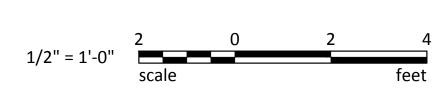
SECTION - BENT 68SCALE: 1/2" = 1'-0"



SECTION - BENT 20
SCALE: 1/2" = 1'-0"



SECTION - BENT 20



Tacoma

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33301 9th Avenue South, Suite 300
Federal Way, Washington 98003-2600
(206) 431-2300 Fax: (206) 431-2250

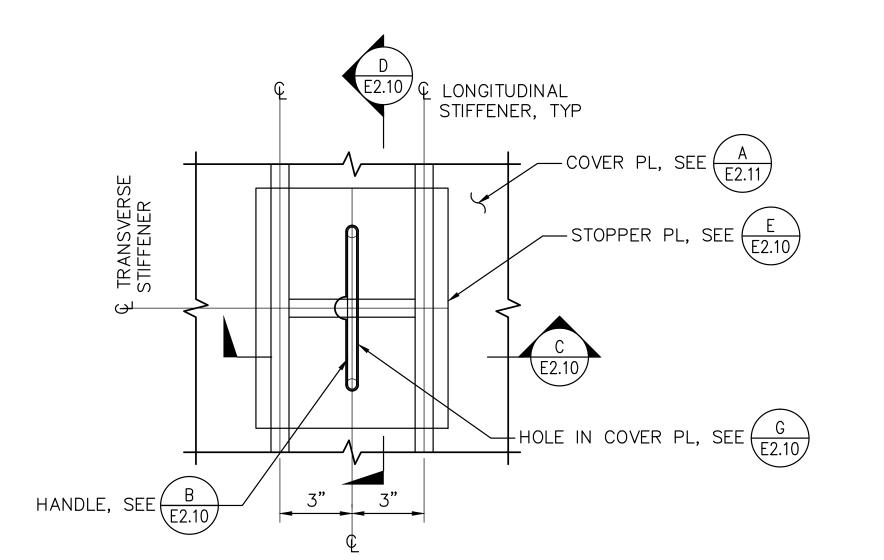
all on: Feb 25, 2015 10:05 AM File: Q:\FederalWay\2012\FAWAT-12-091\CADD\Dwgs\Electrical As-Builts\Electrical AutoCAD Rec

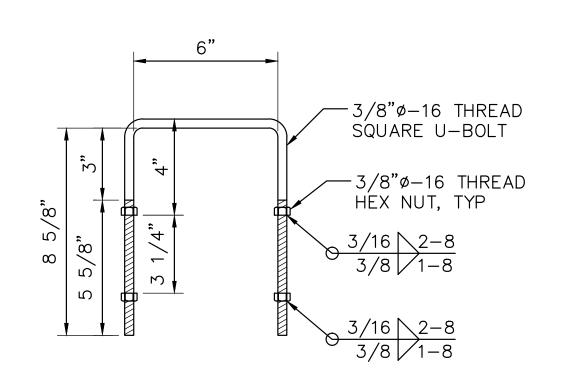
DESIGN VEHICLE LIVE LOAD:

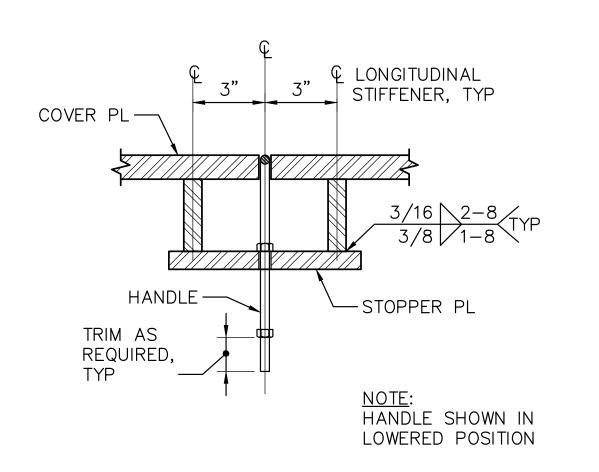
125 KIPS AT 140 PSI APPLIED

OVER A 24IN x 37.2IN AREA,

PLUS 15% IMPACT









A E2.10 DETAIL - HANDLE

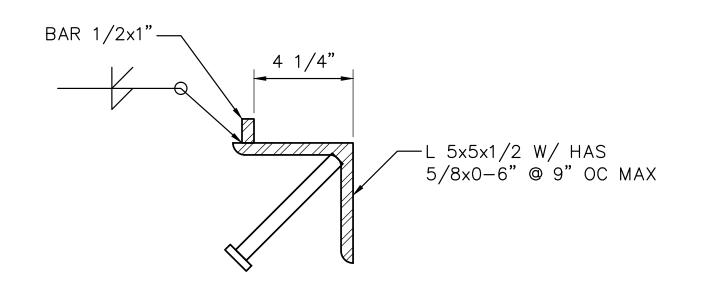
SCALE: 3"=1'-0"

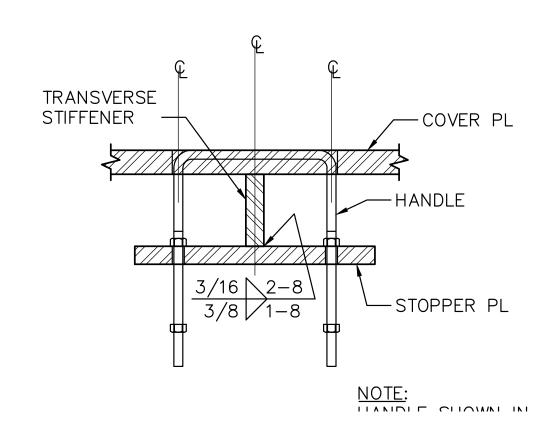
B E2.10

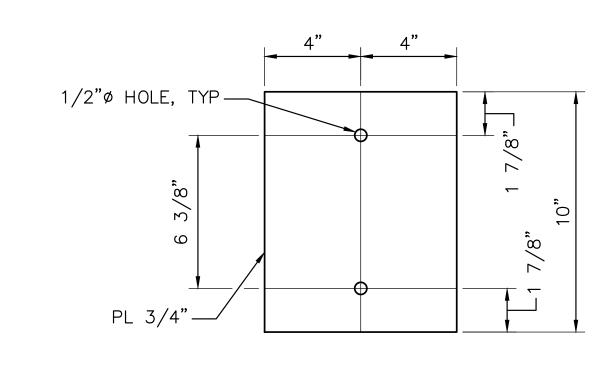
DETAIL
SCALE: 3"=1'-0"

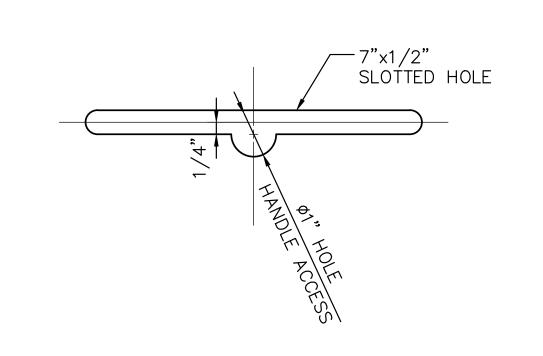
SECTION
SCALE: 3"=1'-0"







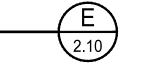




SECTION
SCALE: 3"=1'-0"
E2.10

DETAIL - STOPPER PLATE

SCALE: 3"=1'-0"



DETAIL - SLOTTED HOLE IN COVER PLATE G
SCALE: 6"=1'-0"

E2.10

6502PIER 3 UPGRADEE2 10Temporary crane connectSHEET 110 OF 129Temporary crane connectNT/CONS:069458TOWNSHIP: 21 NRANGE: 3 ESECORD DRAWINGSASE:RECORD DRAWINGSPARCEL:27DRAWING SC

1'-0"

DESIGN VEHICLE LIVE LOAD:

125 KIPS AT 140 PSI APPLIED

OVER A 24IN x 37.2IN AREA,

PLUS 15% IMPACT

1'-0"
1'-4"
1'-0"
1'-0"

COVER PL, TYP

STIFFENER, TYP

STIFFENER, TYP

6 3/8", TYP

PLAN - UNDERSIDE OF STIFFENED COVER PLATE A
SCALE: 2"=1'-0"

4'-8"

1'-4"

- TRANSVERSE STIFFENER PL

0

___COVER_PL ___1"x2'-11_7/8"x4'-8"

— STOPPER PL,TYP

<u>NOTE:</u> HANDLE AND SLOTTED HOLE NOT SHOWN

3/4"x3", TYP

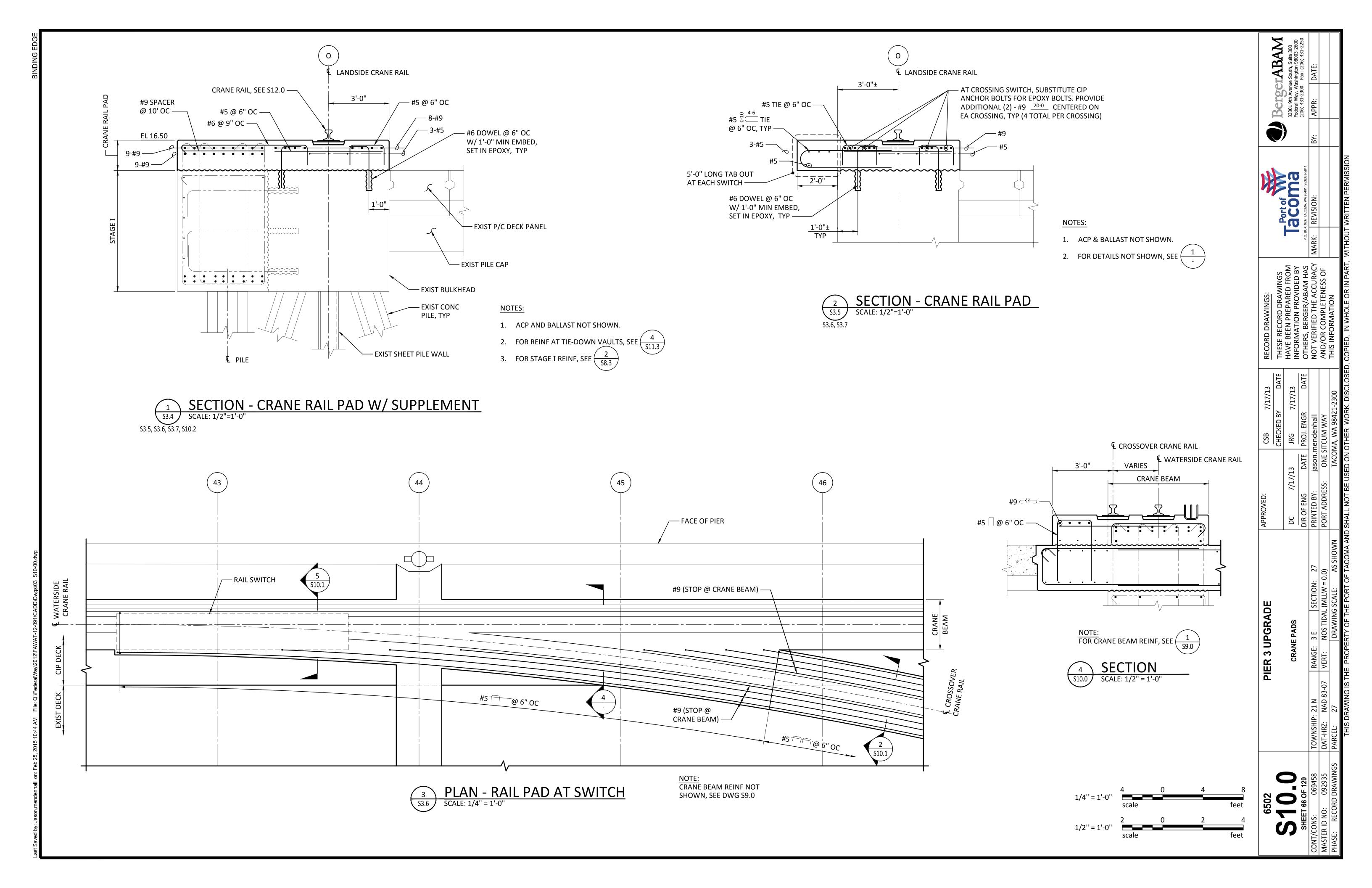
TYP 3/16 2-8

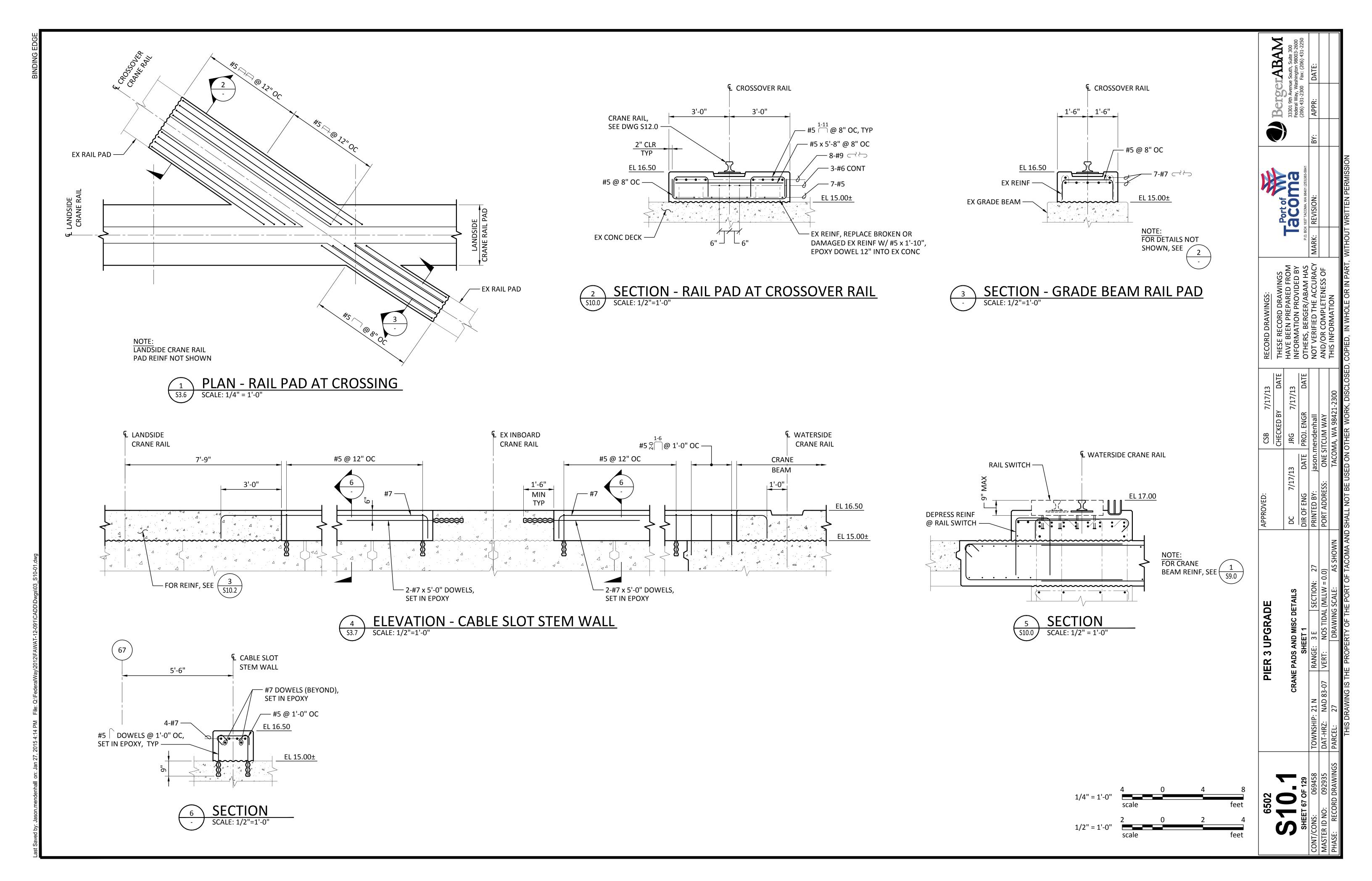
1'-4"

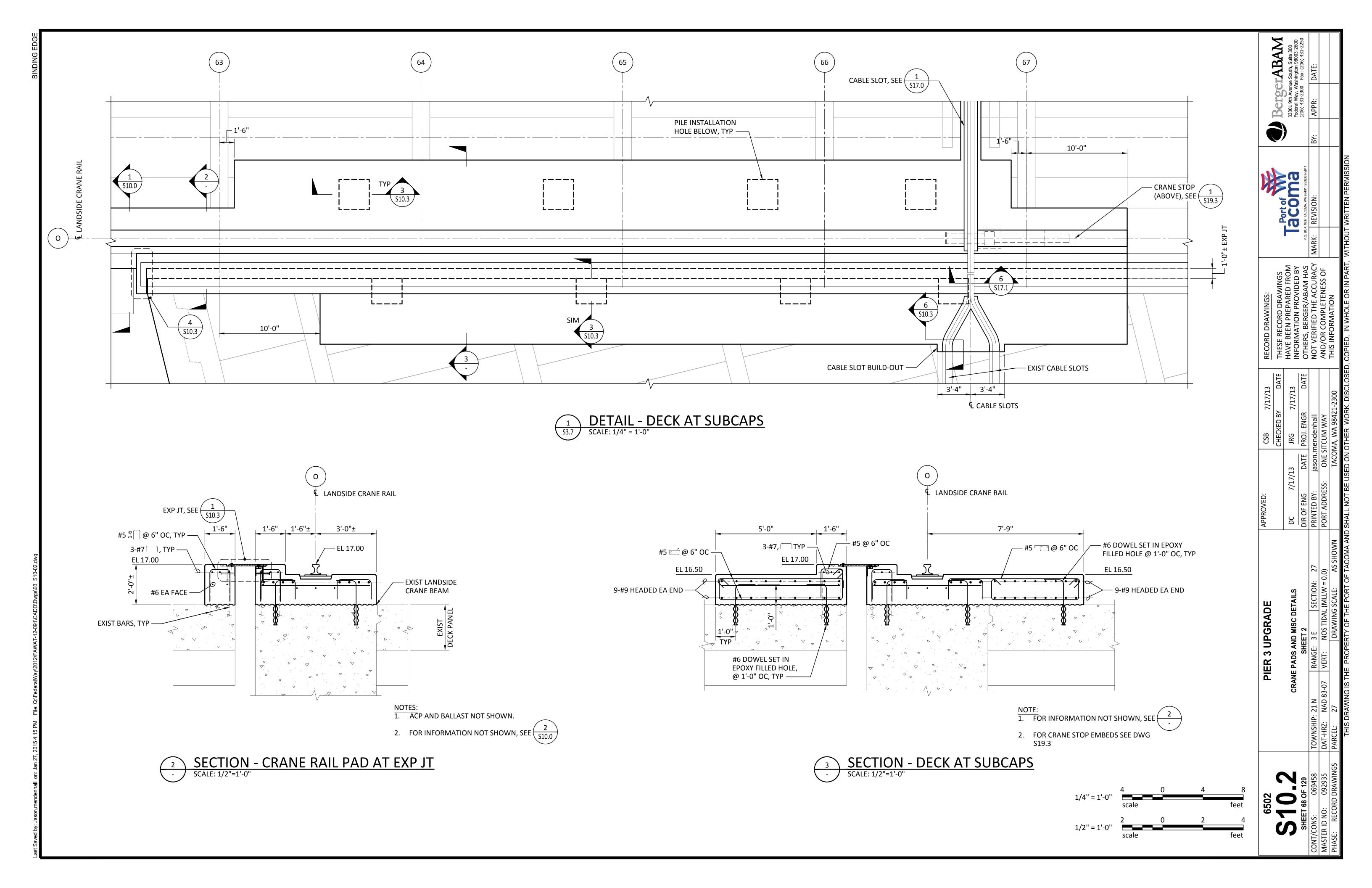
1'-0"

SECTION - STIFFENED COVER PLATE B
SCALE: 2"=1'-0"

PIER 3 UPGRADE





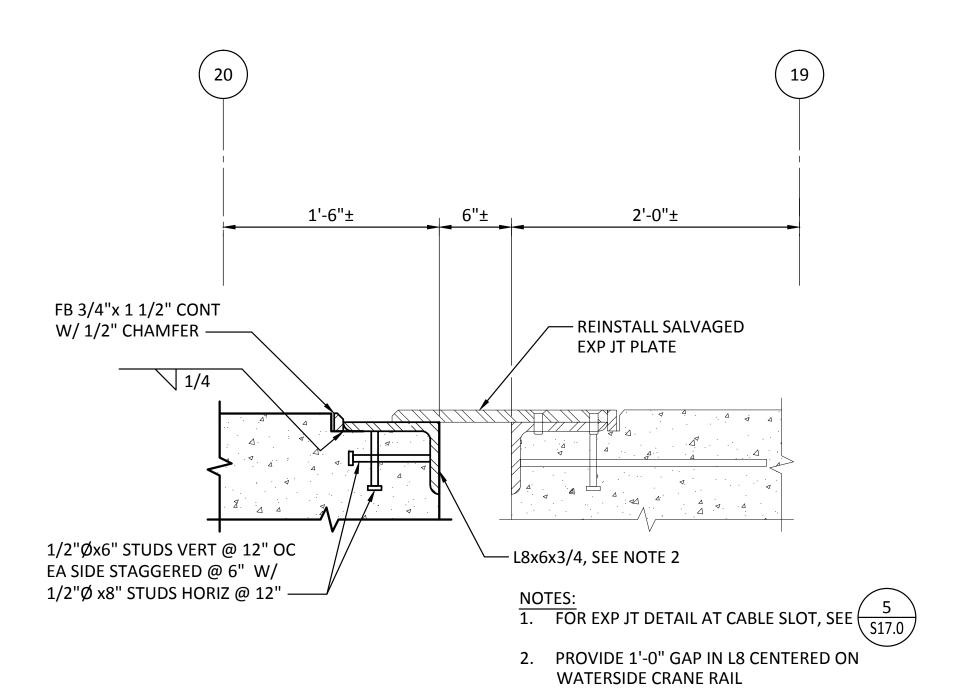




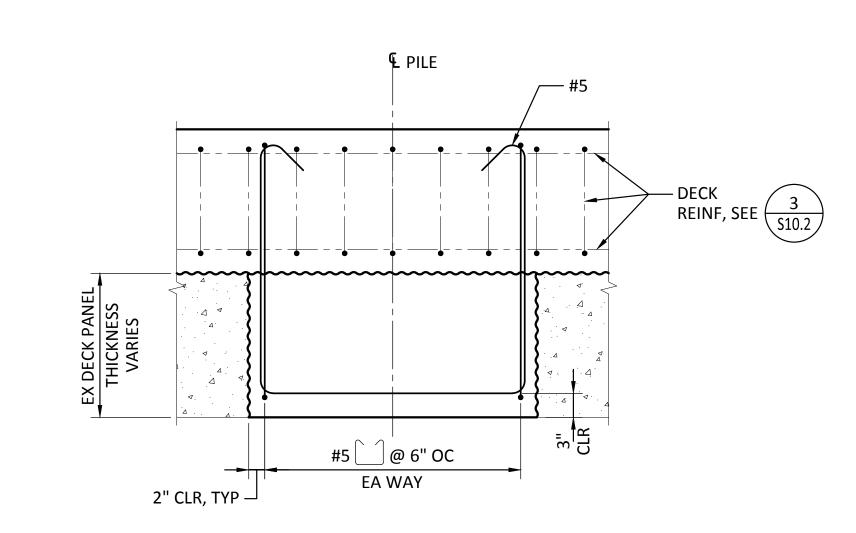
SHALL BE BOLTED TOGETHER AND CAST

INTO CONCRETE. REMOVE TEMP BOLTS

72 HRS AFTER CASTING.

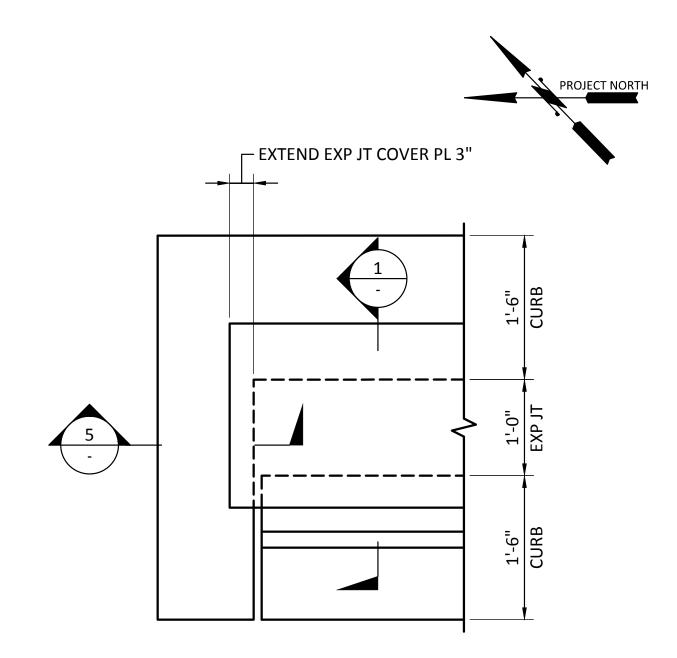


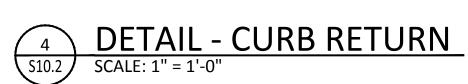


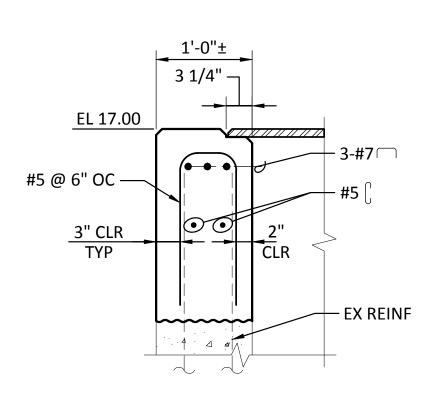


SECTION - PILE HOLE REPAIR

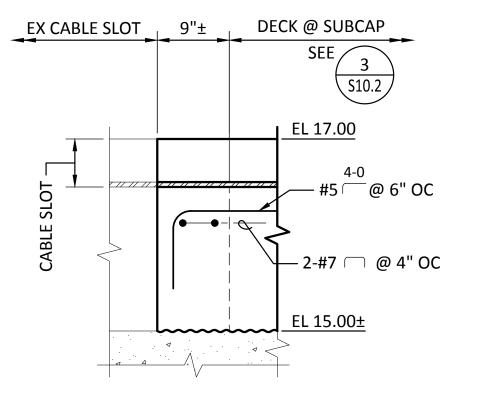
S10.2 SCALE: 1" = 1'-0"











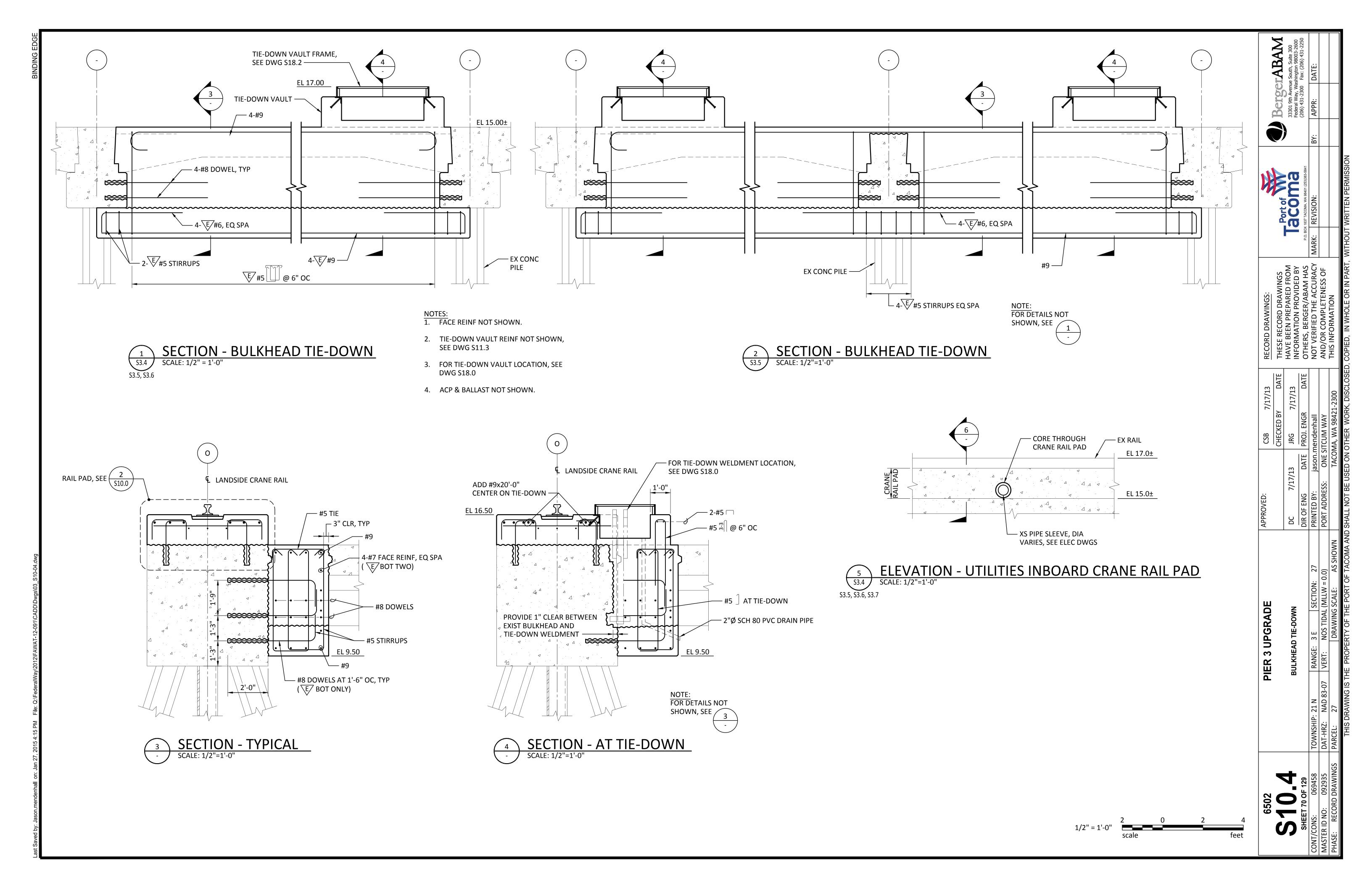
6	SECTION - CABLE SLOT BUILD-OUT
S10.2 /	SCALF: 1" = 1'-0"

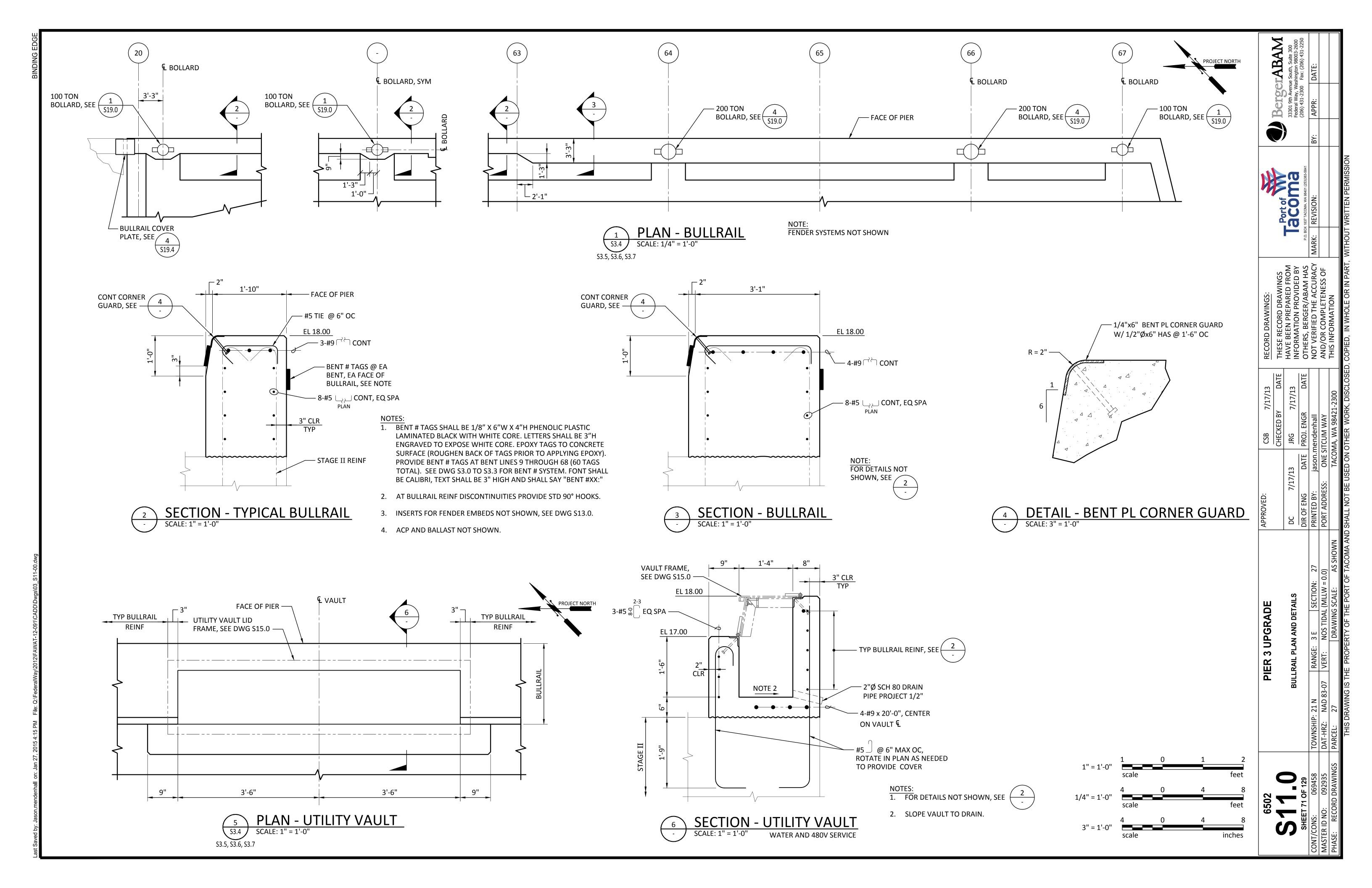
	2" = 1'-0"	4 scale	0 4	4 8 inches
1" = 1'-0"	scale	0	1	feet
1-1/2" = 1'-0"	8 scale	0	8	16 inches

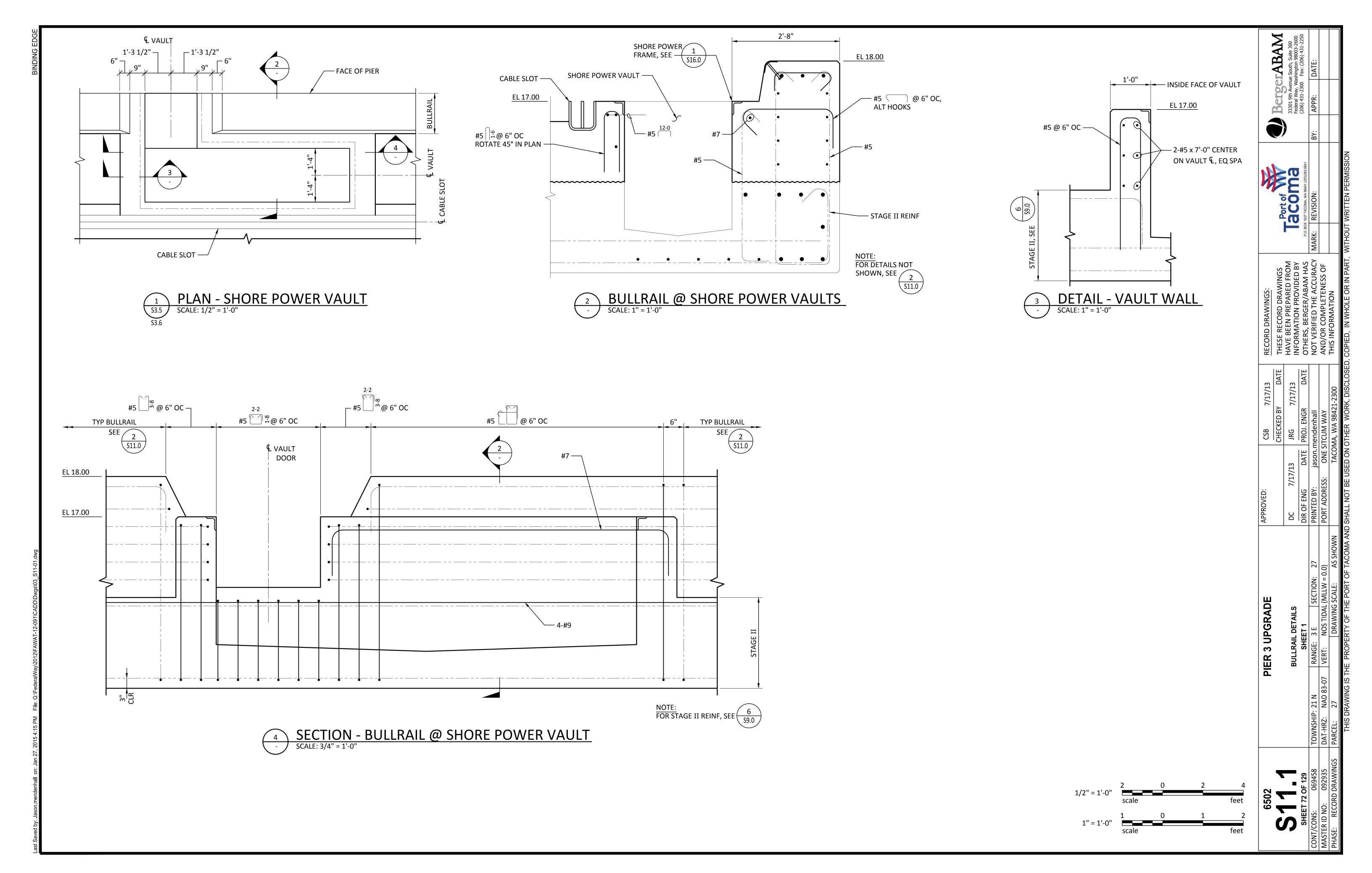
6502		₫.	IER 3 U	PIER 3 UPGRADE) E		APPROVED:		CSB 7	7/17/13
									CHECKED BY	Ď
つ.つ.つ			MISC DEC	MISC DECK DETAILS	40		DC 7/17/13	/13	JRG 7	7/17/13
SHEET 69 OF 129							DIR OF ENG	DATE	DATE PROJ. ENGR	۵
/CONS: 069458	TOWNSHIP: 21 N	: 21 N	RANGE: 3	ш	SECTION: 27	27	PRINTED BY:	jason.n	ason.mendenhall	
ER ID NO: 092935	DAT-HRZ:	DAT-HRZ: NAD 83-07	VERT:	NOS TIDAL	IOS TIDAL (MLLW = 0.0)	(0:	PORT ADDRESS:	ONE S	ONE SITCUM WAY	
E: RECORD DRAWINGS	PARCEL:	27		DRAWING	DRAWING SCALE:	AS SHOWN		TACON	TACOMA, WA 98421-2300	2300
	J SIHL	RAWING IS TE	IF PROPE	RTY OF TH	IF PORT OF	TACOMA AND	THIS DRAWING IS THE PROPERTY OF THE PORT OF TACOMA AND SHALL NOT BE LISED ON OTHER WORK DISCI	SED ON (THFR WORK	DISCI

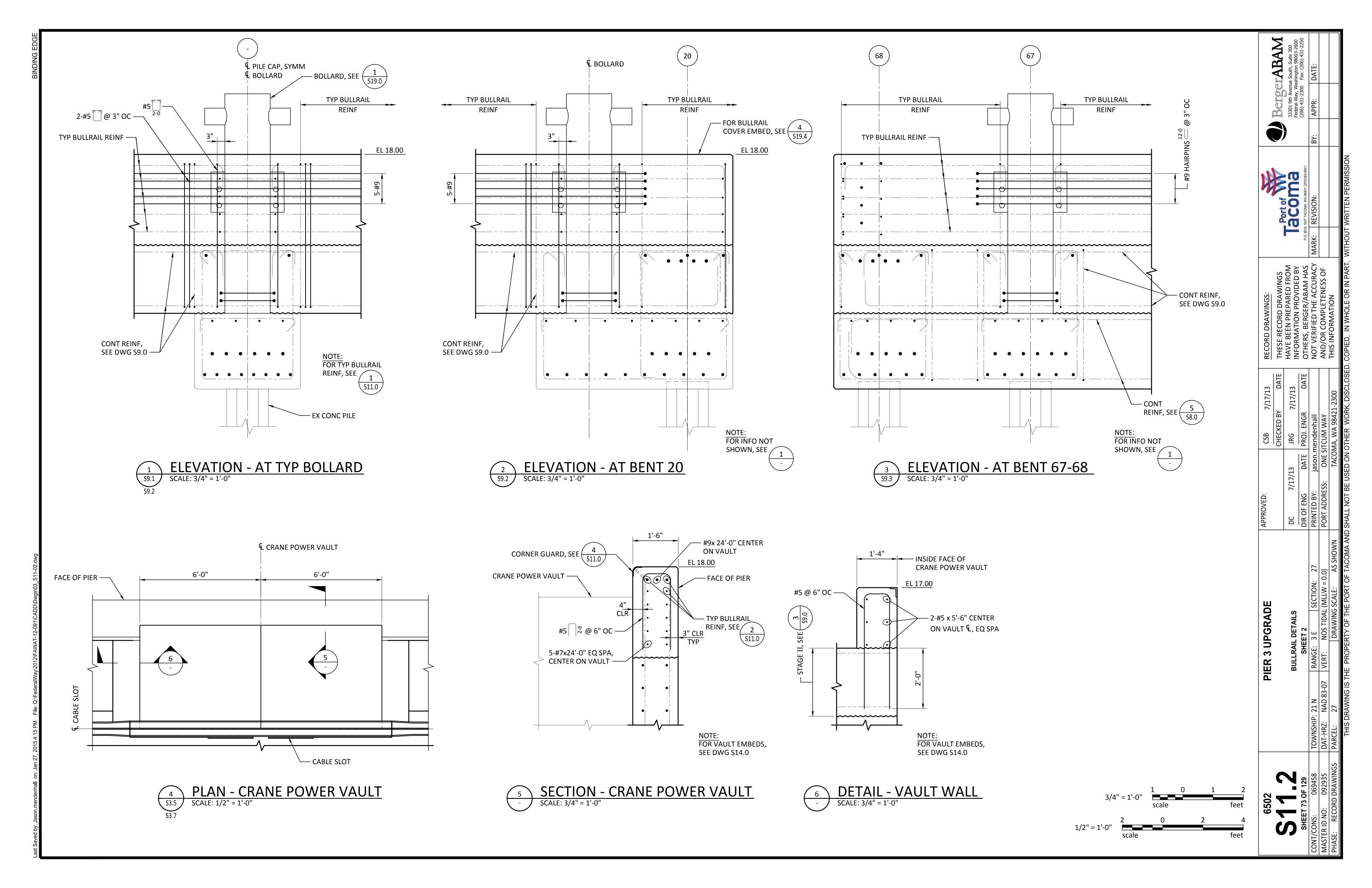
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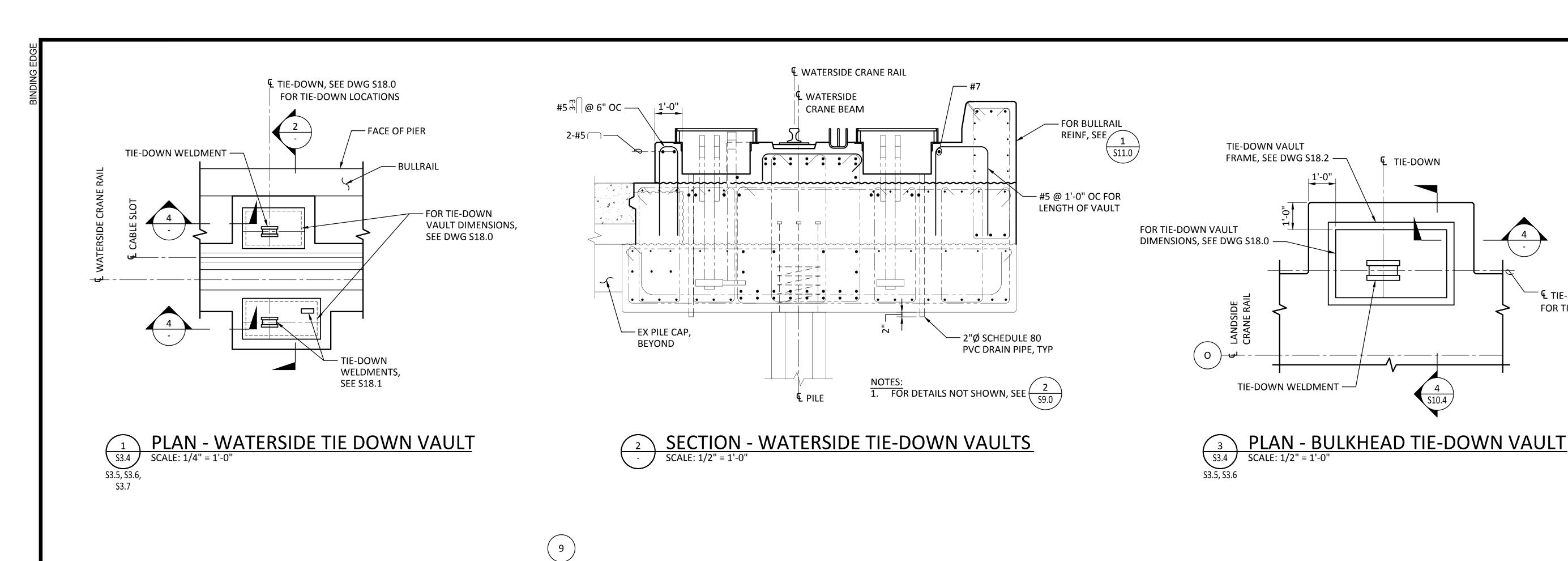
Tacon Tacon









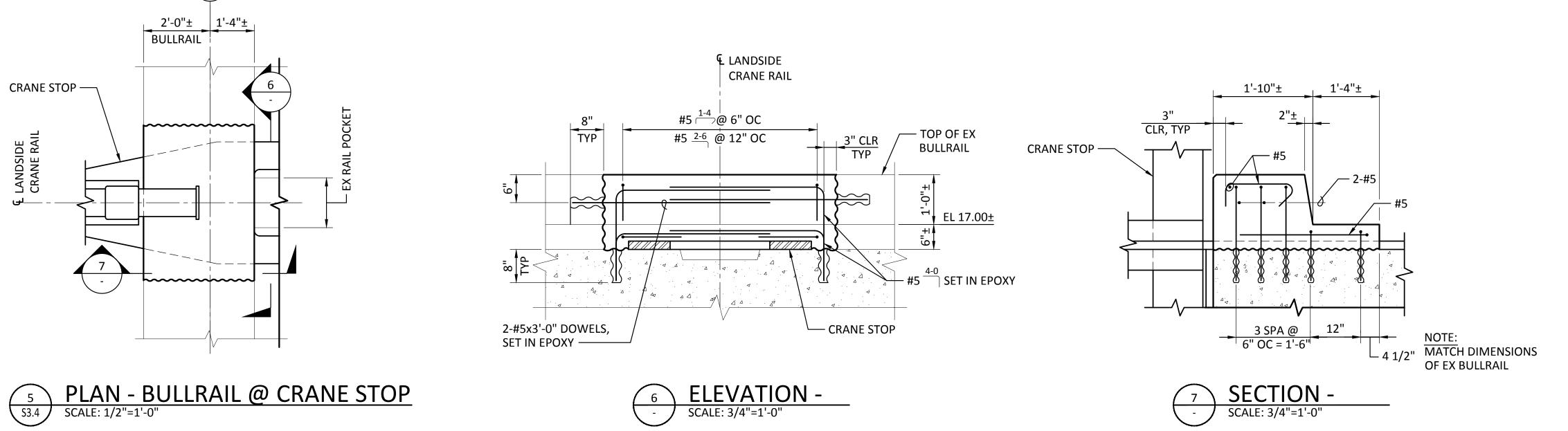


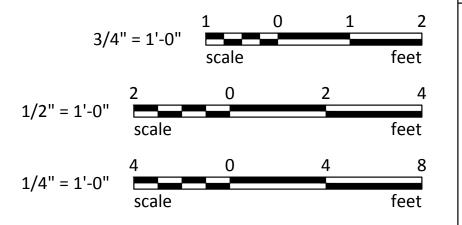
1'-0"

— #5 ເຕັ∏ @ 6" OC

SECTION - VAULT WALL
SCALE: 1/2"=1'-0"

EL 17.00



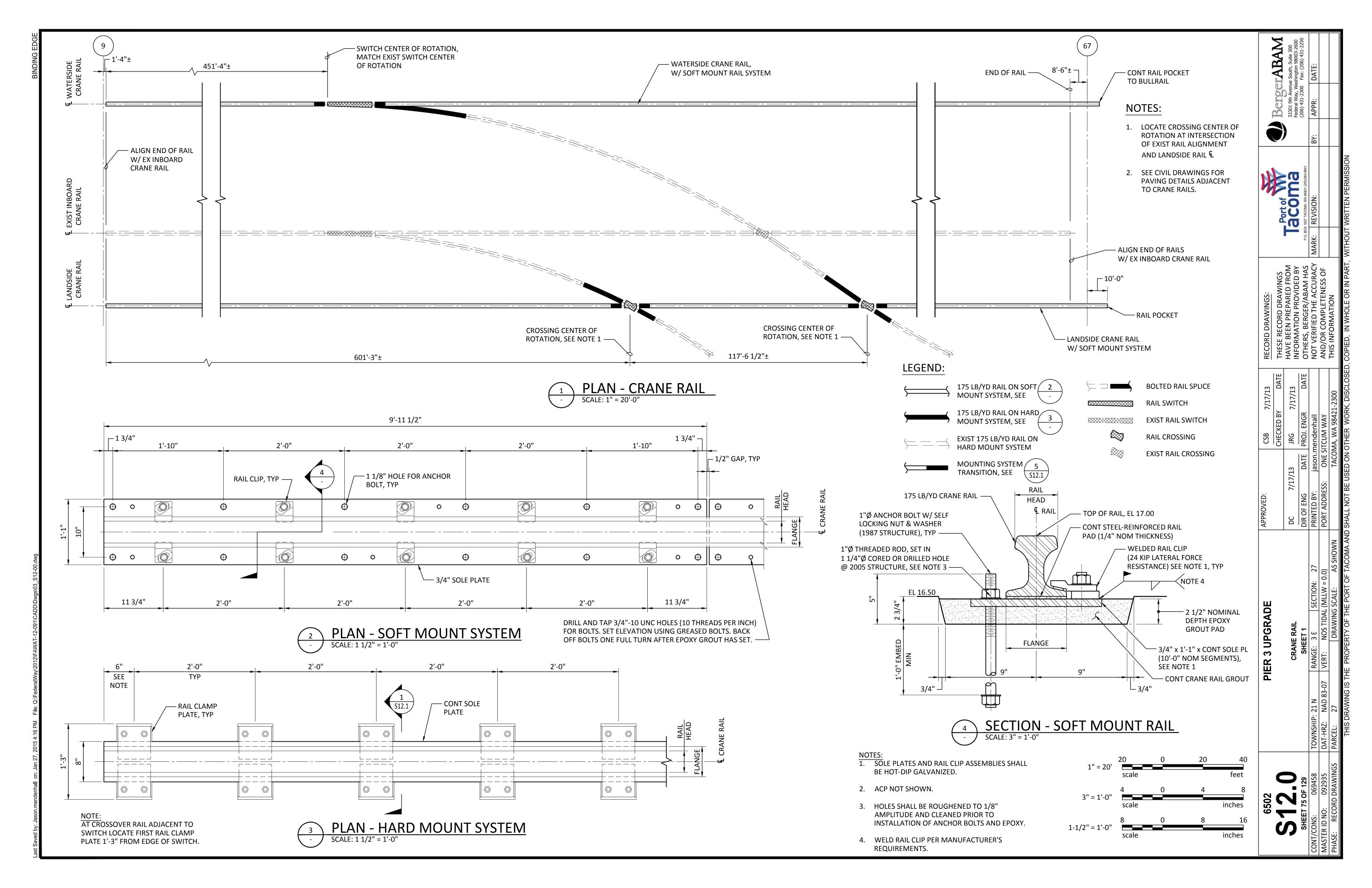


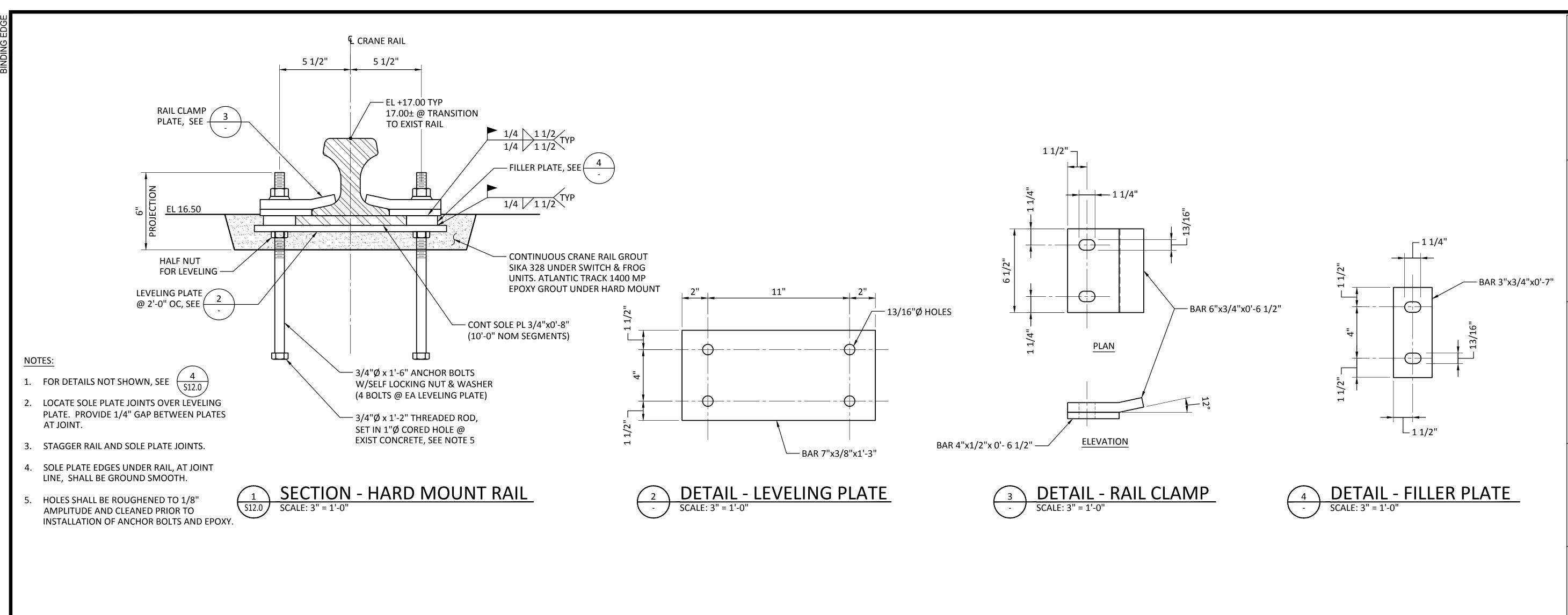
SHEET 74 OF 129 WATERSIDE CRANE TIE-DOWN VAULTS DC 7/13 SHEET 74 OF 129 TOWNSHIP: 21 N RANGE: 3 E SECTION: 27 PRINTED BY: PRINTED BY: PRINTED BY: PHASE: RECORD DRAWINGS PHASE: RECORD DRAWINGS PARCEL: 27 DRAWING SCALE: AS SHOWN AS SHOWN	6502		a	PIER 3 UPGRADE	PGRA	DE		APPROVED:
SHEET 74 OF 129 TOWNSHIP: 21 N RANGE: 3 E SECTION: 27 CONS: 069458 DAT-HRZ: NAD 83-07 VERT: NOS TIDAL (MLLW = 0.0) RECORD DRAWINGS PARCEL: 27 DRAWING SCALE: AS SHOWN	S11.3		WATERSI	DE CRANI	E TIE-DOV	VN VAULTS	40	DC 7/17
ONS: 069458 TOWNSHIP: 21 N RANGE: 3 E SECTION: 27 R ID NO: 092935 DAT-HRZ: NAD 83-07 VERT: NOS TIDAL (MLLW = 0.0) RECORD DRAWINGS PARCEL: 27 DRAWING SCALE: AS SHOWN	SHEET 74 OF 129							DIR OF ENG
RECORD DRAWINGS PARCEL: 27 VERT: NOS TIDAL (MLLW = 0.0) DRAWING SCALE: AS SHOWN		TOWNSHIP:	21 N	RANGE:	3 E	SECTION:	27	PRINTED BY:
RECORD DRAWINGS PARCEL: 27 DRAWING SCALE:		DAT-HRZ:	NAD 83-07		NOS TIDA	= MTIM) TY	0.0)	PORT ADDRESS:
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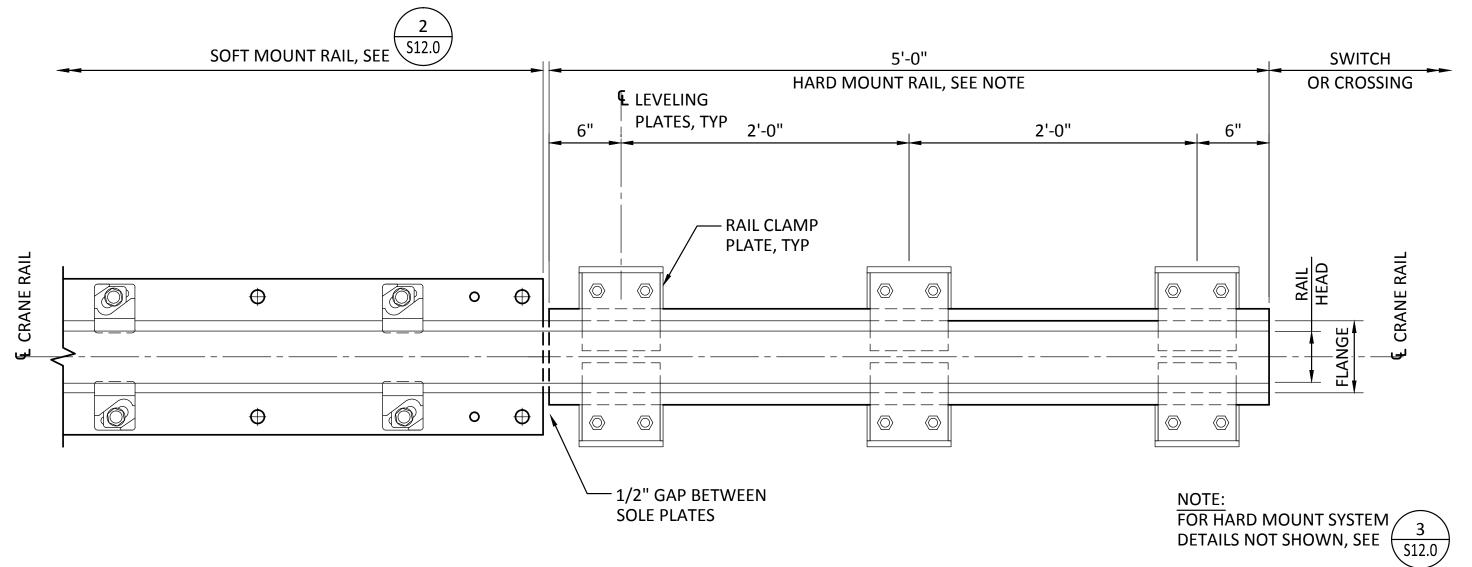
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Tacoma

FOR TIE-DOWN, SEE DWG S18.0





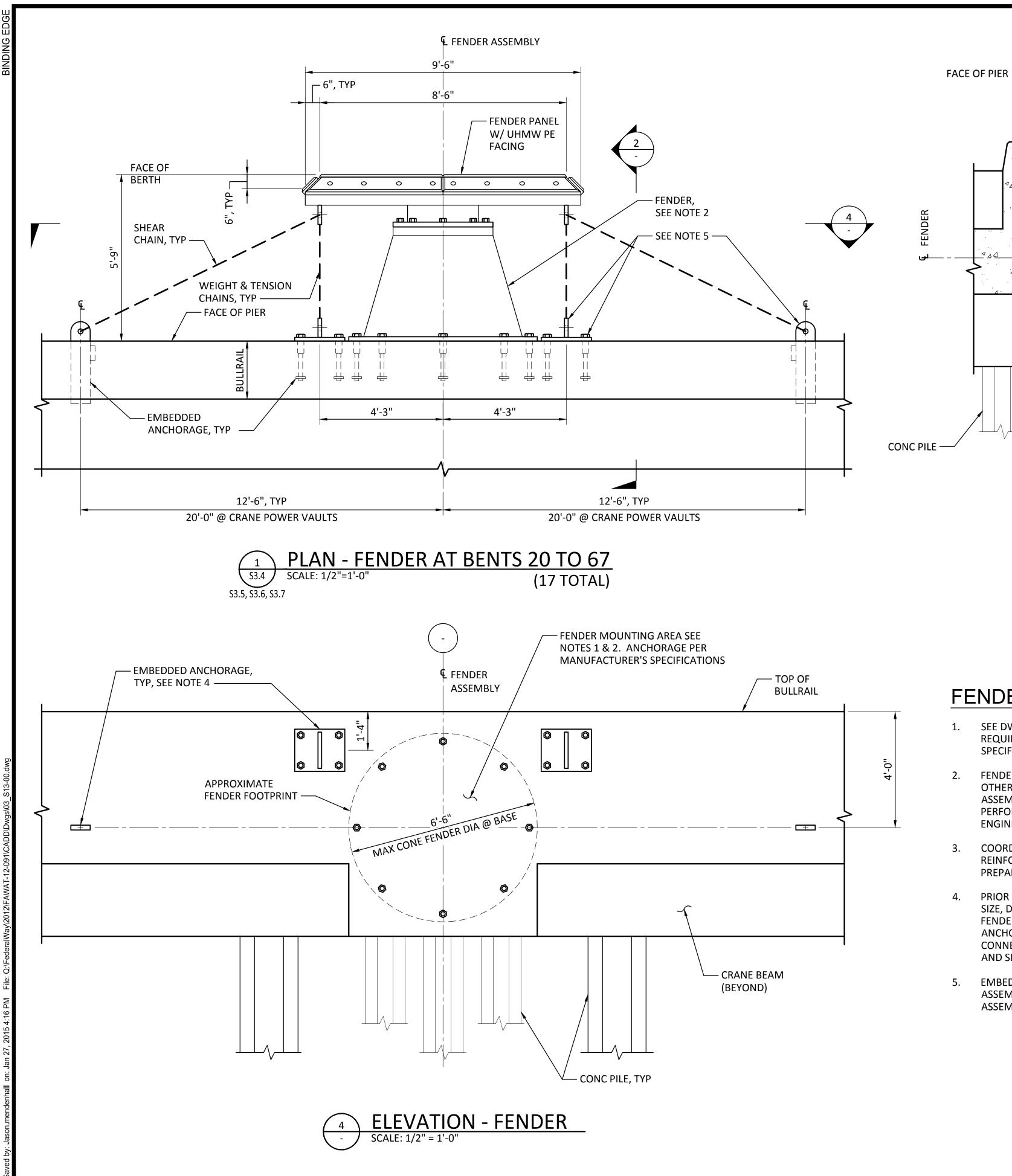


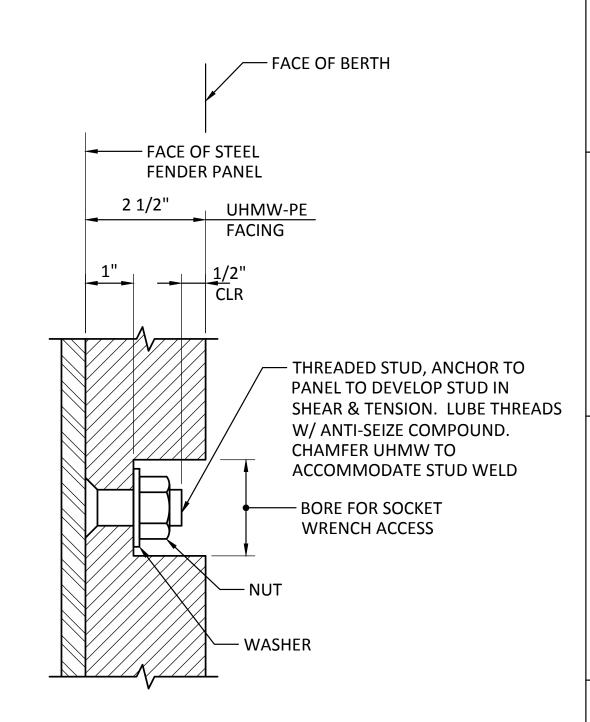


3" = 1'-0"	4 scale	0	4	8 inches
1-1/2" = 1'-0"	8 scale	0	8	16 inches

PIER 3 UPGRADE

BergerABAM
33301 9th Avenue South, Suite 300
Federal Way, Washington 98003-2600
(206) 431-2300
Fax: (206) 431-2550







FENDER SYSTEM NOTES:

CHAIN -

WEIGHT CHAIN

1. SEE DWG S1.1 FOR MINIMUM ENERGY AND MAXIMUM REACTION REQUIREMENTS. FOR FENDER LOCATIONS, SEE DWG S2.0. SEE SPECIFICATIONS FOR DEFLECTION LIMITS AND OTHER REQUIREMENTS.

SECTION

— FACE OF BERTH

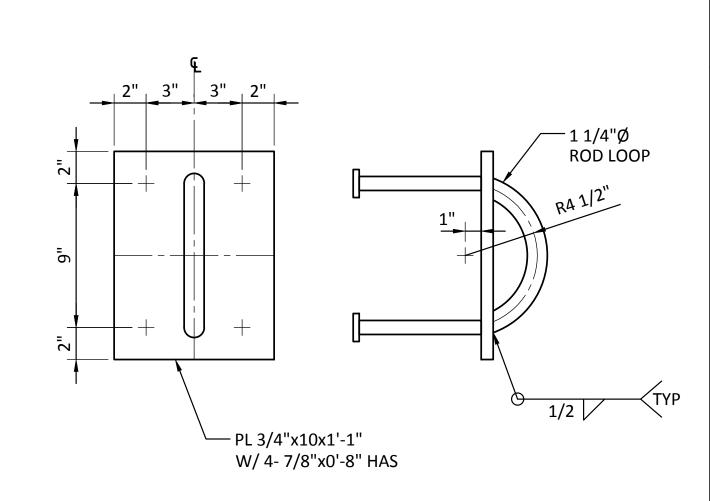
▼ EL 18.00

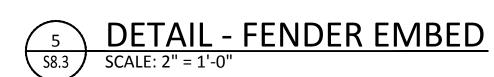
6", TYP

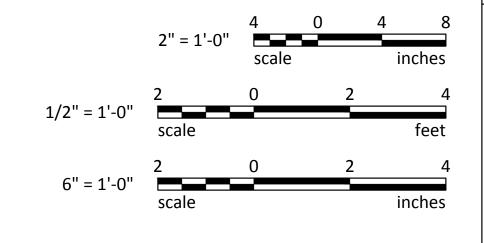
— TENSION

CHAIN

- FENDER SHOWN IS REPRESENTATIVE OF AN ACCEPTABLE PRODUCT. OTHER PRODUCTS THAT FIT WITHIN THE AREA ALLOWED FOR FENDER ASSEMBLY AND MEET SPECIFIED ENERGY AND REACTION PERFORMANCE CHARACTERISTICS MAY BE ACCEPTED SUBJECT TO ENGINEER'S PRIOR APPROVAL.
- COORDINATE LOCATION OF EMBEDDED ITEMS WITH PLACEMENT OF REINFORCING STEEL AND OTHER APPURTENANCES DURING SUBMITTAL PREPARATION.
- PRIOR TO FABRICATING ANY MATERIALS FOR FENDER SYSTEM VERIFY SIZE, DIMENSIONS, AND LOCATIONS OF ALL ITEMS. INCLUDING RUBBER FENDER, UHMW-PE FACING, FENDER PANEL, CHAINS, FENDER BOLTS, ANCHORS RODS OR BOLTS, EMBEDDED PLATES, ETC., SO THAT TIGHT CONNECTIONS CAN BE MADE AND ALL COMPONENTS ARE COMPATIBLE AND SNUG UPON COMPLETION.
- EMBEDDED ANCHORAGE (1987 STRUCTURE) IS PART OF THE FENDER ASSEMBLY AND SHALL BE DESIGNED AND PROVIDED BY THE FENDER ASSEMBLY MANUFACTURER.

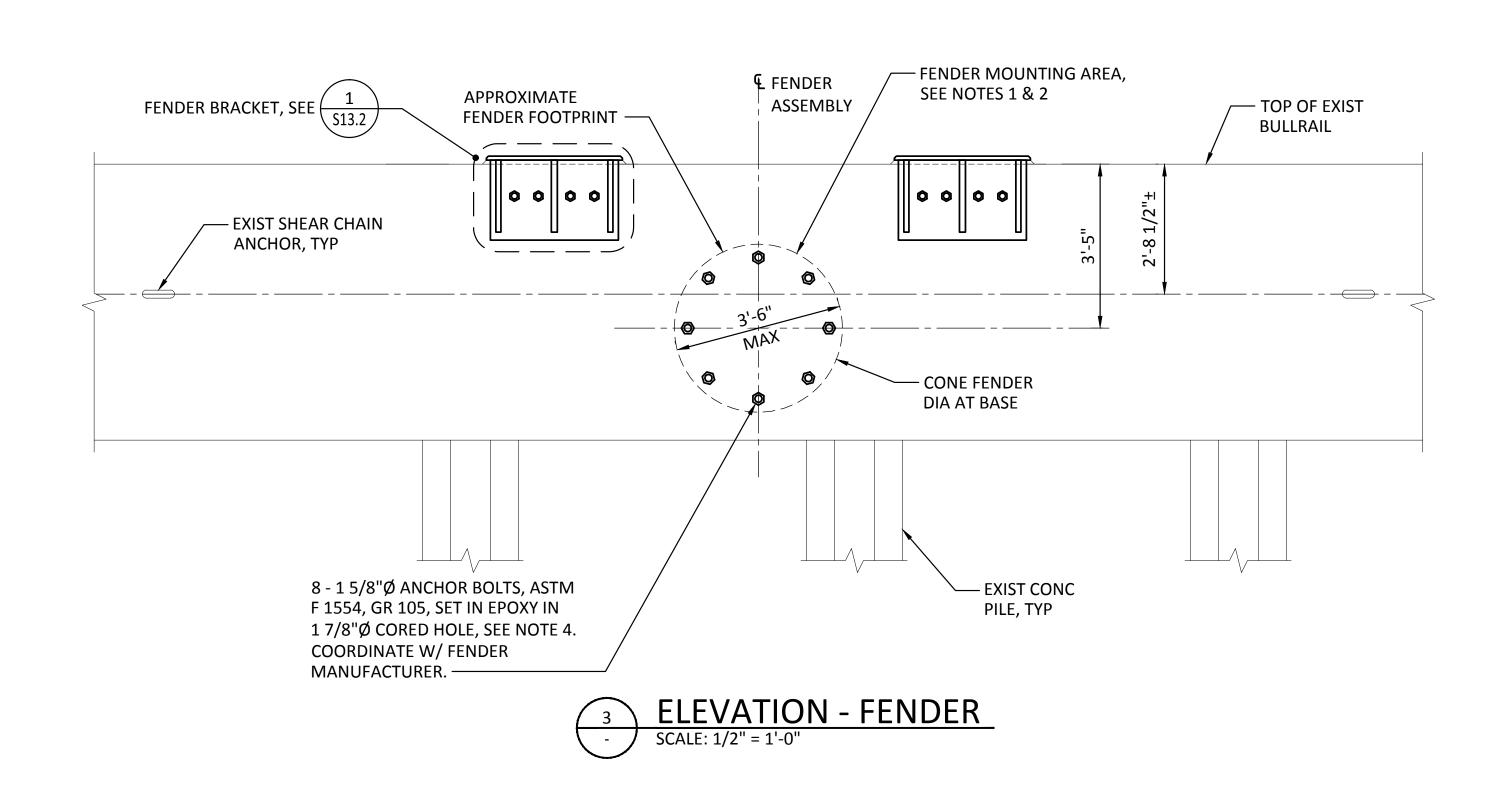


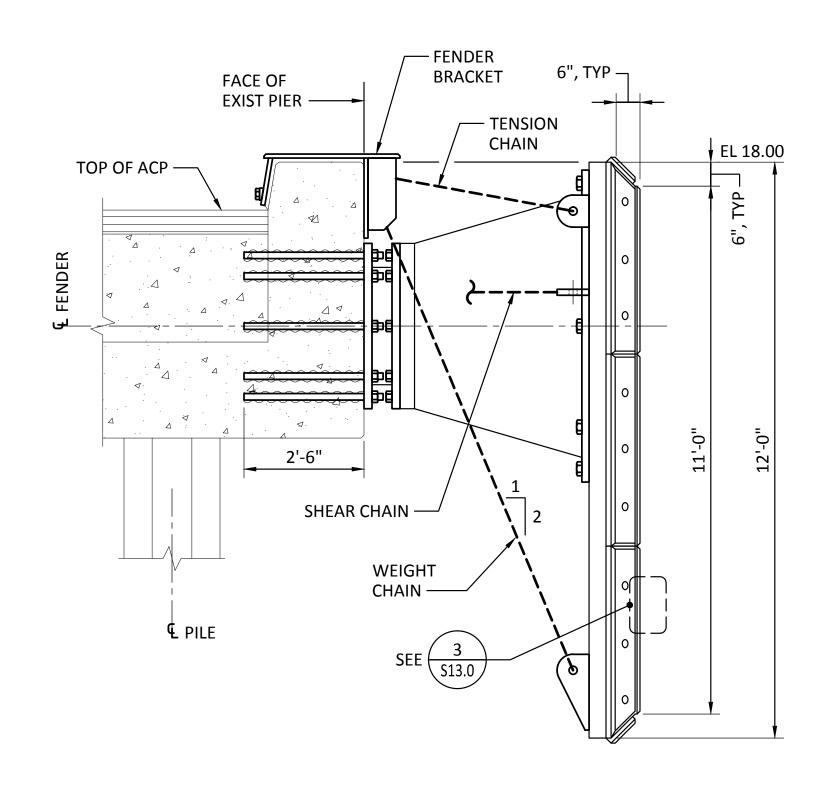


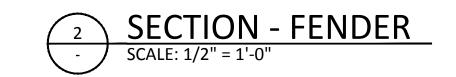


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0.5.0	ii ii	ENDER SYS	FENDER SYSTEM DETAILS	S	DC 7/17/13	<u> </u>	JRG 7/17/13	13	HAVE BEEN PREPARED FROM INFORMATION PROVIDED BY		acoma		33301 Pedera	33301 9th Avenue South, 9 Federal Way, Washington	outh, s
SHEET 77 OF 129		SHE	SHEET 1		DIR OF ENG D	ATE PF	DATE PROJ. ENGR	DATE	OTHERS, BERGER/ABAM HAS	P.O.B	P.O. BOX 1837 TACOMA, WA 98401 (253)383-5841		(206) 4	(206) 431-2300 Fax: (20	x: (20
T/CONS: 069458	TOWNSHIP: 21 N	RANGE: 3 E		SECTION: 27	PRINTED BY: jas	son.mer	jason.mendenhall		NOT VERIFIED THE ACCURACY MARK: REVISION:	MARK:	REVISION:	BY:	APPR:		DATE:
TER ID NO: 092935	DAT-HRZ: NAD 83-07 VERT:	VERT:	NOS TIDAL (MILLW = 0.0)	MLLW = 0.0)	PORT ADDRESS: C	ONE SITC	ONE SITCUM WAY		AND/OR COMPLETENESS OF						
SE: RECORD DRAWINGS	PARCEL: 27		DRAWING SCALE:	CALE: AS SHOWN	_	ACOMA	TACOMA, WA 98421-2300	0	THIS INFORMATION						
	THIS DRAWING IS T	THE PROPE	ERTY OF THE	PORT OF TACOMA AND	SHALL NOT BE USED	ON OT	HER WORK, DIS	SCLOSED,	THIS DRAWING IS THE PROPERTY OF THE PORT OF TACOMA AND SHALL NOT BE USED ON OTHER WORK, DISCLOSED, COPIED, IN WHOLE OR IN PART, WITHOUT WRITTEN PERMISSION	WITHOU	T WRITTEN PERMISS	NOI			

1 PLAN - FENDER AT BENTS 9 TO 19 (5 TOTAL) S3.5 SCALE: 1/2" = 1'-0"

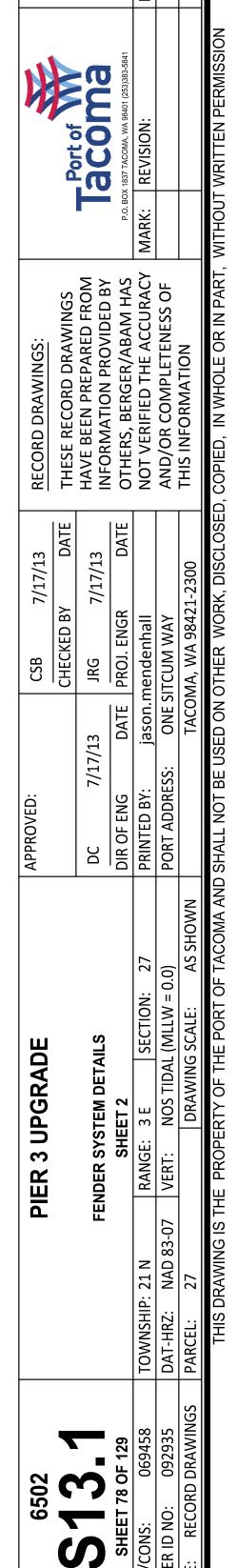




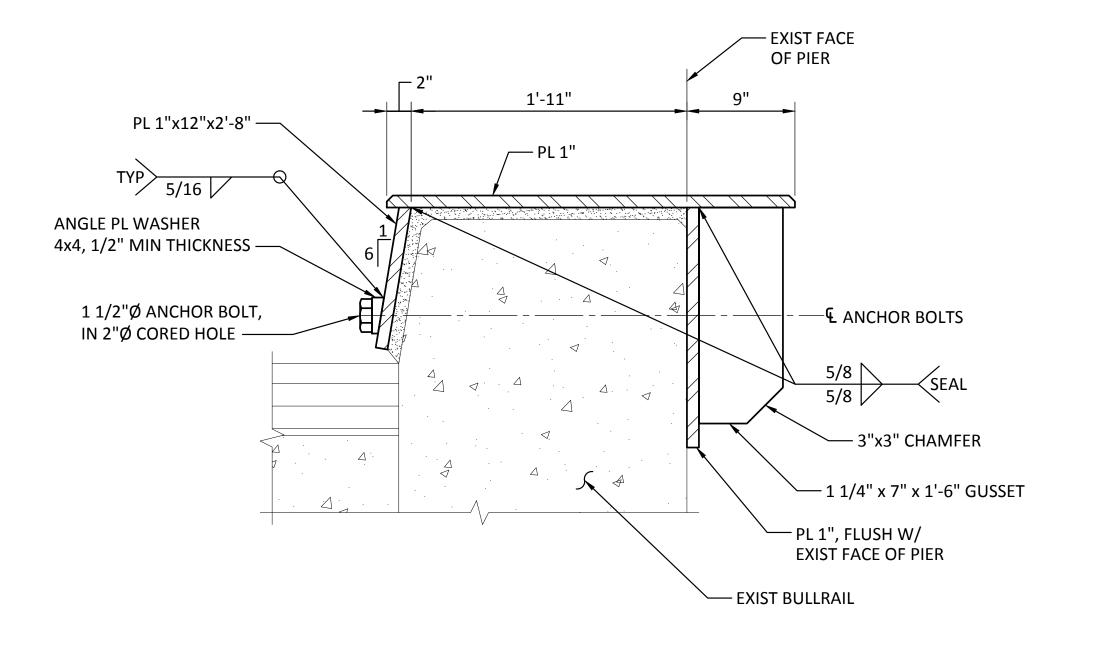


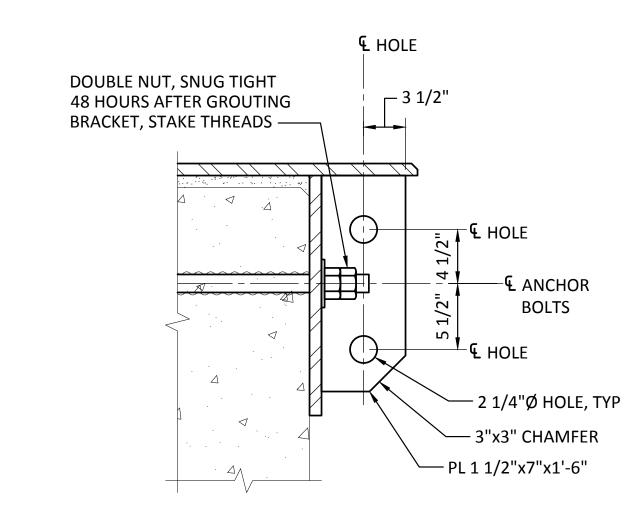
FENDER SYSTEM NOTES:

- 1. SEE DWG S1.1 FOR MINIMUM ENERGY AND MAXIMUM REACTION REQUIREMENTS. FOR FENDER LOCATIONS, SEE DWG S2.0. SEE SPECIFICATIONS FOR DEFLECTION LIMITS AND OTHER REQUIREMENTS.
- 2. FENDER SHOWN IS REPRESENTATIVE OF AN ACCEPTABLE PRODUCT. OTHER PRODUCTS THAT FIT WITHIN THE AREA ALLOWED FOR FENDER ASSEMBLY SHALL MEET SPECIFIED ENERGY AND REACTION PERFORMANCE CHARACTERISTICS MAY BE ACCEPTED SUBJECT TO ENGINEER'S PRIOR APPROVAL.
- 3. PRIOR TO FABRICATING ANY MATERIALS FOR FENDER SYSTEM VERIFY SIZE, DIMENSIONS, AND LOCATIONS OF ALL ITEMS. INCLUDING RUBBER FENDER, UHMW-PE FACING, FENDER PANEL, FENDER SPACER, CHAINS, FENDER BOLTS, ANCHORS RODS OR BOLTS, EMBEDDED PLATES, ETC., SO THAT TIGHT CONNECTIONS CAN BE MADE AND ALL COMPONENTS ARE COMPATIBLE AND SNUG UPON COMPLETION.
- HOLES SHALL BE ROUGHENED TO 1/8" AMPLITUDE AND CLEANED PRIOR TO INSTALLATION OF ANCHOR BOLTS AND EPOXY.



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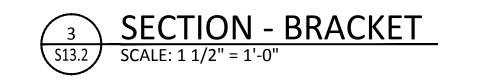




ELEVATION - FENDER BRACKET

S13.1 SCALE: 1 1/2" = 1'-0"

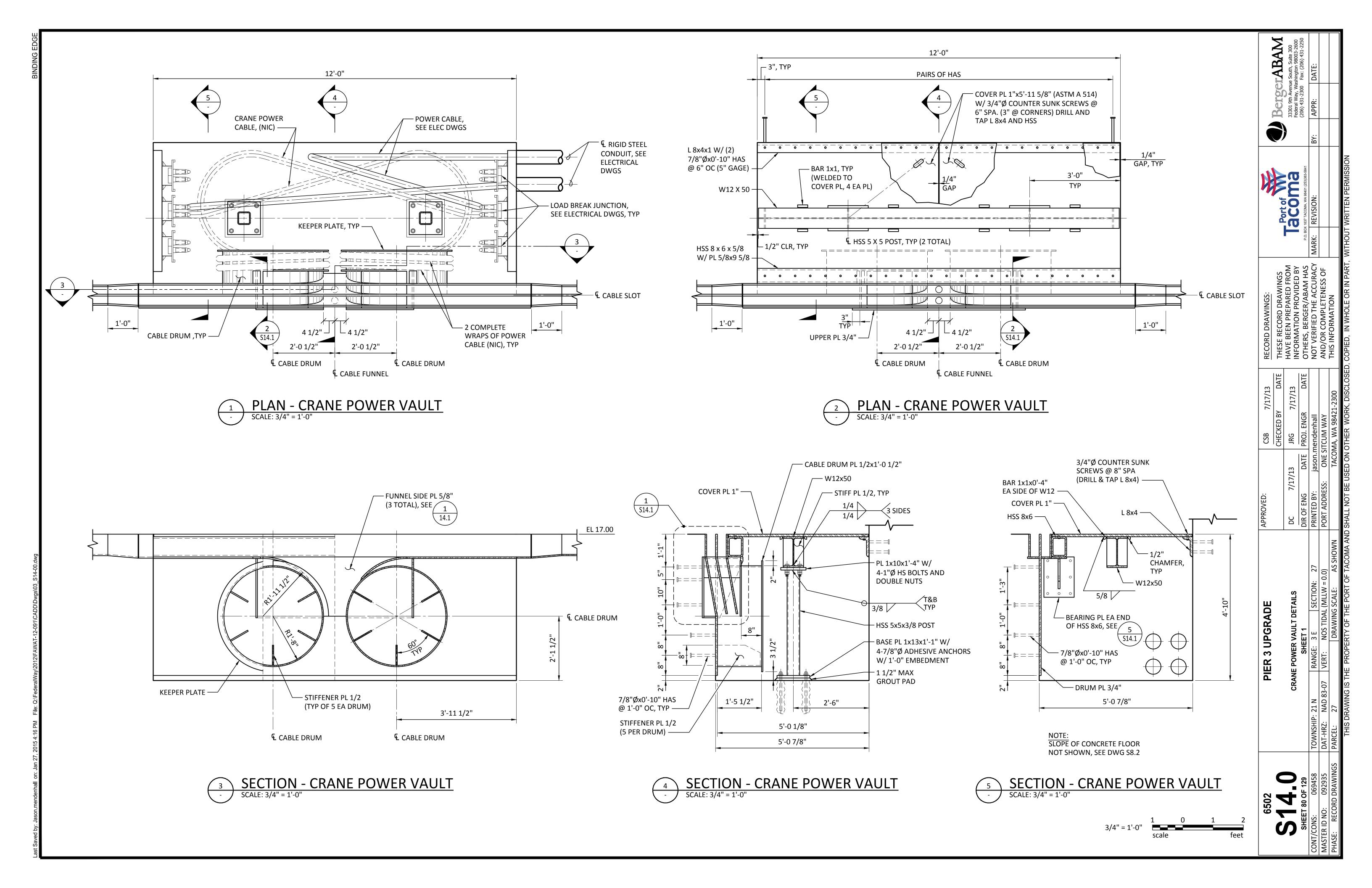
2 SECTION - BRACKET
S13.2 SCALE: 1 1/2" = 1'-0"

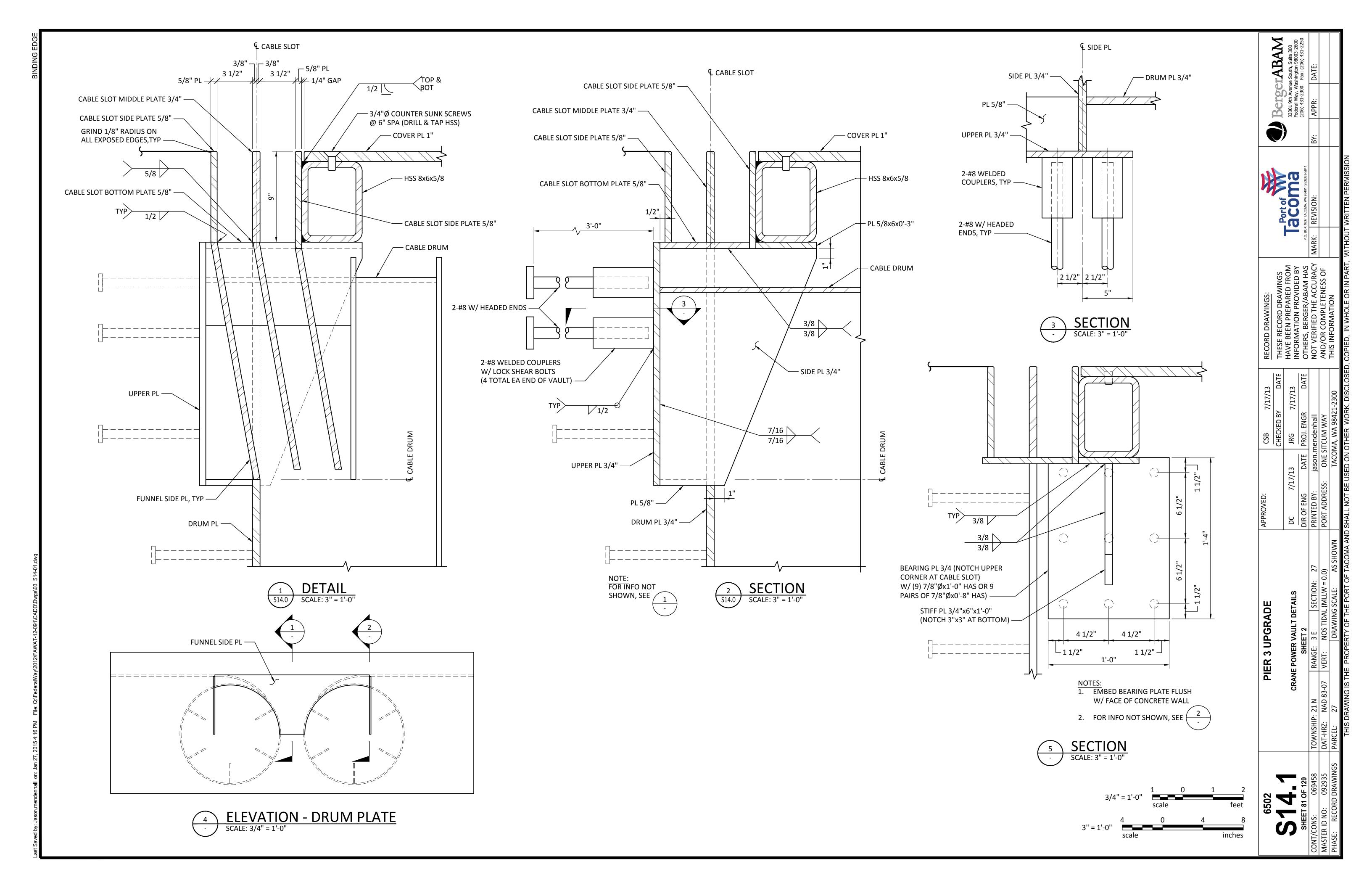


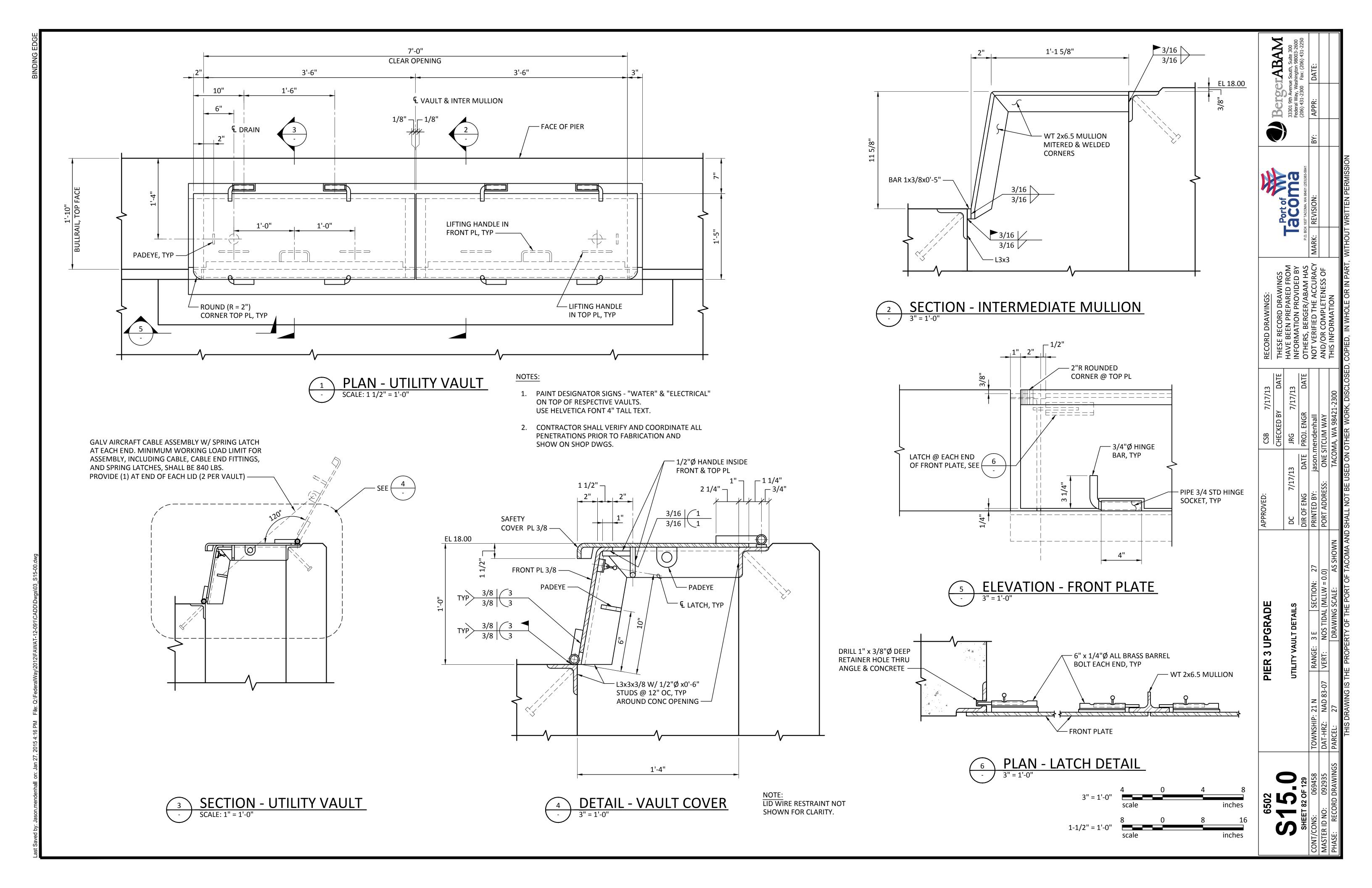
S13 2 2 SHEET 79 OF 129

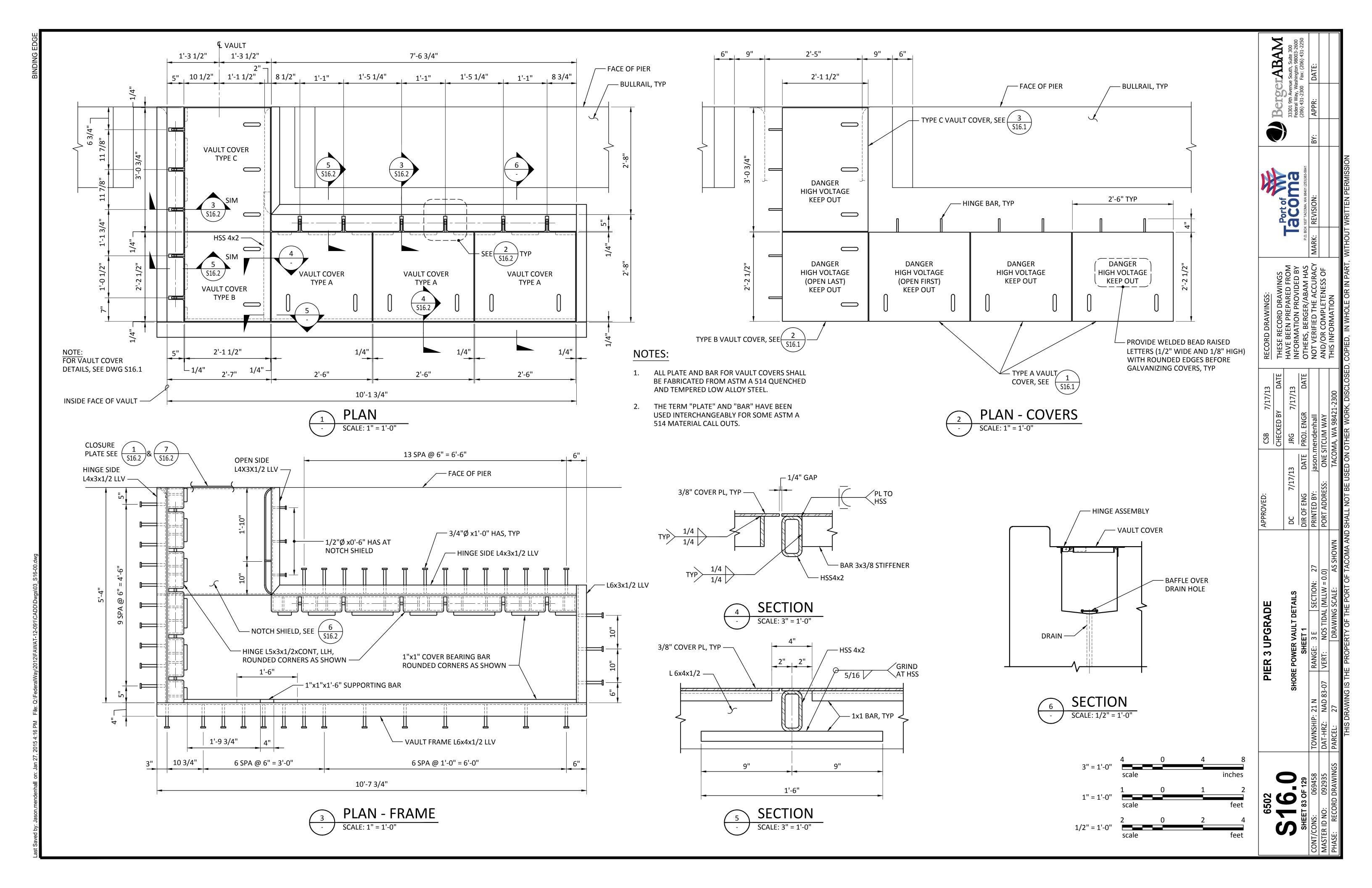
CONT/CONS: 069458 TOWNSHIP: 21 N RANGE: 3 E SECTION: DAT-HRZ: NAD 83-07 VERT: NOS TIDAL (MLLW = 0 PHASE: RECORD DRAWINGS PARCEL: 27 DRAWING IS THE PROPERTY OF THE PORT OF

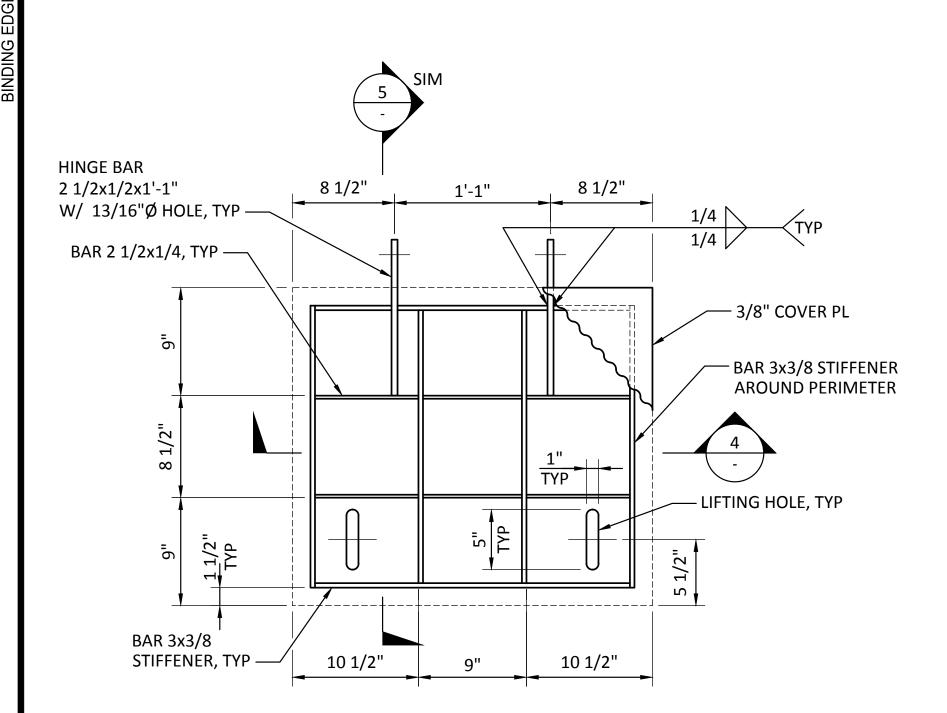
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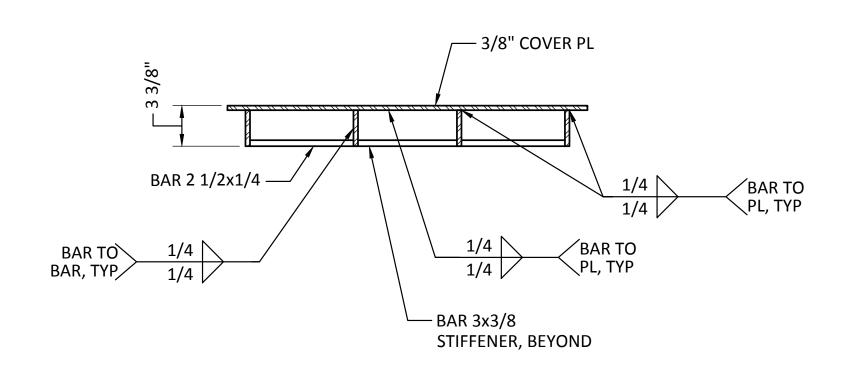




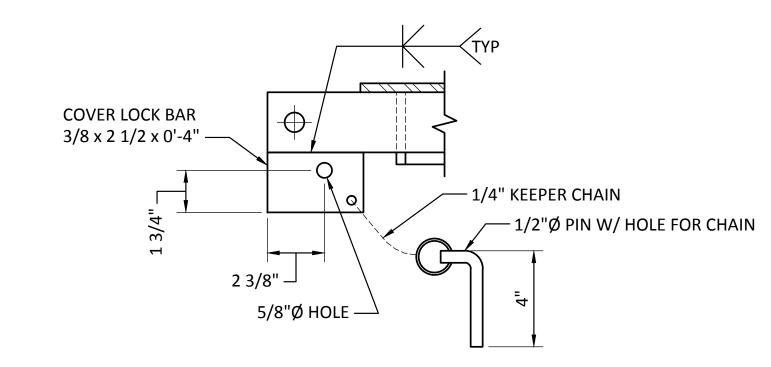




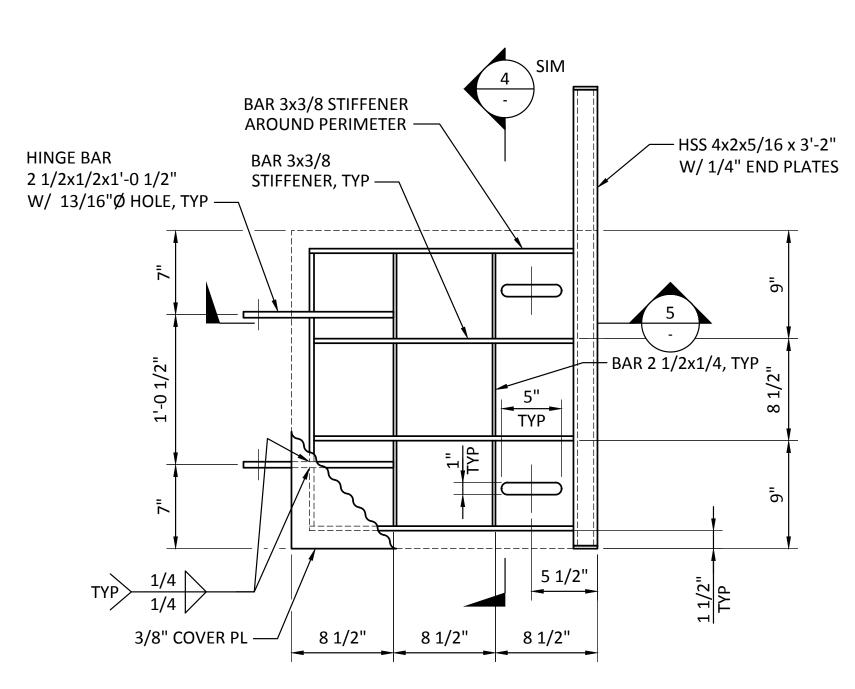
DETAIL - VAULT COVER (TYPE A) SCALE: 1 1/2" = 1'-0"



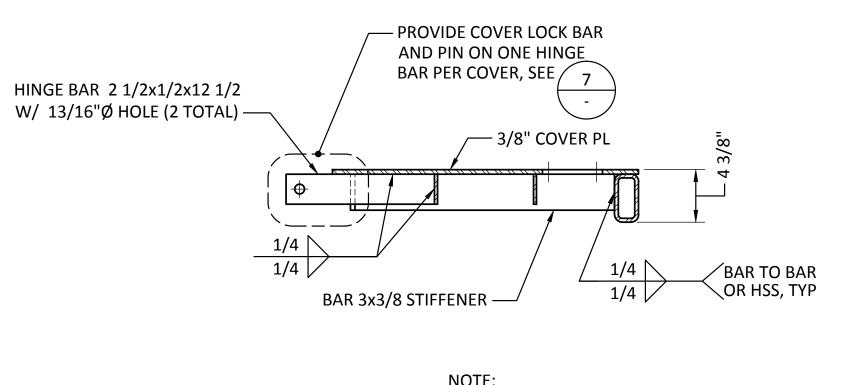
4 SECTION - SCALE: 1 1/2" = 1'-0"

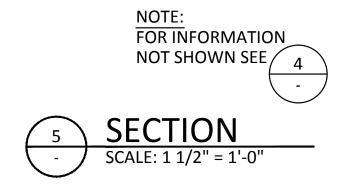


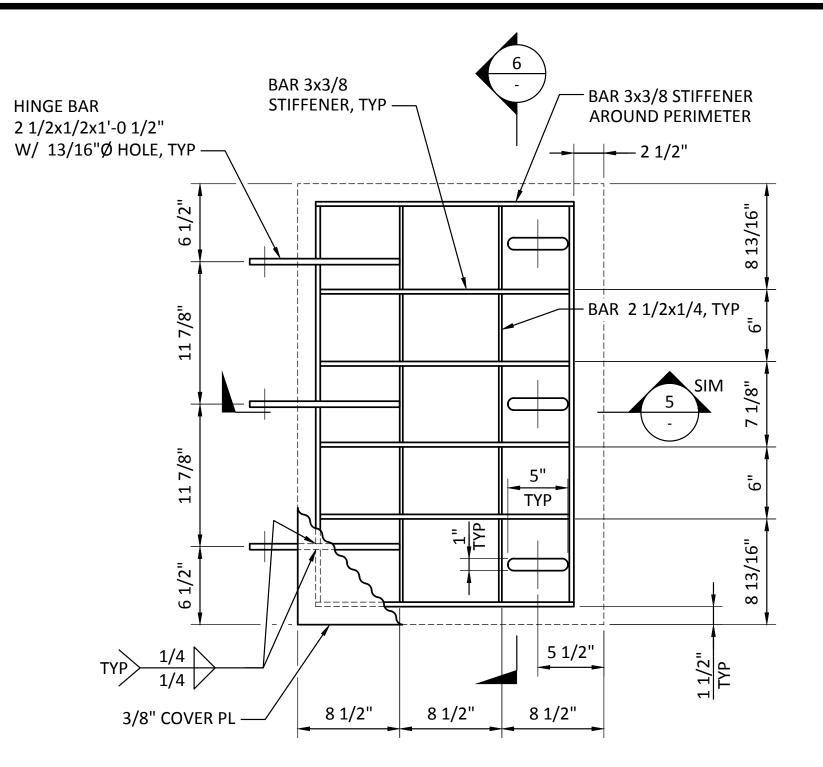
7 DETAIL - COVER LOCK
- SCALE: 3" = 1'-0"



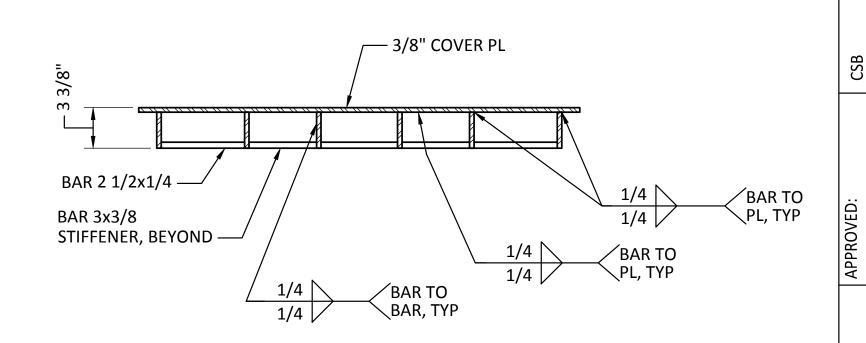
DETAIL - VAULT COVER (TYPE B) SCALE: 1 1/2" = 1'-0"







3 DETAIL - VAULT COVER (TYPE C) S16.0 SCALE: 1 1/2" = 1'-0"





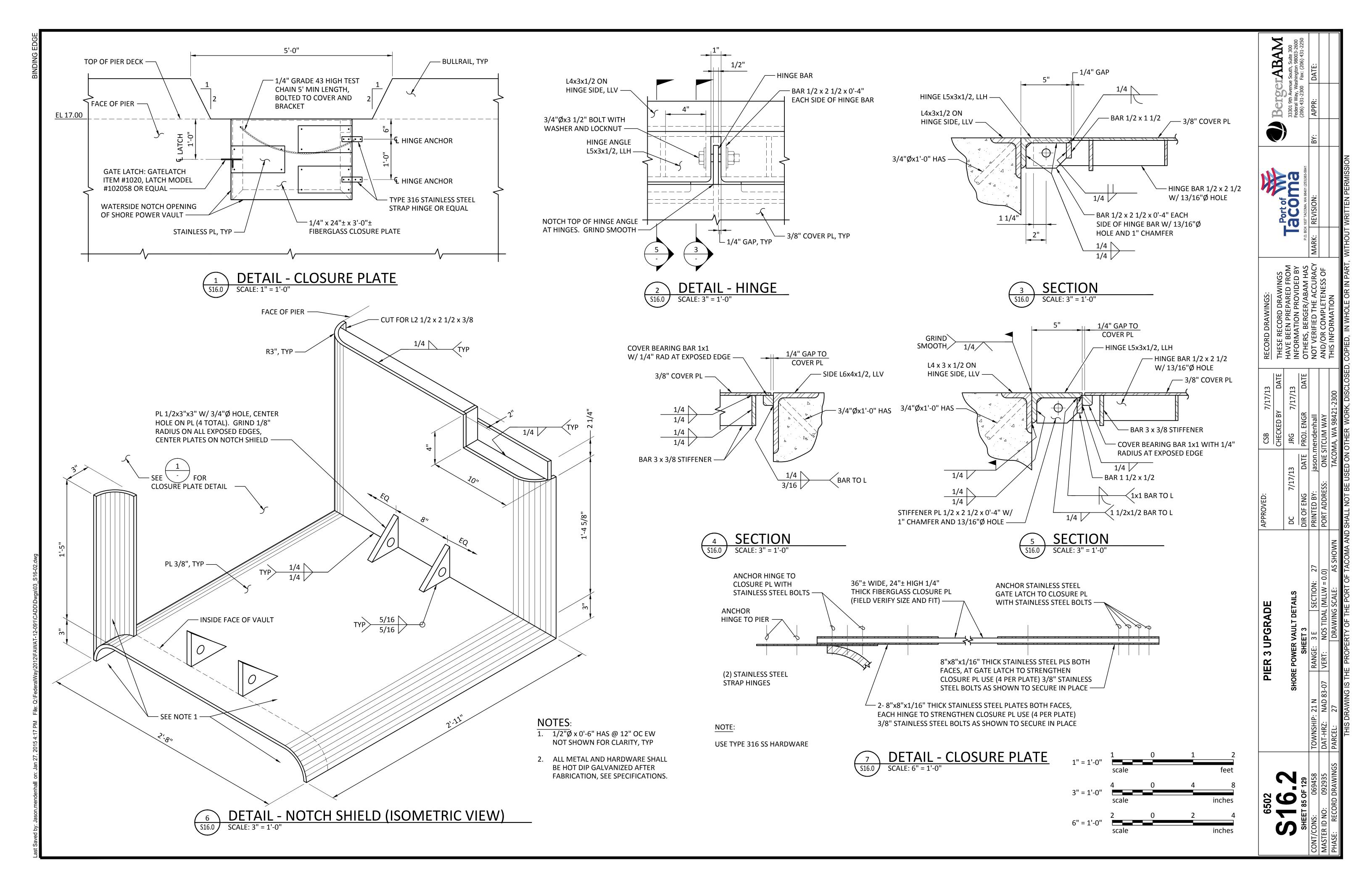
NOTE:

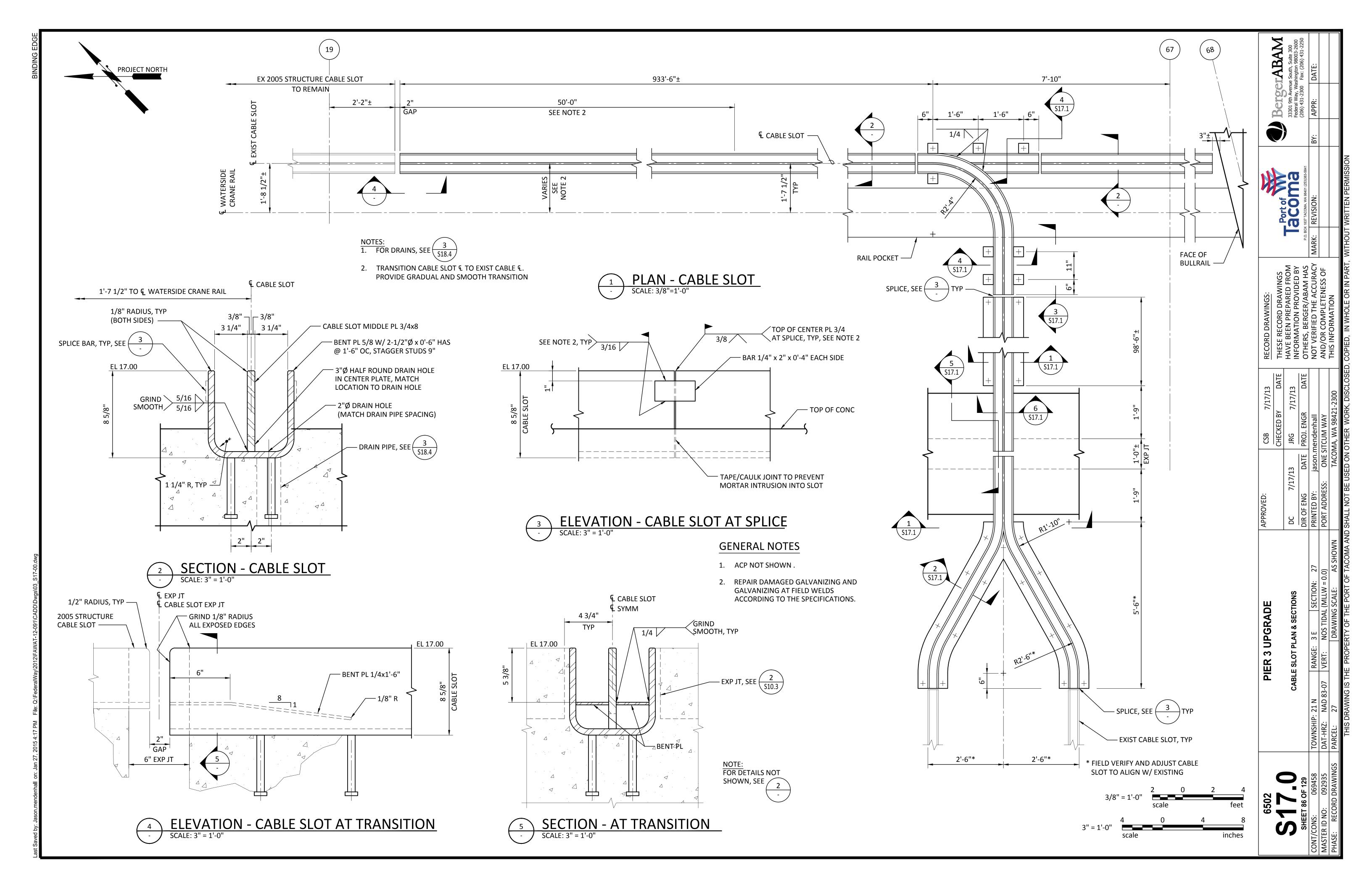
SEE NOTES ON DWG \$16.0.

3" = 1'-0"	4 scale	0	4	8 inches
1-1/2" = 1'-0"	8 scale	0	8	16 inches

6502	<u>a</u>	PIER 3 UPGRADE	ADE	APPROVED:		CSB	7/17/13	RECO
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_ _ _	SHOR	SHORE POWER VAULT DETAILS	DETAILS	DC	7/17/13	JRG	7/17/13	HAVE
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: 069458	TOWNSHIP: 21 N	RANGE: 3 E	SECTION: 27	PRINTED BY:		jason.mendenhall		NOT
VO: 092935	DAT-HRZ: NAD 83-07	VERT: NOS TID	NOS TIDAL (MLLW = 0.0)	PORT ADDRESS:		ONE SITCUM WAY		AND/
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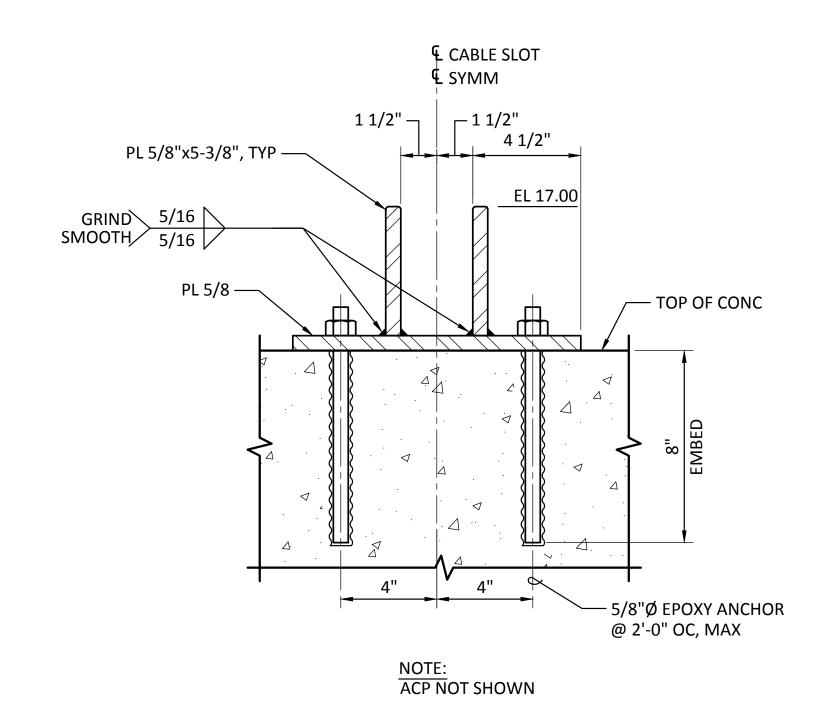
SECTION - DOUBLE CABLE SLOT

SCALE: 3" = 1'-0"

SECTION SCALE: 3" = 1'-0"

PL 5/8"x8", TYP —

PL 3/8x4x0'-4" -

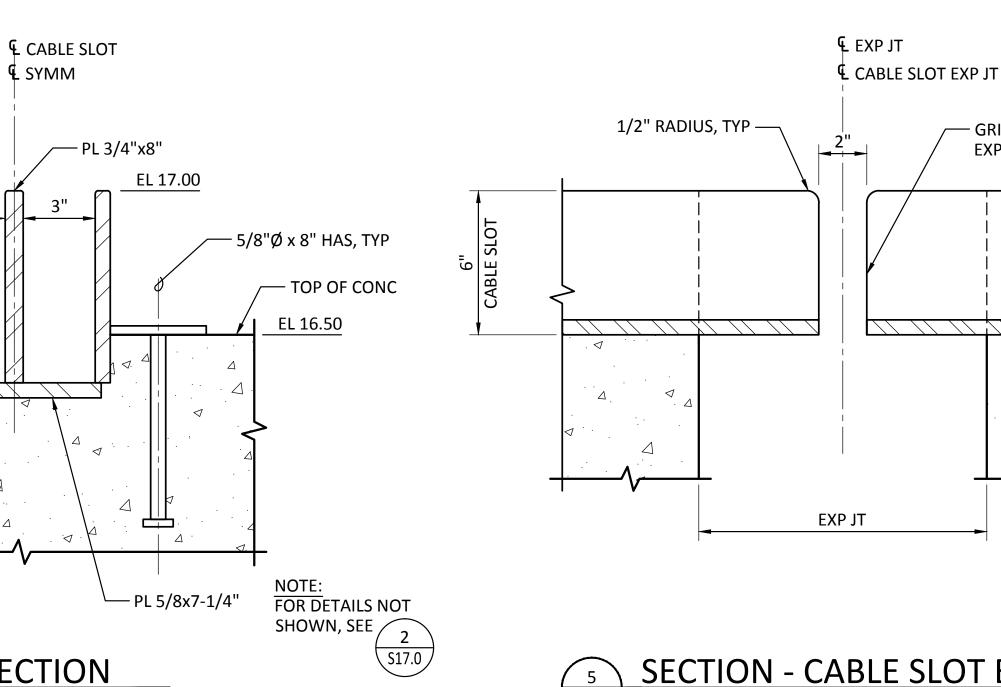


SECTION - SINGLE CABLE SLOT

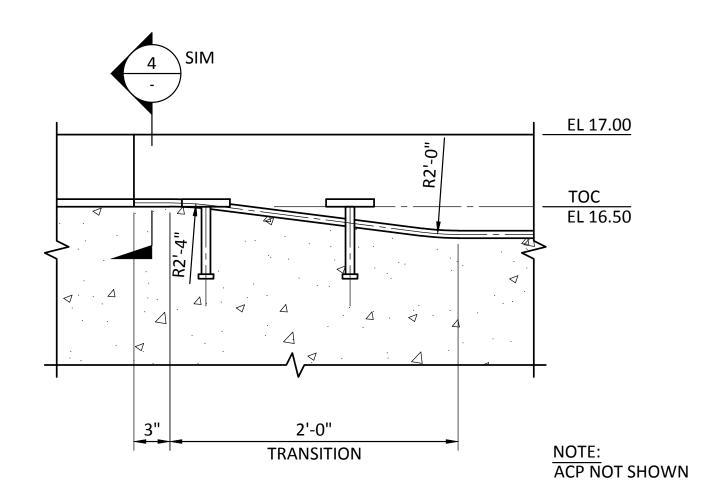
SCALE: 3" = 1'-0"

— GRIND 1/8" RADIUS ALL EXPOSED SURFACES, TYP

EL 17.00

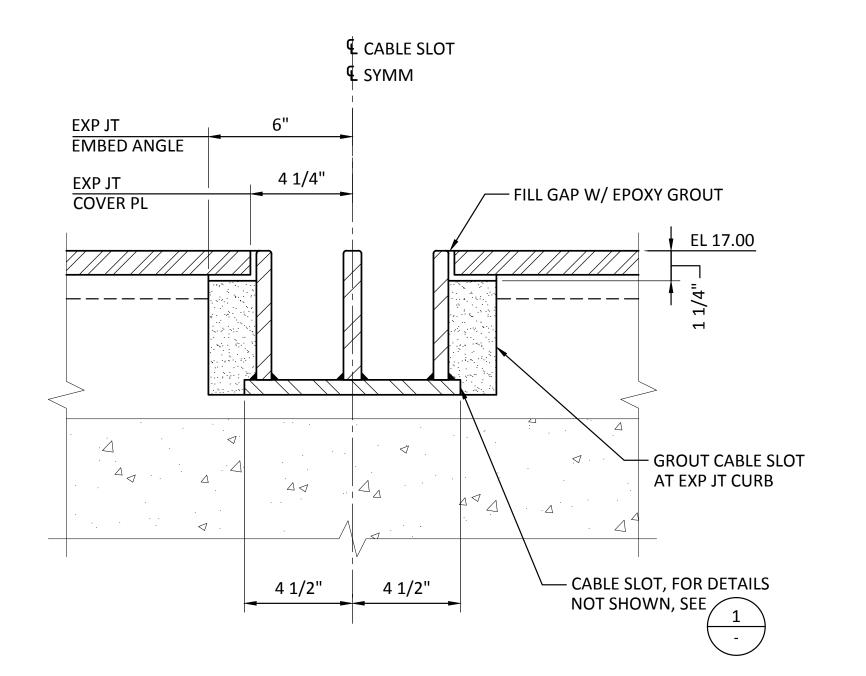


SECTION - CABLE SLOT EXP JOINT
SCALE: 3" = 1'-0"



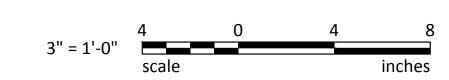
SECTION - CABLE SLOT TRANSITION

SCALE: 1 1/2"=1'-0"



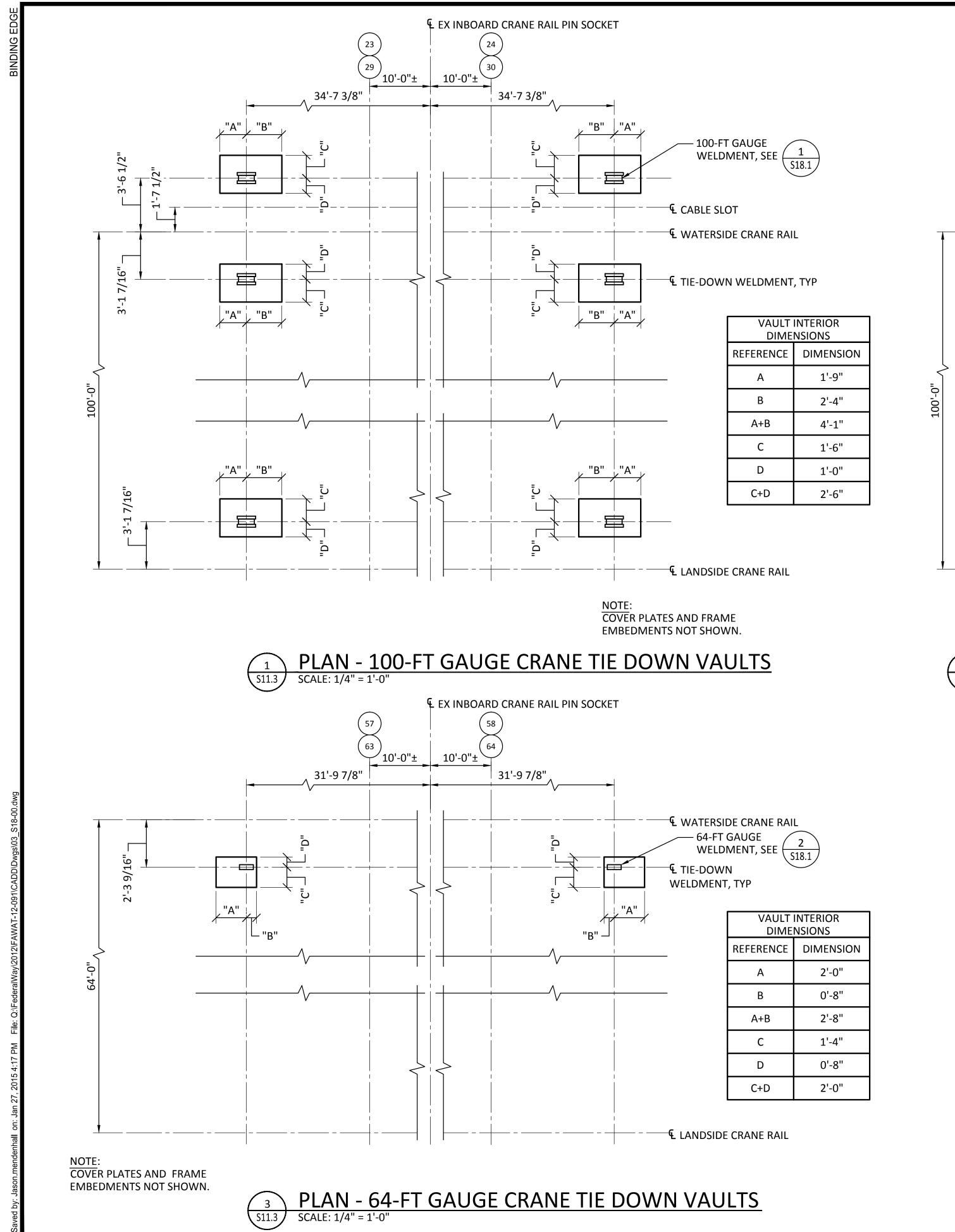
SECTION - CABLE SLOT AT EXP JOINT

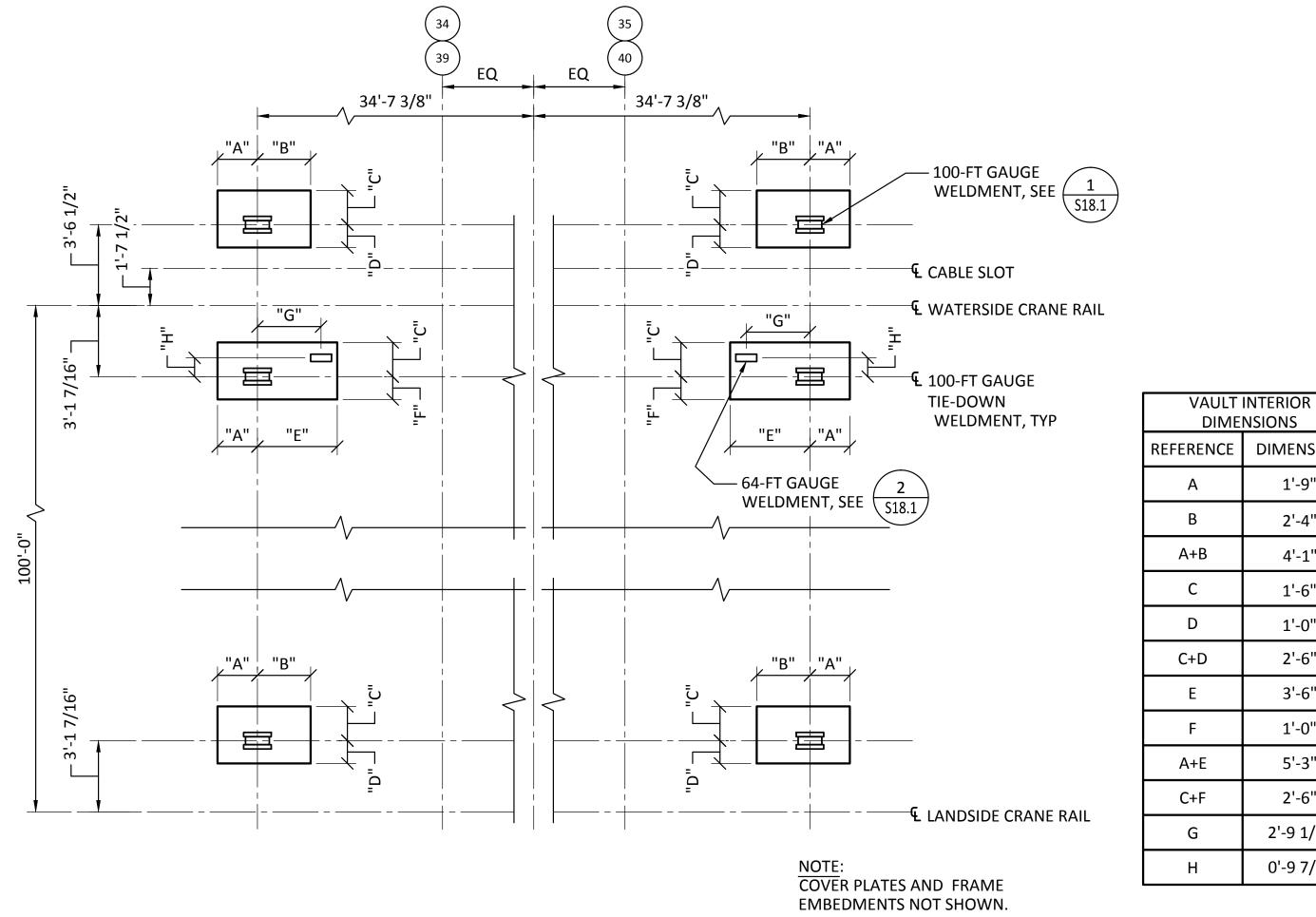
SCALE: 2" = 1'-0"



SMOOTH INSIDE 5/16 TYP, GRIND 5/16

PIER 3 UPGRADE





REFERENCE	DIMENSION
Α	1'-9"
В	2'-4"
A+B	4'-1"
С	1'-6"
D	1'-0"
C+D	2'-6"
E	3'-6"
F	1'-0"
A+E	5'-3"
C+F	2'-6"
G	2'-9 1/2"
Н	0'-9 7/8"

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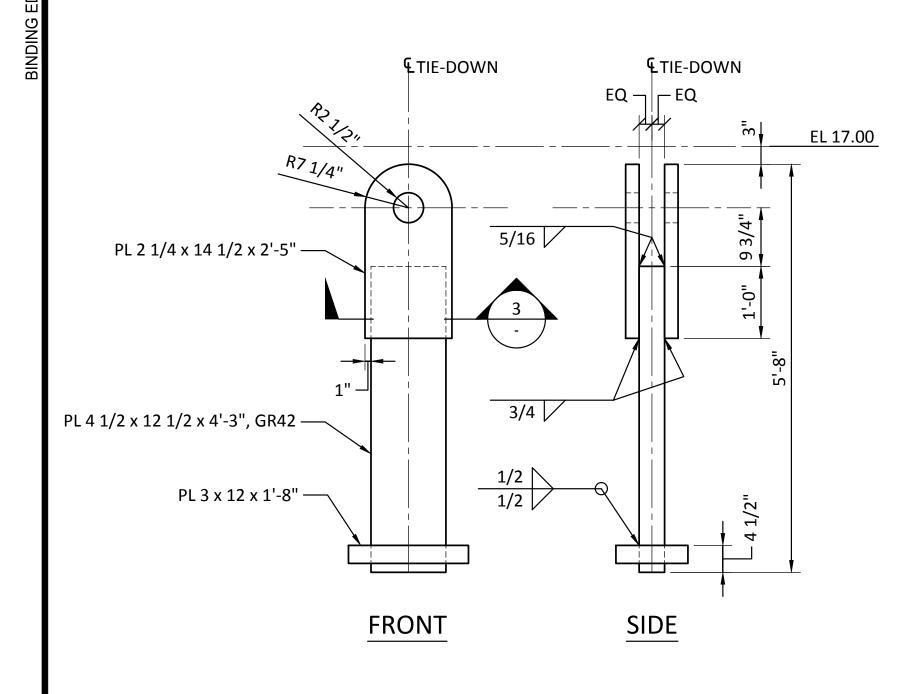
PLAN - COMBINED 64-FT AND 100-FT GAUGE CRANE TIE DOWN VAULTS

SCALE: 1/4" = 1'-0"

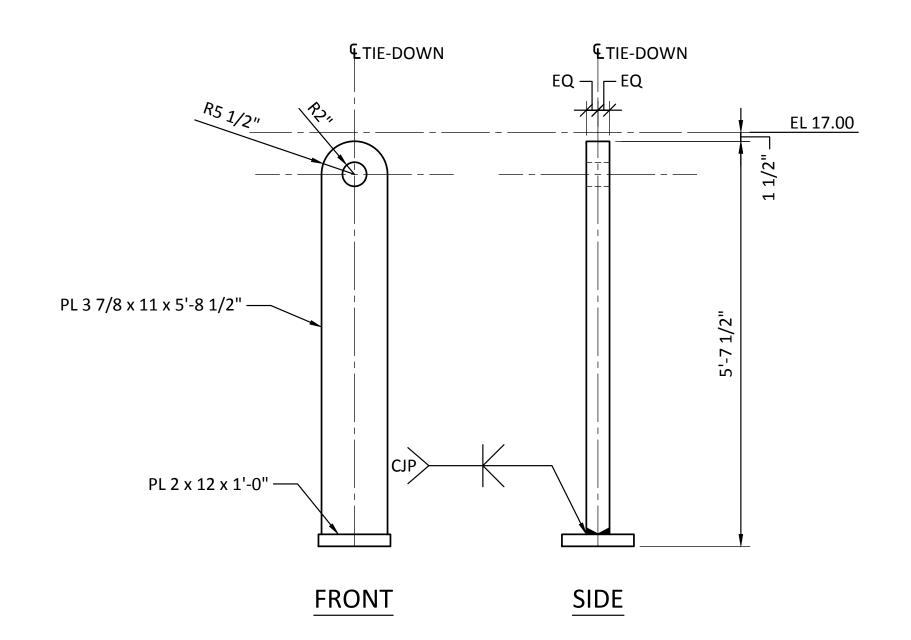
. / . !! ! !!	4	0	4	8
1/4" = 1'-0"	scale			feet

6502 S1

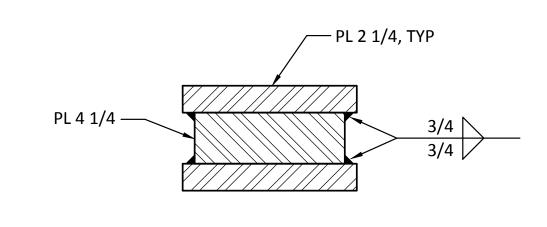
PIER 3 UPGRADE











3	SECTION
	SCALE: 1 1/2" = 1'-0"

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DC	DIR OF ENG	PRINTED BY:	PORT A		SHALL

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PIER 3 UPGRADE	CRANE TIE-DOWN WELDMENTS	3 E	(O O - /VIII/V) IV OIT OOM . TOI/V
ER 3 U	E TIE-DO\	RANGE: 3 E	1/FDT.
d	CRAN	TOWNSHIP: 21 N	70 00 00 10

3/4" = 1'-0"

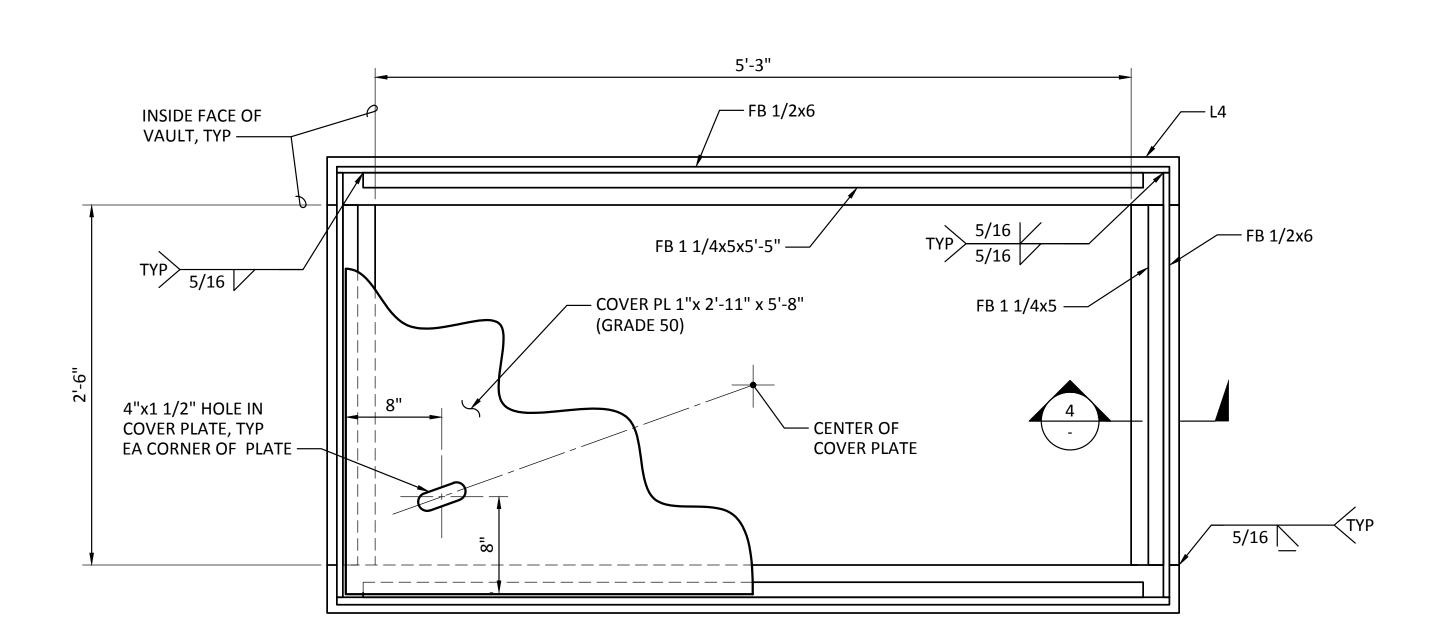
1 0 1 2
scale feet

1-1/2" = 1'-0"

8 0 8
scale inc

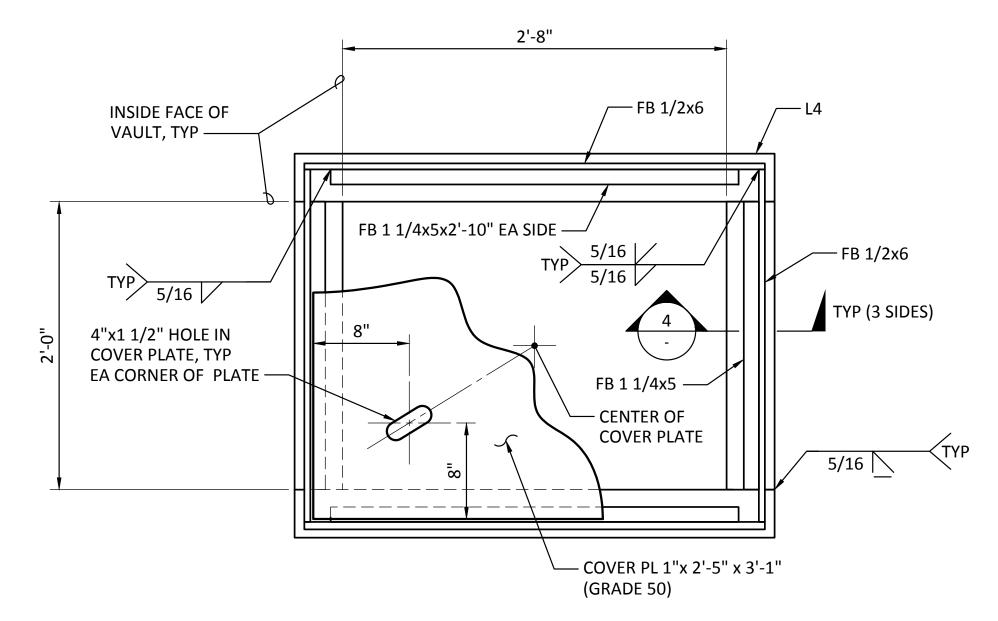
PLAN - TIE-DOWN VAULT FRAME - TYPE 1

S11.3 SCALE: 1 1/2" = 1'-0" (20 TOTAL)



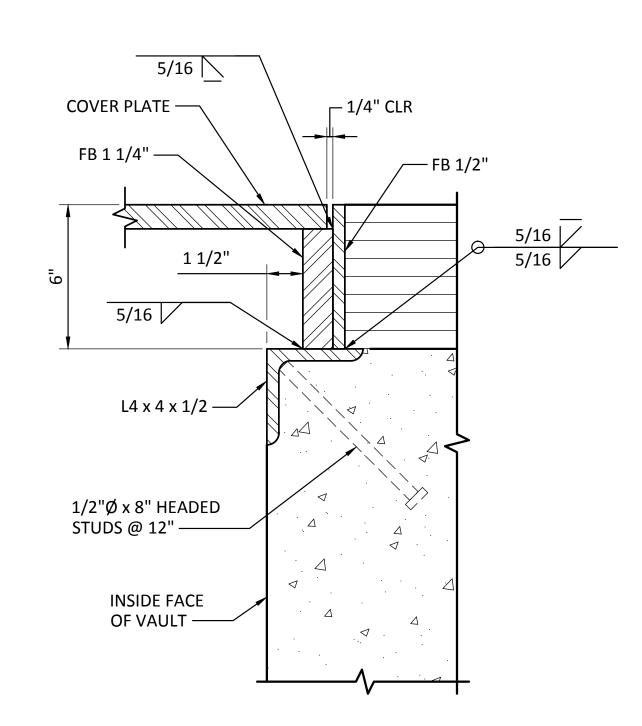
PLAN - TIE-DOWN VAULT FRAME - TYPE 3

S11.3 SCALE: 1 1/2" = 1'-0" (4 TOTAL)



PLAN - TIE-DOWN VAULT FRAME - TYPE 2

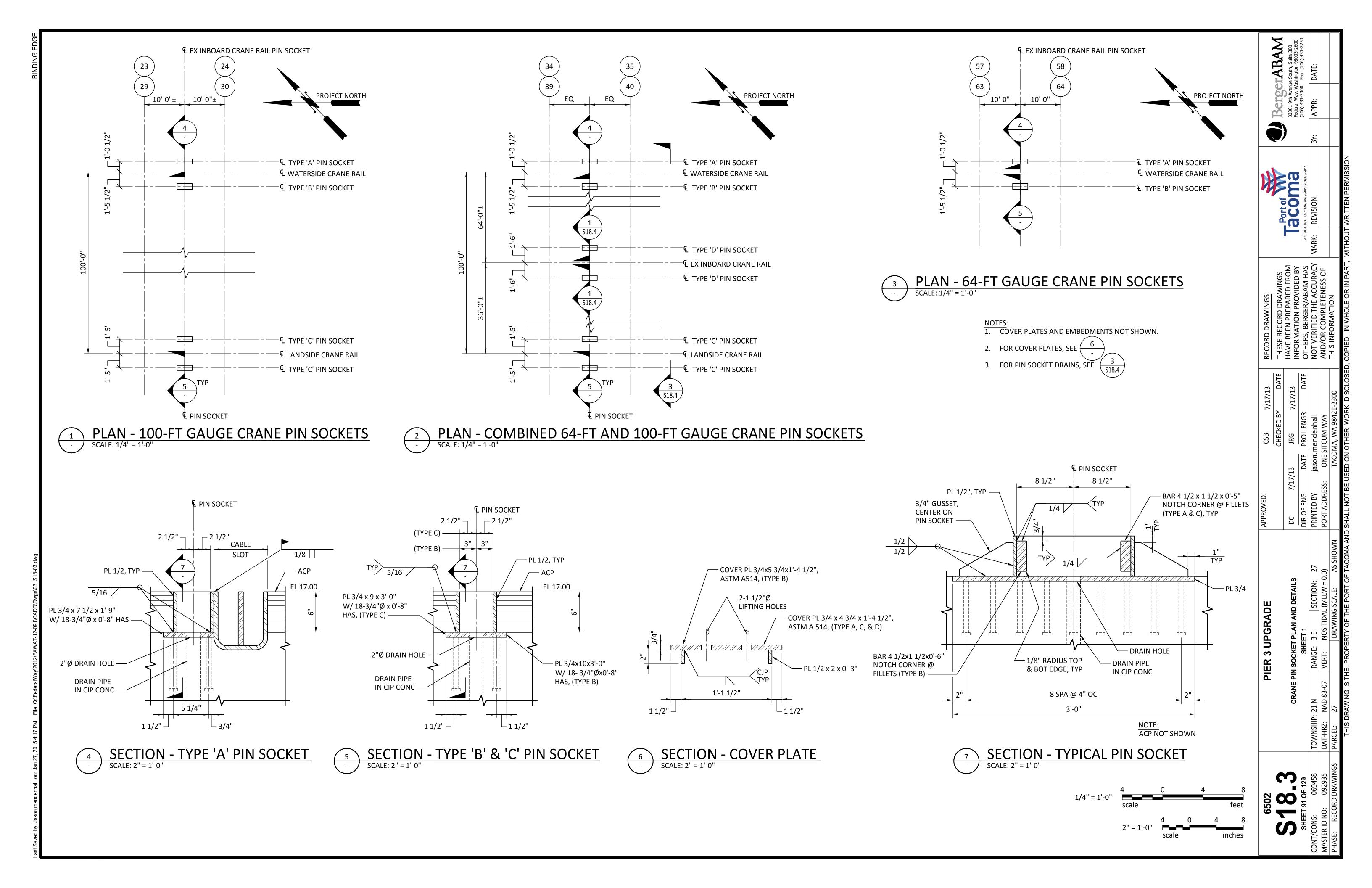
S11.3 SCALE: 1 1/2" = 1'-0" (4 TOTAL)



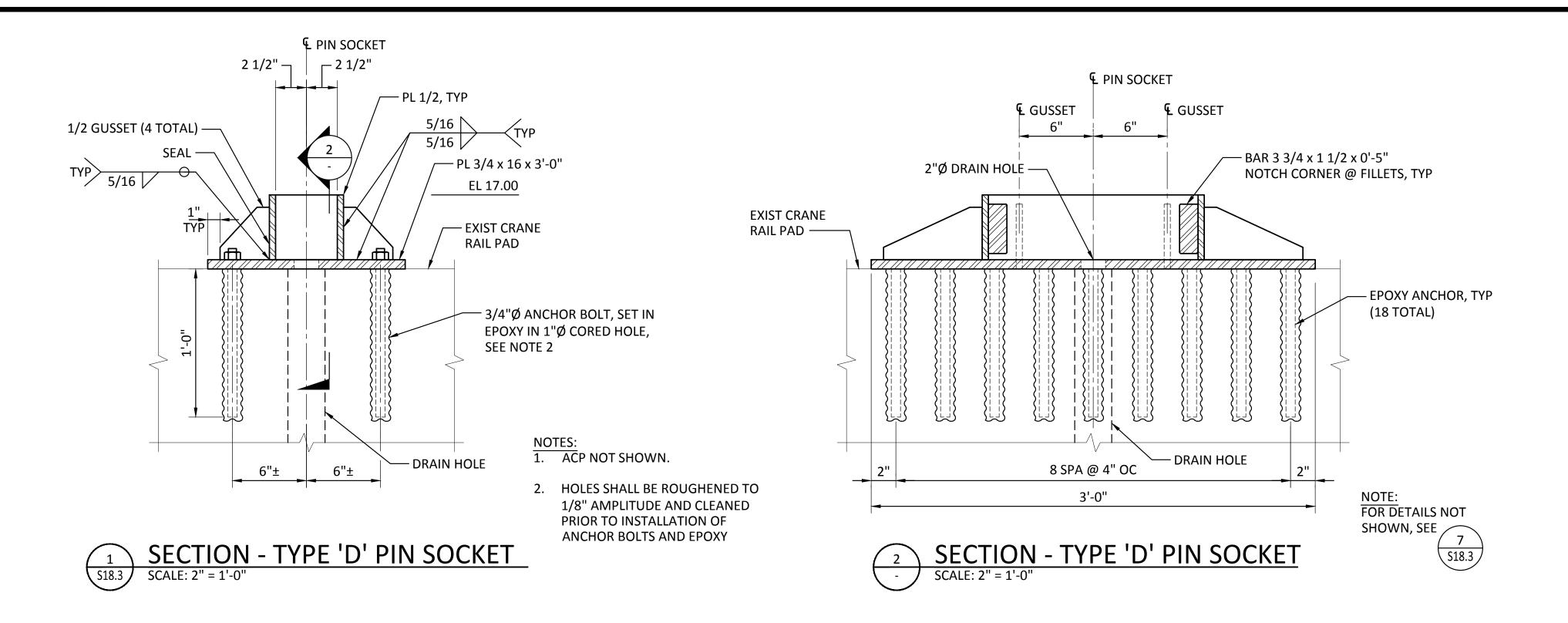
SECTION - COVER PLATE SUPPORT

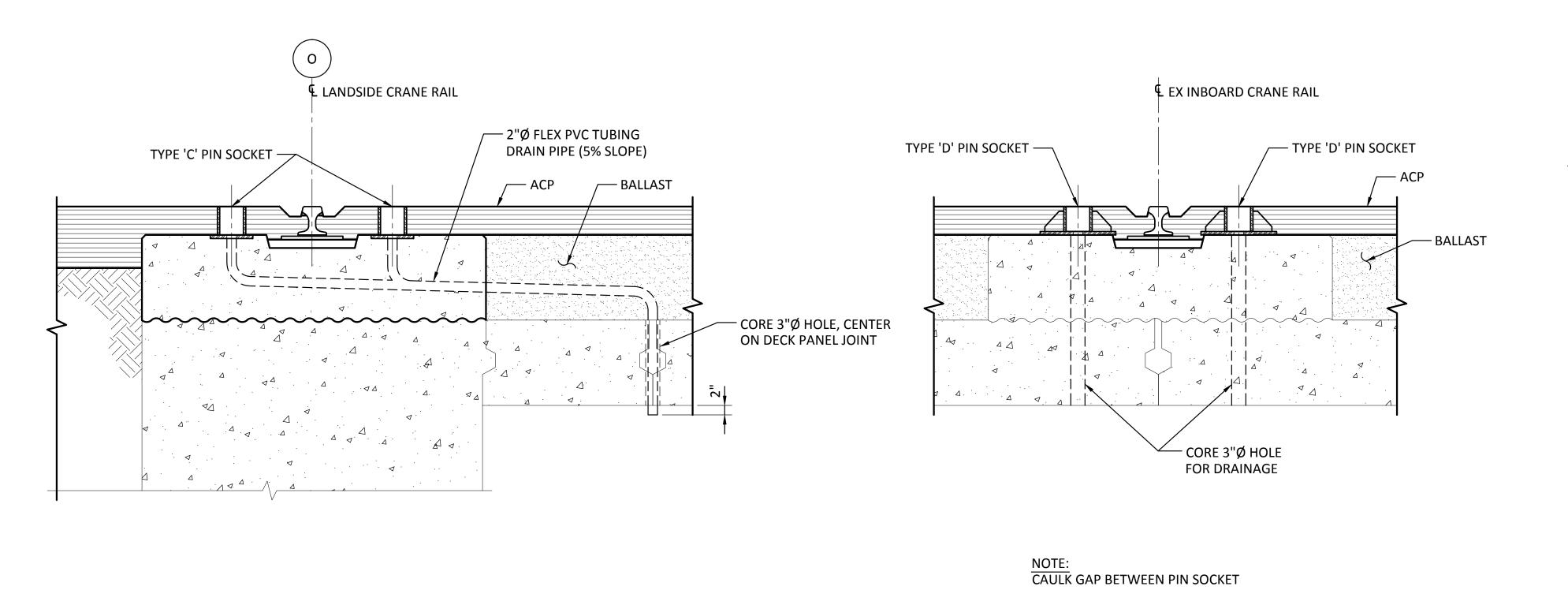
SCALE: 3" = 1'-0"

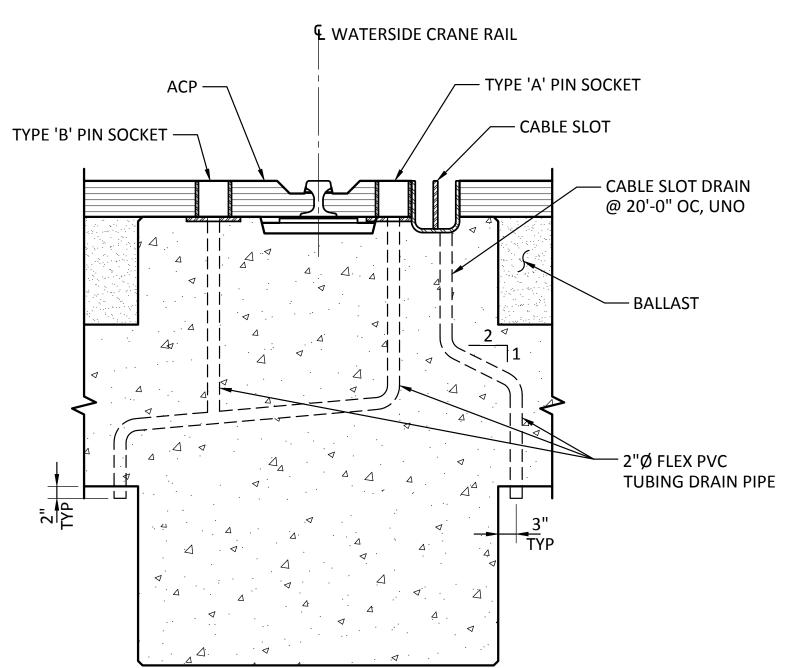
1-1/2" = 1'-0"	8 scale	0	8	16 inches
3" = 1'-0"	4 scale	0	4	8 inches











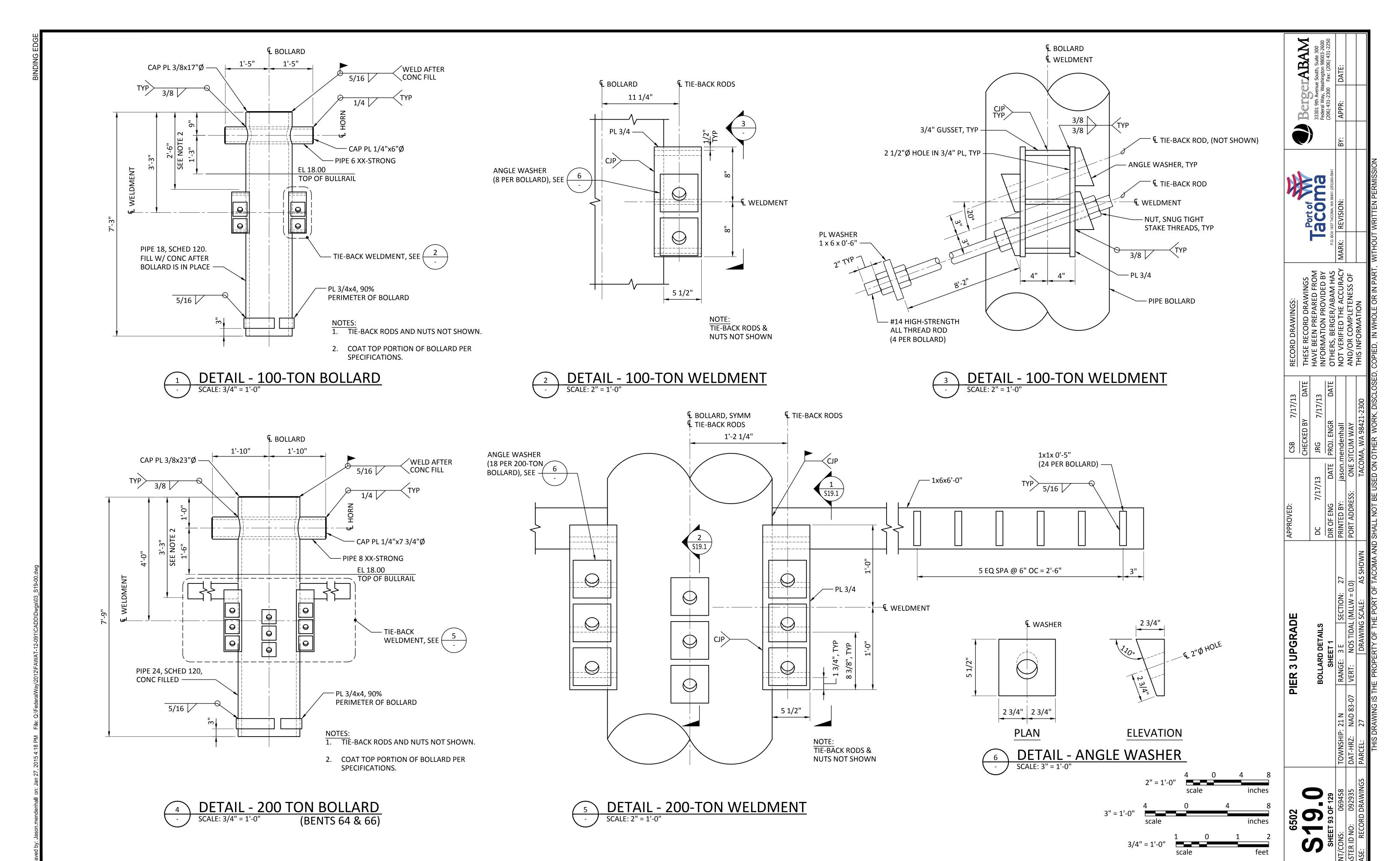
3	SECTION - PIN SOCKET DRAINS
S18.3	SCALE: 3/4" = 1'-0"

DRAIN HOLE AND DRAIN PIPE OR HOLE

3/4" = 1'-0"	1 scale	0	1	2 feet
2" = 1'-0	4)" === sca	0 lle	4 in	8 ches

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7. 4.	CRANE PI	IN SOCKET	CRANE PIN SOCKET PLAN AND DETAILS) DETAILS		DC 7/17/13	/13	JRG	7/17/13	HAVE BEEN PREPARED FROM INFORMATION PROVIDED BY	
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#14 HIGH-STRENGTH
ALL THREAD ROD

PL WASHER
1 x 6 x 0'-6"

ANGLE WASHER, SEE $\binom{6}{$19.0}$

DETAIL - 200-TON WELDMENT
S19.0 SCALE: 2" = 1'-0"

DETAIL - 200-TON TIE-BACK RODS

SCALE: 2" = 1'-0"

€ BOLLARD

— € TIE-BACK ROD, (NOT SHOWN)

— NUT, SNUG TIGHT AND STAKE THREADS, TYP

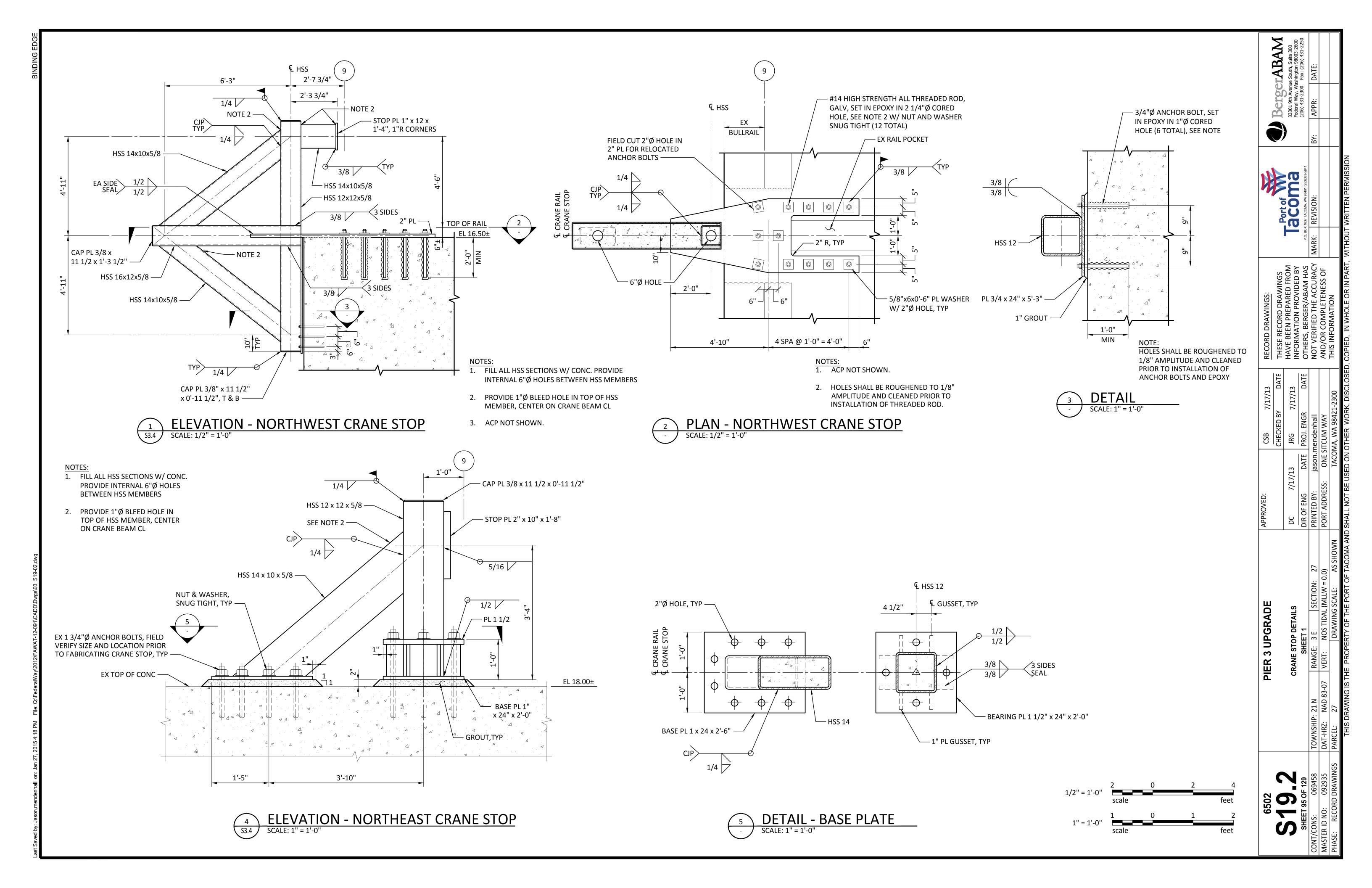
— € TIE-BACK ROD

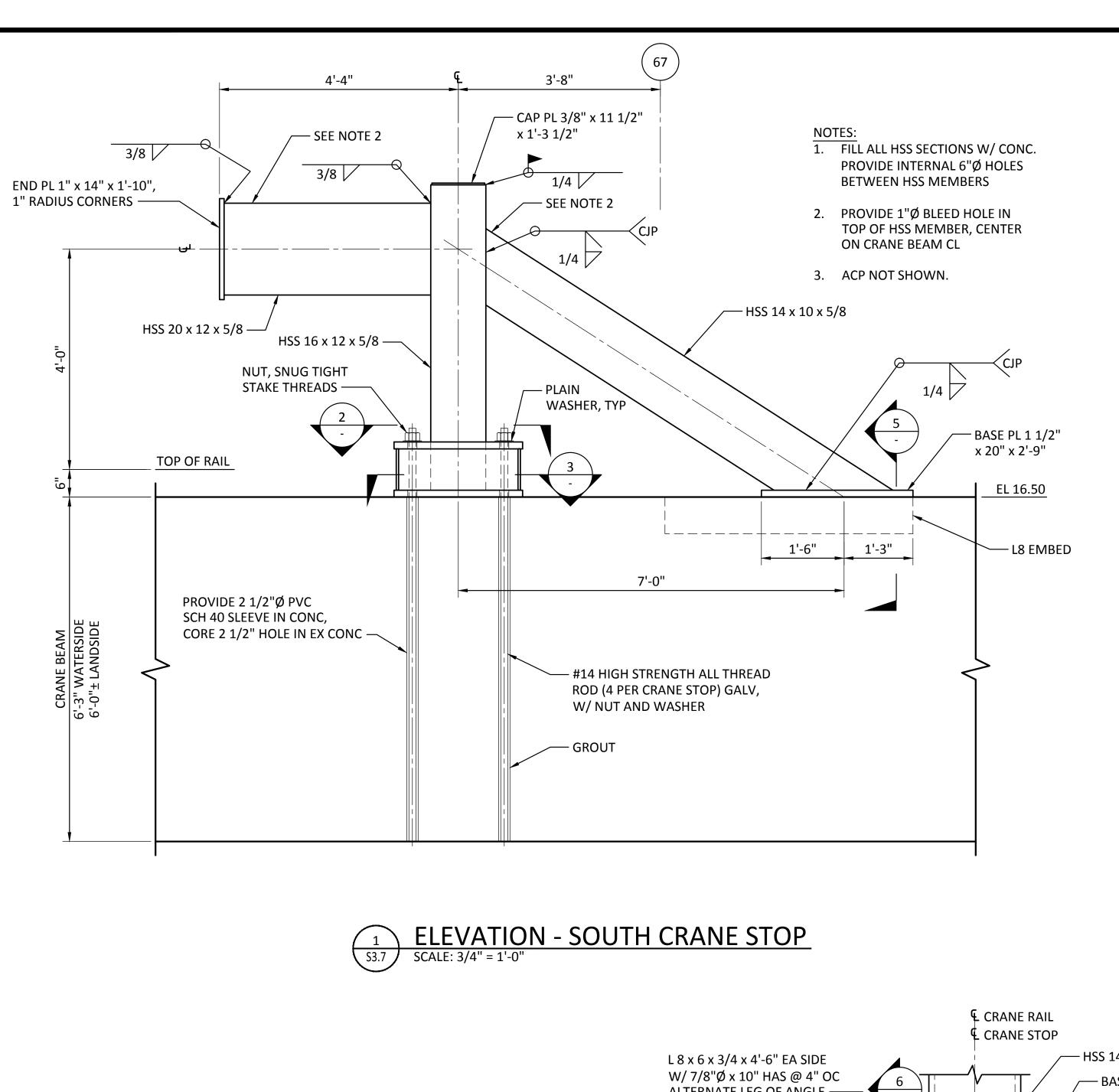
-€ WELDMENT

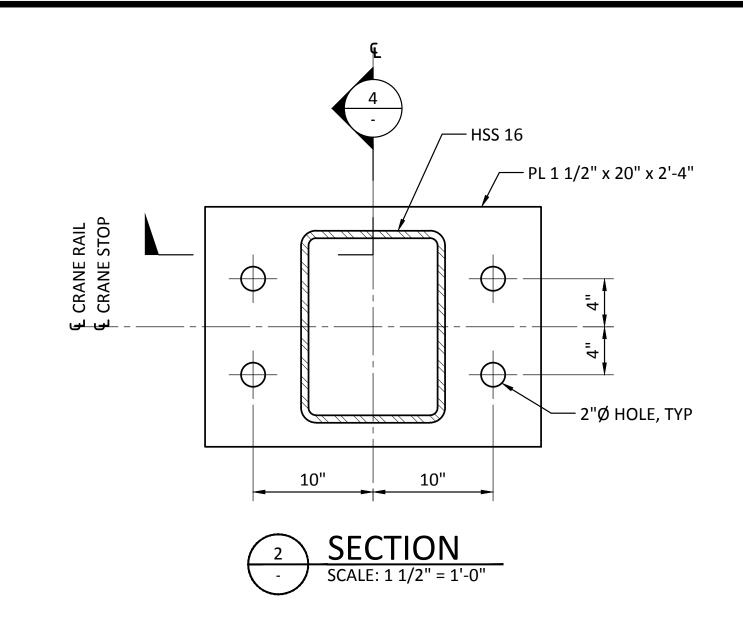
— PIPE BOLLARD

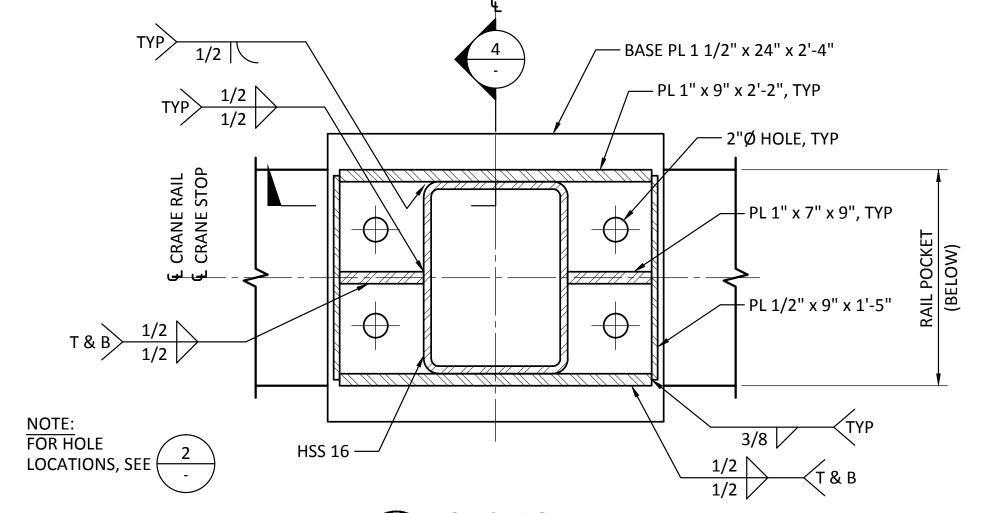
2" = 1'-0" 4 0 4 8 scale inches

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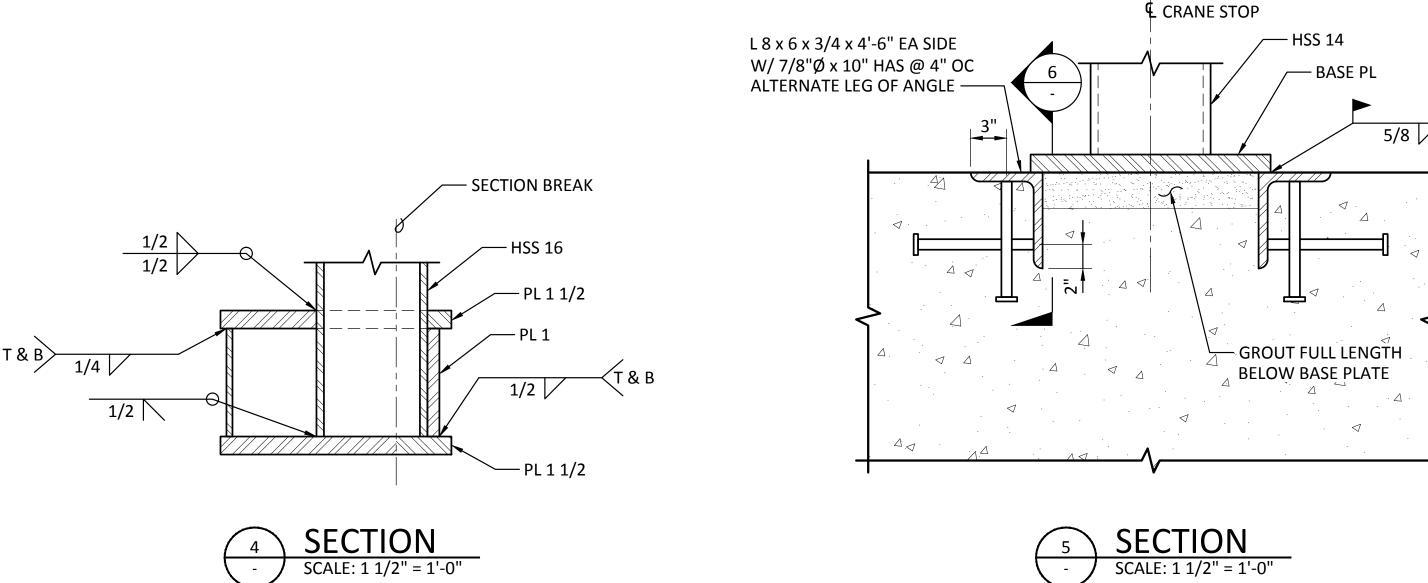


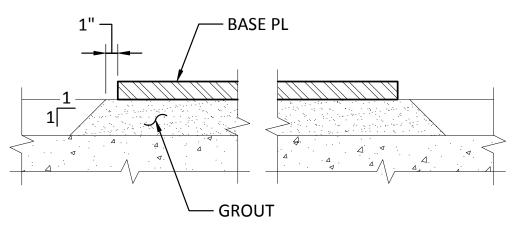




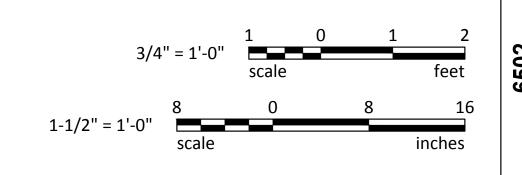


3 SECTION
- SCALE: 1 1/2" = 1'-0"



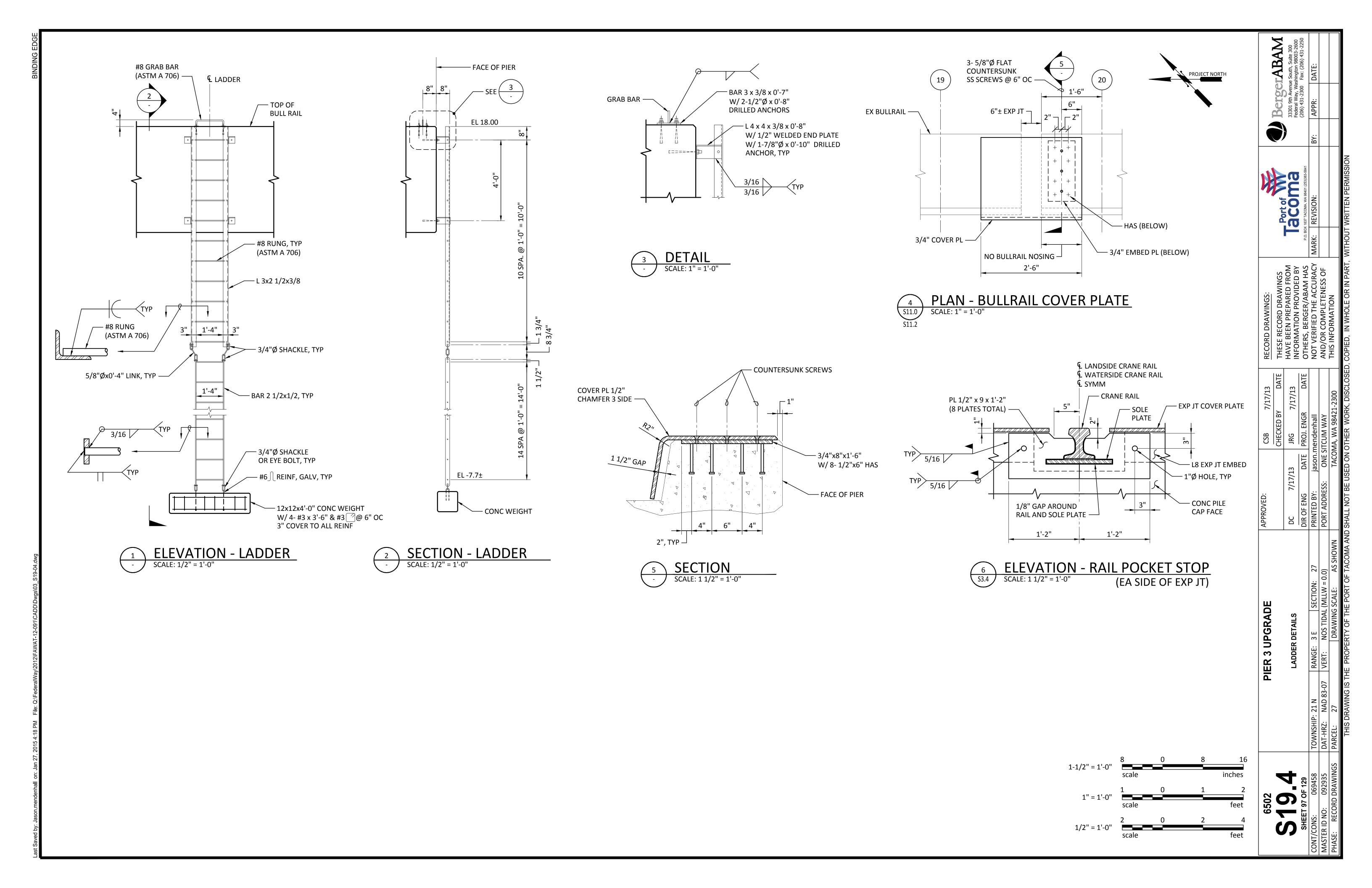


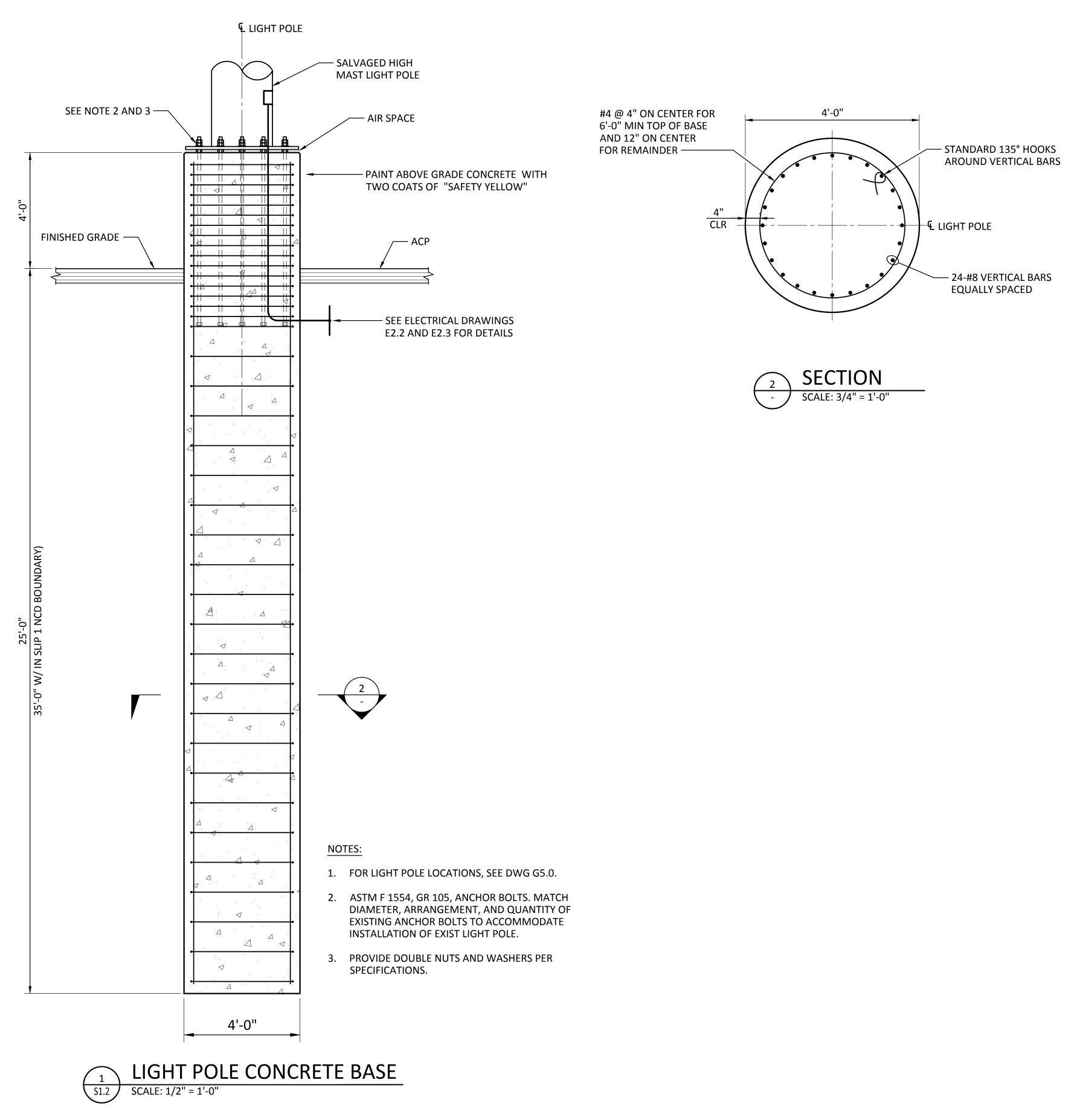
6 DETAIL - GROUT EDGE
- SCALE: 1 1/2" = 1'-0"

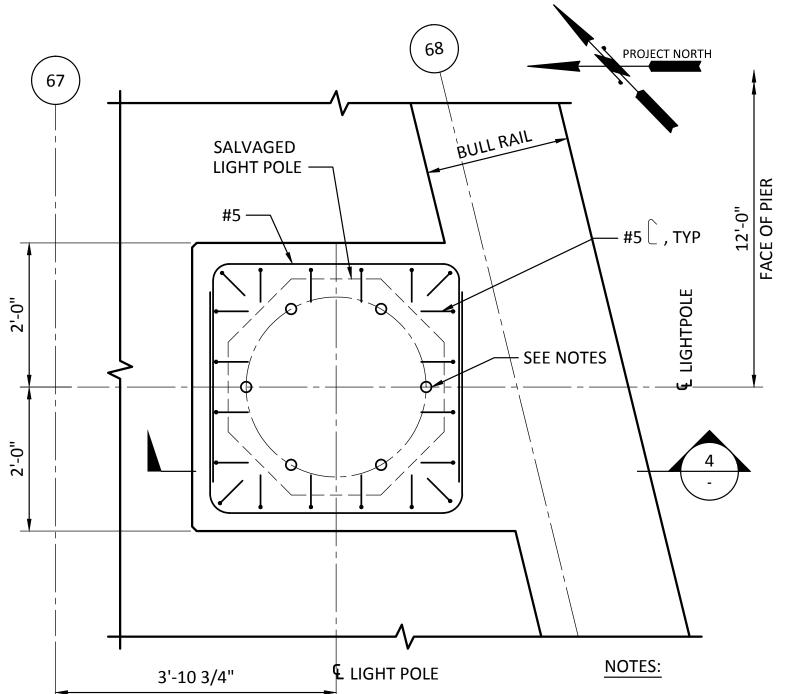


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	RANGE: 3 E		SECTION: 27	27	PRINTED BY:		jason.mendenhall		NOT VERIFIED THE ACCURACY	MARK
83-07	VERT:	NOS TIDA	NOS TIDAL (MLLW = 0.0)	0.0)	PORT ADDRESS:		ONE SITCUM WAY		AND/OR COMPLETENESS OF	
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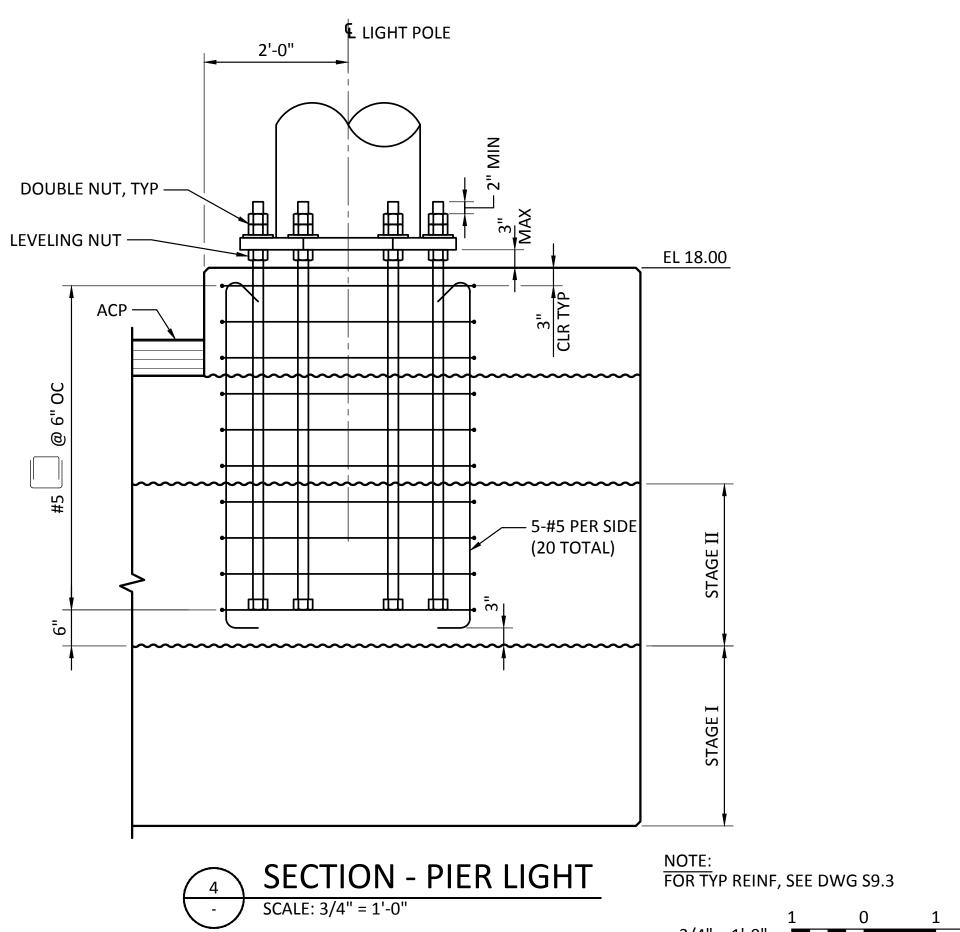


PLAN - HIGH MAST LIGHT

SCALE: 3/4" = 1'-0"

1. ASTM F 1554, GR 105, ANCHOR BOLTS MATCH DIAMETER, ARRANGEMENT, AND QUANTITY OF EXISTING ANCHOR BOLTS TO ACCOMMODATE INSTALLATION OF EXIST LIGHT POLE.

PROVIDE DOUBLE NUTS AND WASHERS PER SPECIFICATIONS.



UPGRADE PIER 3

Tacoma

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